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DESIGN CALCULATION

.

THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

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1.0 DESIGN BRIEF:

THIS DOCUMENT FURNISHES DESIGN CALCULATION FOR THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

1.0 GENERAL DESIGN CRITERIA

2.1. Description of the structure

The Residential Building consists of Ground Floor + 5 Upper floors and the structure is designed as a RCC framed structure.

1. Size of the building : 17.30 x 18.0m
2. Height of the building : 18.0m
3. Number of storeys above GL : Stilt Floor + 5 upper floors
4. Floor to floor height of this building : Ground and First floors - 3.0m
   : Typical floor – 3.0m
5. Type of foundation : Isolated foundation as per geotechnical report

2.2. Materials Used

Following materials are generally used in the design of the various components of the structure.
For Concrete Structure

- M25 grade concrete for all reinforced concrete parts of the structure
- Fe500 grade Structural reinforcement steel
- Masonry for all walls : Brick / Block

2.3. Design Codes & Standards

Following codes and specifications have been generally used in the design of the members of the structure.

- IS 456 : 2000 Code of practice for Plain and Reinforced concrete
- SP: 16 Design Aids for Reinforced concrete
- IS 875 : 1987 Design Loads for building and structures
- IS 1893 : 2002 Criteria for Earthquake Resistant design of structures
- IS 13920: 1993 Ductile Detailing of R.C structures subjected to Seismic forces
- SP: 34 Handbook on Concrete Reinforcement & Detailing
- IS 800 : 1984 Code of practice for general construction in steel
- IS 2062 : 1999 Steel for general structural purposes specification
- IS 1786: 1985 Specification for high strength deformed bars and wires for concrete reinforcement

3.0 LOADING

Primary Load:

- Dead Load (DL)
- Live Load (LL)

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3.1. Dead load (DL)

Material weight

- Density of Concrete =25 kN/m^3
- D.L. due to partitions
  (Light Partition as per IS 875) =1.50 kN/m^2
- D.L due to floor finish 50mm thick = 0.05x25 = 1.25 kN/m^2

3.2. Live load (LL)

Minimum Uniformly Distributed Live Loads:

Occupancy or use	Live Load (kn/m^2)
• All rooms and kitchens	2.00
• Toilet and bath rooms	3.00
• Corridors, passages, staircases	3.00
• Balconies	3.00
• Terrace Floor	1.50
• Basic wind speed	=50 m/sec
• Seismic Zone	=Zone III

3.5 Loading Combinations

The following combinations of Loads as per IS 1893 is considered in the STAAD Analysis

- 1.5(DL +LL)
- 1.2(DL + LL)

The below load combinations are used to check the structure in serviceability condition.

- 1.0(DL +LL)

4.0 STRUCTURAL MATERIAL:

4.1 Concrete Material Specifications:

Concrete compressive cube strength	- 25 N/mm^2
Minimum cement content	- Refer standard drawings for general & Legends
Cement type	- OPC
Free water cement ratio	- 0.5 (Maximum)
Nominal maximum size of aggregate	- 20mm UNO

4.2 Reinforcement:

Steel reinforcement shall be deformed bars, the characteristic of reinforcement bars shall conform to the following specification:

Grade of Steel	Actual Yield Strength (MPa)
Grade Fe500	Minimum 500 MPa,

4.4 Concrete cover:

Cover for Reinforcement is considered as per Table 16A of NBC, for a Fire resistance of 2 hrs.

- Raft Slab = 50mm
- Footing = 50mm
- RC Wall = 30mm

- Columns = 40mm
- Beams = 30mm
- Slabs = 25mm

5.0 Structural Description

5.1. Method of analysis

Structure analysis & design:

The structure is analyzed as a Space frame structure in STADD Pro Vi8. Columns, Footing, Basement walls, slabs, Beams and Staircases are designed as reinforced concrete members.

5.2. Framing System, Analysis and Design

Limit State concept of design will be used as per IS: 456-2000 & SP - 16. The detailing of reinforcement shall be done satisfying the requirements of IS: 456-2000, SP-16, IS: 13920, IS: 4326-1993 and SP-34.

The Slabs and beams are checked for deflection as per IS 456:2000. Cover for Reinforcement is considered as per Table 16A of NBC, for a Fire resistance of 2 hrs.

APPENDIX I

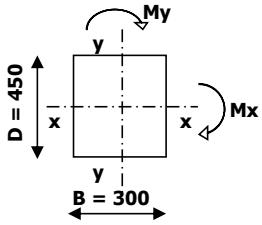
Design Calculations

Column Design

PROJECT	STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL	DOCUMENT. NO.	DATE	DESIGNED	DATE
TITLE	Design for a Short Column with Biaxial Bending	REVISION	DATE	CHECKED	DATE

CL. NO.	DESIGN CALCULATIONS	REFERENCES

Column No:



CRITICAL LOAD CASE:

1.0 Basic Data

1.1 Material Data

Grade of Concrete — M **25**

Grade of Steel — Fe **500**

1.2 Codal Data

Nominal Cover		=	**40** mm			Cl 26.4.2.1,IS456:2000
Diameter of the Bar		=	**25** mm	≥	12 mm	
d'		=	52.5 mm			

1.3 Basic Dimensions

Unsupported Length of the Column	L_{ox}	=	**3000** mm			
	L_{oy}	=	**3000** mm			
	L_{ex}	=	**3000** mm			Table 28 ,
	L_{ey}	=	**3000** mm			IS456:2000
Size of the Column	B X D	=	**300**	x	**450** mm	

1.4 Analysis Results

Factored Axial Load	P_u	=	**780.0** kN	STAAD
Factored Moment	M_{ux}	=	**22.00** kNm	OUTPUT
Factored Moment	M_{uy}	=	**71.00** kNm	

2.0 Check for Slenderness

Slenderness Ratio about Major Axis	L_{ex} / D	=	6.667	
Slenderness Ratio about Minor Axis	L_{ey} / B	=	10.000	

Since Leff / LLD <12 Designed as Short Column — Cl 25.1.2, IS456:2000

3.0 Check for Eccentricity

Minimum Eccentricity	e_x = L_{ox} / 500 + D / 30	=	21.0 mm	>20		Cl 25.4
	e_y = L_{oy} / 500 + B / 30	=	16.0 mm	<20		IS456:2000

3.1 Moments Due to Minimum Eccentricity

	Me_x	=	16.38 kN.m
	Me_y	=	15.60 kN.m

4.0 Design Moment

Design Moment about X axis	M_{ux}	=	22.00 kN.m
Design Moment about Y axis	M_{uy}	=	71.00 kN.m

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CL. NO.	DESIGN CALCULATIONS				REFERENCES
5.0	**Uniaxial Moment Carrying capacity of the Section about X & Y- axis**				
	d' / D		=	0.117	
	d' / B		=	0.175	
	Assume the Reinforcement Percentage	p	=	**2.00**	
	p / f_{ck}		=	0.080	
	$p_u / f_{ck}BD$		=	0.231	
	Reinforcement distributed equally on four sides			**4**	
	$M_{ux1} / f_{ck}BD^2$		=	0.137	Chart 43 - 46
	$M_{uy1} / f_{ck}DB^2$		=	0.119	SP 16 : 1980
	Moment carrying capacity about X axis	M_{ux1}	=	207.89 kN.m	
	Moment carrying capacity about Y axis	M_{uy1}	=	120.61 kN.m	
6.0	**Calculation of P_{uz}**				
	$P_{uz} = 0.45 f_{ck} A_c + 0.75 f_y A_{sc}$		=	2500.88 kN	Cl 39.6 , IS456:2000
	P_u/P_{uz}		=	0.312	
	α_n		=	1.186	Cl 39.6 , IS456:2000
7.0	**Criteria for Biaxial Bending (Bresler's Formulation)**				
	$(M_{ux}/M_{ux1})^{\alpha n} + (M_{uy}/M_{uy1})^{\alpha n}$		=	**0.603** < 1	Cl 39.6 , IS456:2000
				Hence Safe	
8.0	**Longitudinal Reinforcement**				
	Area of compression Steel	$A_{sc,reqd}$	=	**2700** mm^2	
	10 Nos Y20 + Nos Y20	$A_{sc,pro}$	=	**3142** mm^2	
8.1	**Check for Minimum & Maximum Area of Steel**				
	Minimum Area of steel (0.8 % of A_g,dir)		=	**441** mm^2	Cl 26.5.3.1 a & b ,
	(Based on the area of concrete reqd to resist direct stress)				IS 456 : 2000
	Maximum Area of steel (4 % of A_g)		=	**5400** mm^2	
	441 < **3142** < **5400** mm^2				
	Area of Steel is with in the permissible limit ; Hence O.K				
9.0	**Transverse Reinforcement**				
9.1	Diameter of Transverse Reinforcement		=	**8** mm ≥ 6 mm	Cl 26.5.3.2 c 2) ,
9.2	**Spacing of Lateral Ties :**				
i)	Least Lateral Dimension of the column		=	300 mm	Cl 26.5.3.2 c 1) ,
ii)	16 times the Smallest dia of Longitudinal Steel		=	320 mm	IS 456 : 2000
iii)			=	300 mm	
	Reqd. Spacing of Ties (Minimum of the above)		=	300 mm	
	Provide 8 mm dia Lateral Ties @ **300** mm Spacing				
	Provide closer spacing on either side of the column above and below beam / slab				

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Footing Design

STILT+5 FLOORS @ PLOT NO:7 &
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Design of Isolated Footing **F2** **Node :** **38**

Basic Data:

Net Safe Bearing Capacity	150	kN/m^2
Unit wieght of Soil	18	kN/m^3
Unit wieght of Concrete	25	kN/m^3
Grade of Concrete	25	N/mm^2
Grade of Steel	500	N/mm^2
Clear Cover to Reinforcement	50	mm
Earth Fill above FGL	0	mm
Base Area of Footing	8.37	m^2
Ixx	6.70	m^4
Izz	5.08	m^4
Weight of Footing	156.94	kN
Weight of soil	181.91	kN

Gross Bearing Pressure Calculation (kN/m^2)

Critical Load Combinations	Pmax	Pmin
200	183.85	183.8530466
0	40.48	40.48

Gross SBC (w/o increase) 186 kN/m^2

Hence **OK**

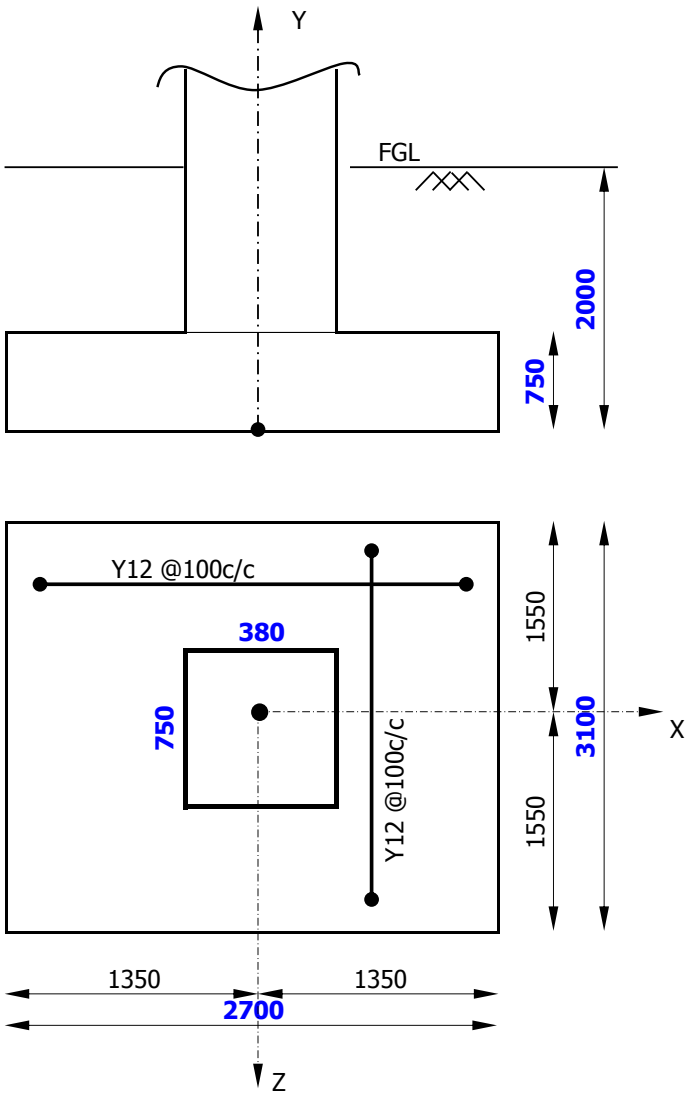
Factored Net Upward Pressure Calculation

Load Factor for Design 1.5

Critical Load Combinations	Pmax	Pmin
200	215.05	215.0537634
0	0.00	0.0

Dsgn.(+ve) BM abt. X-axis Mux	148.45	kN.m
Dsgn.(+ve) BM abt. Z-axis Muz	144.69	kN.m
Dsgn.(-ve) BM abt. X-axis Mux	0.00	kN.m
Dsgn.(-ve) BM abt. Z-axis Muz	0.00	kN.m
Provided Eff. depth for Mux	694.00	mm
Provided Eff. depth for Muz	682.00	mm
Mu,lim	1547	kN.m

Depth provided is **OK**



Bottom Reinforcement

						Ast Provided	Ast Required		
Provide	12	@	100	C/C	along Z	1131	900.0	mm^2	Hence
Provide	12	@	100	C/C	along X	1131	900.0	mm^2	**OK**

Bottom Reinforcement

Ast Required for Mux	498.9	mm^2
Ast Required for Muz	494.9	mm^2
Min. Ast Reqd.	900	mm^2

Top Reinforcement

Ast Required for Mux	0.00	mm^2
Ast Required for Muz	0.00	mm^2
Min. Ast Reqd.	Not Reqd.	mm^2

STILT+5 FLOORS @ PLOT NO:7 &
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Design of Isolated Footing **F2** **Node :** **38**

Check For Flexural / Oneway Shear

	Actual	Allowable		Hence
Shear Stress (Plane ll to X)	0.149	0.302	N/mm^2	**OK**
Shear Stress (Plane ll to Z)	0.151	0.304	N/mm^2	**OK**

Check For Punching / Twoway Shear

Allowable Shear Stress 1.25 N/mm^2
Actual Shear Stress 0.50 N/mm^2 Hence **OK**

Check For Overturning

Co-efficient of friction	=	0.57				
Factor of safety for overturning, sliding	=	1.4				
Net Moments acting at Pad Level, M_{xx}	=	0.00	kNm			
Net Moments acting at Pad Level, M_{zz}	=	0.00	kNm			
Weight of pad	=	156.94	kN			
Weight of Over burden Earth	=	181.91	kN			
Total downward Load	=	1538.85	kN			
Stabilizing moment along X-Dir	=	1538.85	x	1.35		
	=	2077.45	kNm			
Actuating moment, M_{zz}	=	0.00	kNm			
Factor of Safety (Along X-Dir)	=	2077.45	/	0.0		
		-	>	1.40	Hence	**OK**

Check For Sliding

Resistance against Sliding	=	877.1445	kN			
Max. Shear Load @ Pad level in X-Dir.	=	2.5	kN			
Max. Shear Load @ Pad level in Z-Dir.	=	5.3	kN			
Factor of Safety (Along X-Dir)	=	877.1	/	2.5		
		345.33	>	1.40	Hence	**OK**
Factor of Safety (Along Z-Dir)	=	877.1	/	5.3		
		165.50	>	1.4	Hence	**OK**

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Plinth Beam Design

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CL. NO.	BEAM DESIGN CALCULATIONS				REFERENCES
1.1	**Geomentry Data**				
	Beam Number		=	**PB1**	
	Type of Beam		=	Rectangular Beam	
	Support Condition		=	Continuous Beam	
	Width of beam	b	=	230 mm	
	Overall Depth of Beam	D	=	380 mm	
	Dia of bar	Φ	=	25 mm	
	Nominal Cover	cc	=	30 mm	IS 456:2000 Table 19
	Effective Depth	d	=	337.5 mm	IS 456:2000
	Effective Cover		=	42.5 mm	Cl 23.0
	Effective Span of beam	l_{eff}	=	4500 mm	
	Concrete grade		=	M 25	
	Steel Grade		=	Fe 500	
	$p_{t,lim}$		=	0.944 %	
	$p_{t,min}$		=	0.170 %	IS 456:2000
	p_t & p_{cmax}		=	4.00 %	Cl 26.5.1.1
1.2	**Force Input**				
	Left negative moment (Support 1)		=	84 kNm	
	Mid span positive moment		=	84 kNm	
	Right negative moment (Support 2)		=	84 kNm	
	Shear @ (Support 1)		=	149 kN	
	Shear @ (Span)		=	149 kN	
	Shear @ (Support 2)		=	149 kN	
1.3	**Limiting Moment**				
	M_{ulim} = 0.133 f_{ck} b d^2		=	87.11 kNm	
1.4	**Area of Steel**				
	Minimum Area of Stee = 0.17% b d		=	131.963 mm^2	
	Support 1				
	Negative moment (Support 1)		=	84 kNm	
	Hence the Section is designed as Singly Reinforced Section				
		M_u/bd^2	=	3.206	
	Percentage of tension steel reqd	p_t	=	0.028 %	
	Percentage of compression steel reqd	p_c	=	0 %	
	Required Area of tension steel	$A_{st\ reqd}$	=	131.96 mm^2	
	Required Area of compression steel	$A_{sc\ reqd}$	=	0.00 mm^2	

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CL. NO.	**BEAM DESIGN CALCULATIONS**			**REFERENCES**	

Detail of bars

	I st Layer Nos	I st Layer Dia	II nd Layer Nos	II nd Layer Dia
Tension Reinforcement	3	16		

A_{st} provided = 603.19 mm^2

	Nos	Dia	Nos	Dia
Compression Reinforceme	3	16	0	

A_{sc} provided = 603.19 mm^2

Percentage of tension steel provided	p_t =	0.777	%	**Hence O.K**
Percentage of compression steel provid	p_c =	0.777	%	**Hence O.K**

Check for Shear

Shear		=	149	kN	
Nominal Shear Stress	τv	=	1.92	N/mm^2	Cl 40.1, IS456:2000
Permissible Shear Stress	τc	=	0.92	N/mm^2	β = 1.000
Maximum Shear Stress	$\tau c,max$	=	2.5	N/mm^2	Table 19, IS456:2000
Diameter of the Stirrup bar		=	8	mm	Table 20, IS456:2000
Number of Legs		=	2		
Spacing of Shear reinforcement Required		=	190.0		Cl 40.4, IS456:2000
Maximum Spacing of Shear Reinforcement		=	250		Cl 26.5.1.5,IS456:2000

Provide 8 mm dia 2 legged stirrups @ 190mm c/c spacing

Support 2

Negative moment		=	84	kNm
Hence the Section is designed as Doubly Reinforced Section				
	M_u/bd^2	=	3.206	
Percentage of tension steel reqd	p_t	=	0.028	%
Percentage of compression steel reqd	p_c	=	0	%
Required Area of tension steel	$A_{st\ reqd}$	=	131.96	mm^2
Required Area of compression steel	$A_{sc\ reqd}$	=	0	mm^2

Detail of bars

	I st Layer Nos	I st Layer Dia	II nd Layer Nos	II nd Layer Dia
Tension Reinforcement	3	16	0	0

A_{st} provided = 603.186 mm^2

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CL. NO.	BEAM DESIGN CALCULATIONS	REFERENCES

	Nos	Dia	Nos	Dia	
Compression Reinforceme	3	16	0	0	
A_{sc} provided	=	603.19	mm^2		

Percentage of tension steel provided	p_t	=	0.777	% **Hence O.K**
Percentage of compression steel provid	p_c	=	0.777	% **Hence O.K**

Check for Shear

Shear		=	149	kN	
Nominal Shear Stress	τv	=	1.92	N/mm^2	Cl 40.1, IS456:2000
Permissible Shear Stress	τc	=	0.92	N/mm^2	β = 1.000
Maximum Shear Stress	$\tau c,max$	=	2.5	N/mm^2	Table 19, IS456:2000
Diameter of the Stirrup bar		=	8	mm	Table 20, IS456:2000
Number of Legs		=	2		
Spacing of Shear reinforcement Required		=	190.0		Cl 40.4, IS456:2000
Maximum Spacing of Shear Reinforcement		=	250		Cl 26.5.1.5,IS456:2000

Provide 8 mm dia 2 legged stirrups @ 190mm c/c spacing

Span

Positive moment (Span)		=	84	kNm
Hence the Section is designed as Singly Reinforced Section				
	M_u/bd^2	=	3.325	
Percentage of tension steel reqd	p_t	=	0.029	%
Percentage of compression steel reqd	p_c	=	0	%
Required Area of tension steel	$A_{st\ reqd}$	=	131.96	mm^2
Required Area of compression steel	$A_{sc\ reqd}$	=	0	mm^2

Detail of bars

	I st Layer		II nd Layer	
	Nos	Dia	Nos	Dia
Tension Reinforcement	3	16	0	0
A_{st} provided	=	603.19	mm^2	

	Nos	Dia	Nos	Dia
Compression Reinforceme	3	16	0	0
A_{sc} provided	=	603.19	mm^2	

Percentage of tension steel provided	p_t	=	0.777	% **Hence O.K**
Percentage of compression steel provid	p_c	=	0.777	% **Hence O.K**

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CL. NO.	BEAM DESIGN CALCULATIONS				REFERENCES
	Check for Shear				
	Shear		=	149 kN	
	Nominal Shear Stress	τv	=	1.92 N/mm^2	Cl 40.1, IS456:2000
	Permissible Shear Stress	τc	=	0.92 N/mm^2	β = 1.000
	Maximum Shear Stress	$\tau c,max$	=	2.5 N/mm^2	Table 19, IS456:2000
	Diameter of the Stirrup bar		=	8 mm	Table 20, IS456:2000
	Number of Legs		=	2	
	Spacing of Shear reinforcement Required		=	190.0	Cl 40.4, IS456:2000
	Maximum Spacing of Shear Reinforcement		=	250	Cl 26.5.1.5,IS456:2000
	Provide 8 mm dia 2 legged stirrups @ 190mm c/c spacing				
	Check For Deflection				
	Basic Value		=	26	Cl.23.2, IS456:2000
	Service Stress in Steel	f_s	=	**63.44** N/mm^2	
	Modification Factor(Tension Reinforcement)	F_1	=	2.00	
	Allowable Span/Depth		=	52.00	
	Actual Span/Depth		=	13.33 **Hence Safe**	

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Floor Beam Design

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TITLE		**REVISION**	**DATE**	**CHECKED** RMK	**DATE**

CL. NO.	BEAM DESIGN CALCULATIONS					REFERENCES
1.1	**Geomentry Data**					
	Beam Number		=	B3		
	Type of Beam		=	Rectangular Beam		
	Support Condition		=	Continuous Beam		
	Width of beam	b	=	300	mm	
	Overall Depth of Beam	D	=	525	mm	
	Dia of bar	Φ	=	25	mm	
	Nominal Cover	cc	=	30	mm	IS 456:2000 Table 19
	Effective Depth	d	=	482.5	mm	IS 456:2000
	Effective Cover		=	42.5	mm	Cl 23.0
	Effective Span of beam	l_{eff}	=	3700	mm	
	Concrete grade		=	M 25		
	Steel Grade		=	Fe 500		
	$p_{t,lim}$		=	0.944 %		
	$p_{t,min}$		=	0.170 %		IS 456:2000
	p_t & p_{cmax}		=	4.00 %		Cl 26.5.1.1
1.2	**Force Input**					
	Left negative moment (Support 1)		=	254	kNm	
	Mid span positive moment		=	254	kNm	
	Right negative moment (Support 2)		=	254	kNm	
	Shear @ (Support 1)		=	241	kN	
	Shear @ (Span)		=	241	kN	
	Shear @ (Support 2)		=	241	kN	
1.3	**Limiting Moment**					
	M_{ulim} = 0.133 f_{ck} b d^2		=	232.22	kNm	
1.4	**Area of Steel**					
	Minimum Area of Stee = 0.17% b d		=	246.075	mm^2	
	Support 1					
	Negative moment (Support 1)		=	254	kNm	
	Hence the Section is designed as Doubly Reinforced Section					
		M_u/bd^2	=	3.637		
	Percentage of tension steel reqd	p_t	=	1.023	%	
	Percentage of compression steel reqd	p_c	=	0.085	%	
	Required Area of tension steel	$A_{st\ reqd}$	=	1480.79	mm^2	
	Required Area of compression steel	$A_{sc\ reqd}$	=	123.04	mm^2	

PROJECT	STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL	DOCUMENT. NO.	DATE	DESIGNED RMK	DATE
TITLE		REVISION	DATE	CHECKED RMK	DATE

CL. NO.	BEAM DESIGN CALCULATIONS	REFERENCES

Detail of bars

	I st Layer Nos	I st Layer Dia	II nd Layer Nos	II nd Layer Dia
Tension Reinforcement	3	20	3	20

A_{st} provided = 1884.96 mm^2

	Nos	Dia	Nos	Dia
Compression Reinforceme	3	20	3	20

A_{sc} provided = 1884.96 mm^2

Percentage of tension steel provided p_t = 1.302 % **Hence O.K**

Percentage of compression steel provid p_c = 1.302 % **Hence O.K**

Check for Shear

Shear = 241 kN

Nominal Shear Stress	τv =	1.66	N/mm^2	Cl 40.1, IS456:2000
Permissible Shear Stress	τc =	0.92	N/mm^2	β = 1.000
Maximum Shear Stress	$\tau c,max$ =	2.5	N/mm^2	Table 19, IS456:2000
Diameter of the Stirrup bar	=	8	mm	Table 20, IS456:2000
Number of Legs	=	2		
Spacing of Shear reinforcement Required	=	190.0		Cl 40.4, IS456:2000
Maximum Spacing of Shear Reinforcement	=	300		Cl 26.5.1.5,IS456:2000

Provide 8 mm dia 2 legged stirrups @ 190mm c/c spacing

Support 2

Negative moment = 254 kNm

Hence the Section is designed as Doubly Reinforced Section

	M_u/bd^2 =	3.637		
Percentage of tension steel reqd	p_t =	1.023	%	
Percentage of compression steel reqd	p_c =	0.085	%	
Required Area of tension steel	$A_{st\ reqd}$ =	1480.79	mm^2	
Required Area of compression steel	$A_{sc\ reqd}$ =	123.04	mm^2	

Detail of bars

	I st Layer Nos	I st Layer Dia	II nd Layer Nos	II nd Layer Dia
Tension Reinforcement	3	20	3	20

A_{st} provided = 1884.96 mm^2

	Nos	Dia	Nos	Dia
Compression Reinforceme	3	20	3	20

A_{sc} provided = 1884.96 mm^2

PROJECT	**STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL**	**DOCUMENT. NO.**	**DATE**	**DESIGNED** RMK	**DATE**
TITLE		**REVISION**	**DATE**	**CHECKED** RMK	**DATE**
CL. NO.	**BEAM DESIGN CALCULATIONS**				**REFERENCES**

Percentage of tension steel provided	p_t	=	1.302	%	**Hence O.K**	
Percentage of compression steel provid	p_c	=	1.302	%	**Hence O.K**	

Check for Shear

Shear		=	241	kN	
Nominal Shear Stress	τv	=	1.66	N/mm^2	Cl 40.1, IS456:2000
Permissible Shear Stress	τc	=	0.92	N/mm^2	β = 1.000
Maximum Shear Stress	$\tau c,max$	=	2.5	N/mm^2	Table 19, IS456:2000
Diameter of the Stirrup bar		=	8	mm	Table 20, IS456:2000
Number of Legs		=	2		
Spacing of Shear reinforcement Required		=	190.0		Cl 40.4, IS456:2000
Maximum Spacing of Shear Reinforcement		=	300		Cl 26.5.1.5,IS456:2000

Provide 8 mm dia 2 legged stirrups @ 190mm c/c spacing

Span

Positive moment (Span)		=	254	kNm	
Hence the Section is designed as Doubly Reinforced Section					
	M_u/bd^2	=	3.325		
Percentage of tension steel reqd	p_t	=	1.023	%	
Percentage of compression steel reqd	p_c	=	0.085	%	
Required Area of tension steel	$A_{st\ reqd}$	=	1480.79	mm^2	
Required Area of compression steel	$A_{sc\ reqd}$	=	123.04	mm^2	

Detail of bars

	I st Layer		II nd Layer	
	Nos	Dia	Nos	Dia
Tension Reinforcement	3	20	3	20

A_{st} provided = 1884.96 mm^2

	Nos	Dia	Nos	Dia
Compression Reinforceme	3	20	3	20

A_{sc} provided = 1884.96 mm^2

Percentage of tension steel provided	p_t	=	1.302	%	**Hence O.K**
Percentage of compression steel provid	p_c	=	1.302	%	**Hence O.K**

PROJECT	STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL	DOCUMENT. NO.	DATE	DESIGNED RMK	DATE
TITLE		REVISION	DATE	CHECKED RMK	DATE

CL. NO.	BEAM DESIGN CALCULATIONS				REFERENCES
	Check for Shear				
	Shear		=	241 kN	
	Nominal Shear Stress	τ_v	=	1.66 N/mm^2	Cl 40.1, IS456:2000
	Permissible Shear Stress	τ_c	=	0.92 N/mm^2	β = 1.000
	Maximum Shear Stress	$\tau_{c,max}$	=	2.5 N/mm^2	Table 19, IS456:2000
	Diameter of the Stirrup bar		=	8 mm	Table 20, IS456:2000
	Number of Legs		=	2	
	Spacing of Shear reinforcement Required		=	190.0	Cl 40.4, IS456:2000
	Maximum Spacing of Shear Reinforcement		=	300	Cl 26.5.1.5,IS456:2000
	Provide 8 mm dia 2 legged stirrups @ 190mm c/c spacing				
	Check For Deflection				
	Basic Value		=	26	Cl.23.2, IS456:2000
	Service Stress in Steel	f_s	=	**227.82** N/mm^2	
	Modification Factor(Tension Reinforcement)	F_1	=	0.97	
	Allowable Span/Depth		=	25.22	
	Actual Span/Depth		=	7.67 **Hence Safe**	



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Floor Slab Design

PROJECT	STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL	DOCUMENT.NO	DATE	DESIGNED	DATE
TITLE	TWO WAY SLAB DESIGN	REVISION	DATE	CHECKED	DATE

CL. NO.	DESIGN CALCULATIONS	REFERENCES

1.0 Design of Two Way Slab S3

1.0.1 Basic Input Data

Grade of concrete		=	**M 25**		
Grade of steel		=	**Fe 500**		
Thickness of slab	D	=	**125**	mm	
Clear cover	cc	=	**25**	mm	Table 16 Cl.26.4.2 , IS 456 -2000
Effective thickness of slab (d_{eff})	= 125 - 25 - 4	=	96	mm	
Unit weight of concrete		=	25	KN/m^3	Cl 19.2.1, IS 456-2000
Effective Shorter Span	L_x	=	**3.2**	m	
Effective Longer Span	L_y	=	**6.4**	m	
Aspect ratio	L_y/L_x	=	2.00 = 2		

Hence designed as Two way slab — Cl D-1.11, IS 456-2000

Edge Condition : **One Long edge Discontinuous**

1.0.2 Load Data

Self Weight of Slab	= 0.125 x 25	=	3.125	KN/m^2
Floor finish		=	**1.500**	KN/m^2
Partition Load		=	**1.500**	KN/m^2
Sunken Filling		=	**0.000**	KN/m^2
Imposed Load		=	**2.000**	KN/m^2 — IS 875 (Part 2) -1987
Extra Load, (if Any)		=	**0.000**	KN/m^2
Total load		=	8.125	KN/m^2
Design load (Wu) = 1.5 x 8.125		=	**12.188**	KN/m^2 — Table 18 Cl 36.4.1 IS 456 -2000

1.0.3 Moment and Shear Calculations

Short Span Direction

α_x for positive moment	1.75	→	0.059		Table 26 Cl D-1.1, IS 456 -2000
	2	→	0.065		
	for 2.00 α_x	=	0.065		
Positive moment M_x	0.065 x 12.19 x 3.2 ^2	=	**8.11**	KN-m	
α_x for negative moment	1.75	→	0.077		Table 26 Cl D-1.1, IS 456 -2000
	2	→	0.085		
	for 2.00 α_x	=	0.085		
Negative moment M_x :	0.085 x 12.19 x 3.2 ^2	=	**10.61**	KN-m	

PROJECT	STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL	DOCUMENT.NO	DATE	DESIGNED	DATE
TITLE	TWO WAY SLAB DESIGN	REVISION	DATE	CHECKED	DATE

CL. NO.	DESIGN CALCULATIONS	REFERENCES
	Long Span Direction	
	For positive moment α_y = 0.028	Table 26 Cl D-1.1,
	Positive moment M_y : 0.028 x 12.19 x 3.2 ^2 = **3.49** KN-m	IS 456 -2000
	For negative moment α_y = 0.037	Table 26 Cl D-1.1,
	Negative moment M_y : 0.037 x 12.19 x 3.2 ^2 = **4.62** KN-m	IS 456 -2000
	Shear force V_u = 12.19 x 3.2 x 0.5 = **19.50** KN	
1.0.4	**Check Effective Depth for Max.Bending Moment**	
	Depth required = $(M_x, (0.133x\ fck\ x\ b))^{1/2}$ = 56.48 mm	Table C,SP 16 :1980
	Depth provided = 125 - 25 - 8 /2 = 96 mm > 56.48 mm	Cl 2.3
	Hence the section is Safe against Bending	
1.0.5	**Check Depth for Shear**	
	Nominal Shear Stress : $\tau_v = V_u / b\ d$ = 0.20 N/mm^2	Cl.40.1, IS 456:2000
	Design shear strength (τ_c) = 0.42 N/mm^2	Table 19 , IS 456:2000
	k = 1.30	Cl.40.2.1.1,
	k * τ_c = 0.55 N/mm^2	IS 456:2000
	τ_v < k * τ_c **Hence the section is Safe against Shear**	
1.0.6	**Check for Maximum Shear Stress:**	
	Nominal Shear Stress : $\tau_v = V_u / b\ d$ = 0.20 N/mm^2	Cl.40.1, IS 456:2000
	Maximum Shear Stress ($\tau_{c,max}$) = 3.1 N/mm^2	Table 20 , IS 456:2000
	0.5 $\tau_{c,max}$ = 1.55 N/mm^2	
	τ_v < 0.5 $\tau_{c,max}$ **Hence the section is Safe against Max.Shear Stres**	Cl.40.2.3.1, IS 456:20
1.0.7	**Reinforcement calculations**	
	Short Span Direction @ Support	
	Mu/bd^2 = 10.61 x 10 ^ 6 / 1000 x 96 x 96 = **1.151**	
	Pt = **0.28** %	Table 2-4, SP16 :1980

PROJECT	**STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL**	**DOCUMENT.NO**	**DATE**	**DESIGNED**	**DATE**
TITLE	**TWO WAY SLAB DESIGN**	**REVISION**	**DATE**	**CHECKED**	**DATE**
CL. NO.	**DESIGN CALCULATIONS**			**REFERENCES**	

Area of steel Ast = 0.0028 x 1000 x 96 = 269.25 mm^2

Minimum area of steel 0.0012 x 1000 x 125 = 150.00 mm^2 Cl:26.5.2.1

Spacing of Y **8** bars 50.27 x 1000 / 269.25 = 186.59 mm IS 456 -2000

Check for Spacing

Minimum of

1) 3d = 288 mm

2) = 300 mm Cl:26.3.3.(b).1

3) = 186.59 mm IS 456 -2000

Therefore provide Y **8** @ **180** mm C/C

Short Span Direction @ Mid Span

$Mu/bd^2 = \frac{8.11 \times 10 \wedge 6}{1000 \times 96 \times 96} = 0.880$

Pt = 0.211 % SP16 :1980 Table 2-4

Area of steel Ast = 0.0021 x 1000 x 96 = 202.93 mm^2 IS 456:2000

Minimum area of steel 0.0012 x 1000 x 125 = 150.00 mm^2 Cl:26.5.2.1

Spacing of Y **8** bars 50.27 x 1000 / 202.93 = 247.57 mm

Check for Spacing

Minimum of

1) 3d = 288 mm IS 456:2000

2) = 300 mm Cl:26.3.3.(b).1

3) = 247.57 mm

Therefore provide Y **8** @ **150** mm C/C 150

Provided % of Tension Reinforcement (pt) = **0.349** %

Long Span Direction @ Support

$Mu/bd^2 = \frac{4.62 \times 10 \wedge 6}{1000 \times 96 \times 96} = 0.501$

Pt = 0.118 % SP16 :1980 Table 2-4

Area of steel Ast = 0.0012 x 1000 x 96 = 113.3 mm^2 IS 456:2000

Minimum area of steel 0.0012 x 1000 x 125 = 150.00 mm^2 Cl:26.5.2.1

Spacing of Y **8** bars 50.27 x 1000 / 150.00 = 334.93 mm

PROJECT	**STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL**	**DOCUMENT.NO**	**DATE**	**DESIGNED**	**DATE**
TITLE	**TWO WAY SLAB DESIGN**	**REVISION**	**DATE**	**CHECKED**	**DATE**

CL. NO.	DESIGN CALCULATIONS	REFERENCES

Check for Spacing

Minimum of
1) 3d = 288 mm — IS 456:2000
2) = 300 mm — Cl:26.3.3.(b).1
3) = 334.93 mm

Therefore provide Y **8** @ **280** mm C/C

Long Span Direction @ mid Span

$Mu/bd^2 = \frac{3.49 \times 10 \wedge 6}{1000 \times 96 \times 96} = 0.379$

Pt = 0.089 % — SP16 :1980 Table 2-4

Area of steel Ast = 0.0009 x 1000 x 96 = 85.233 mm^2 — IS 456:2000

Minimum area of steel 0.0012 x 1000 x 125 = 150.00 mm^2 — Cl:26.5.2.1

Spacing of Y **8** bars 50.27 x 1000 / 150.00 = 334.93 mm

Check for Spacing

Minimum of
1) 3d = 288 mm — IS 456:2000
2) = 300 mm — Cl:26.3.3.(b).1
3) = 334.93 mm

Therefore provide Y **8** @ **280** mm C/C

1.0.8 **Check for Deflection:** **(Simplified Approach)**

Basic Span/depth Ratio = **26** — IS 456:2000 Cl:23.2.1

% of Tension Reinforcement provided (p_t) = 0.349 %

Service stress of steel (fs) 0.58 fy (Ast_R/Ast_P) = 175.62 N/mm^2 — IS456:2000 Fig 4

Modification Factor for Tension Reinforcement (F_1 = 1.98 — Page 38

Allowable Span / Effective Depth Ratio = 51.51 — IS 456:2000 Cl:23.2.1

Actual Span/Eff Depth Rati = 3.2 x 1000 / 96 = 33.33

Hence Safe against Deflection

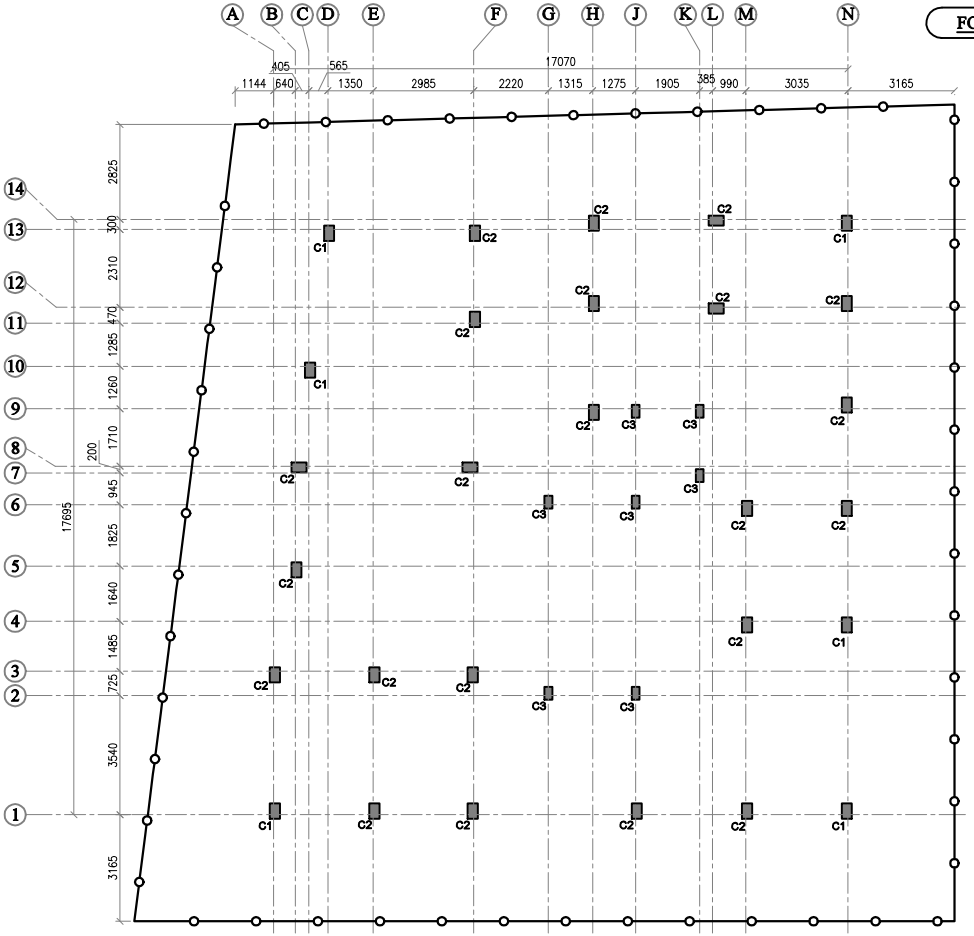
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APPENDIX II

GENERAL ARRANGEMENT DRAWINGS

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FOR APPROVAL ONLY

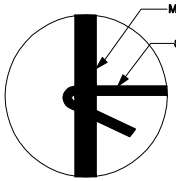


LAYOUT OF COLUMNS

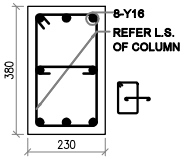
NOTE:

N.G.L - NATURAL GROUND LEVEL.

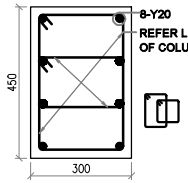
ALL HOOKS SHOULD BE BENT TO AN ANGLE OF 135° WITH A LENGTH NOT LESS THAN 75 MM



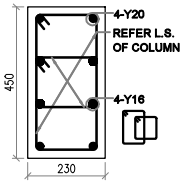
HOOK DETAIL



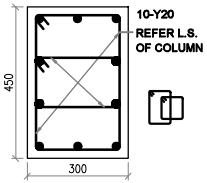
C.S. OF COLUMN "C3"
(230 x 380)
UP TO TOP



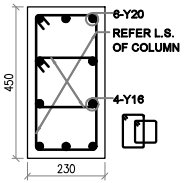
C.S. OF COLUMN "C1"
(300 x 450)
UP TO GFR



C.S. OF COLUMN "C1"
(230 x 450)
GFR TO TOP



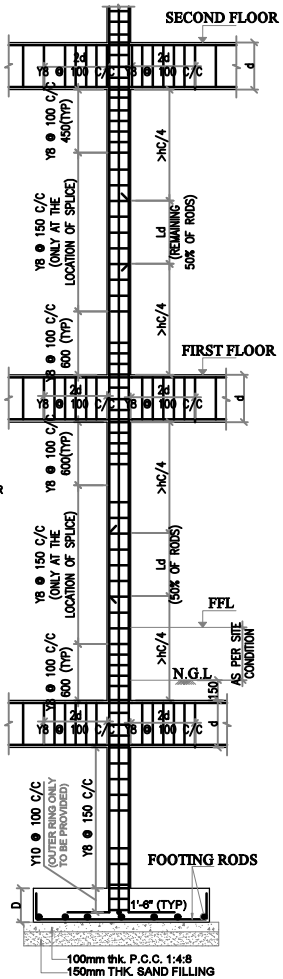
C.S. OF COLUMN "C2"
(300 x 450)
UP TO GFR



C.S. OF COLUMN "C2"
(230 x 450)
GFR TO TOP

COLUMN SCHEDULE

S.NO.	NAME	SIZE	
		UPTO STILT ROOF	STILT ROOF TO TOP
1.	C1	300 X 450	230 X 450
2.	C2	300 X 450	230 X 450
3.	C3	230 X 380	230 X 380



REINFORCEMENT DETAILING COLUMN

(ALTERNATE RODS TO BE LAPPED IN ALTERNATE FLOORS)

GENERAL NOTES:

REMOVAL OF FORMWORK(SHUTTERING) :

SHUTTERING OF ANY PART OF THE STRUCTURE SHALL NOT BE REMOVED UNTIL THE CONCRETE ATTAINS 70% THE SPECIFIED STRENGTH OF CONCRETE. HOWEVER THE FOLLOWING MINIMUM PERIOD (AFTER FINAL POUR) SHALL BE ALLOWED BEFORE REMOVAL OF SHUTTERING AS PER CLAUSE 11.3.1, IS 456:2000

A) VERTICAL SIDE OF SHUTTERING OF COLUMNS,WALLS

LAP JOINTS FOR REINFORCEMENT BARS :

I) AT ANY CROSS SECTION OF THE MEMBER,NOT MORE THAN 50% THE BARS SHALL BE LAPPED.

II) LAPS SHALL BE STAGGERED WITH A MINIMUM CENTRE TO CENTRE DISTANCE OF1.3 TIMES LAP LENGTH OF THE BAR FOR TENSION & COMPRESSION MEMBERS.

III) DEVELOPMENT LENGTH (Ld) SHALL BE AS PER (SP:34-1999) TABLE BELOW:

A) DEVELOPMENT LENGTH FOR COLUMN BARS -40ϕ
B) DEVELOPMENT LENGTH FOR BEAM BARS -50ϕ

IV) LAPS IN COLUMNS SHALL BE PROVIDED AT MID HEIGHT OF FLOOR AND NOT AT SLAB LEVEL .

VII) DETAILING OF REBARS SHALL CONFIRM TO IBC

EXCAVATION AND BACKFILL

I) BACKFILL FOR FOUNDATIONS AND PITS SHALL BE PLACED EVENLY LAYERS OF 300 MM THICKNESS AND SHALL BE WELL COMPACTED. THE FILL SHALL CONTAIN NO STONES WITH A DIAMETER GREATER THAN 100 MM. THE DEGREE OF COMPACTION OF BACKFILL SHALL BE 95% OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT OBTAINED BY THE MODIFIED PROCTOR COMPACTION TEST.

II) UNSUITABLE MATERIALS LIKE ROCKS/ EXISTING FOOTINGS ARE DETECTED BELOW FOUNDATION LEVEL IN AREAS OF CUT SHALL BE EXCAVATED TO FIRM GROUND. THE RESULTANT EXCAVATION SHALL BE FILLED TO THE REQUIRED LEVELS WITH MASS CONCRETE GRADE M10. ALSO WHEREVER EXCAVATION OCCURS BELOW THE FOUNDATION LEVEL STATED ON THE DESIGN DRAWING THE RESULTING VOID SHALL BE BACK FILLED WITH MASS CONCRETE GRADE-10.

III) BEFORE CASTING FOUNDATIONS, ANY IMMEDIATE OR ADJACENT AREAS OF DISTURBED OR SOFT SOIL MUST BE REMOVED AND REPLACED WITH MASS CONCRETE OF GRADE 10

IV) A LAYER OF BLINDING CONCRETE 100 MM THICK SHALL BE PLACED UNDER ALL FOUNDATIONS. BLINDING CONCRETE SHALL HAVE A CHARACTERISTIC STRENGTH OF 10 N/MM AT 28 DAYS.

V) EXCAVATED FINE-GRAINED SOIL ENCOUNTERED AT SHALLOW DEPTHS SHALL BE IN NO CASE USED FOR BACK FILLING PURPOSES.

GENERAL NOTES:

1. ALL DIMENSIONS ARE IN MM.
2. CONCRETE GRADE SHALL BE M25 (1:1.5:3) CONFIRMING TO IS 456:2000
3. GRADE OF REINFORCEMENT MARKED THUS (Y) SHALL BE Fe 500 CONFIRMING TO IS 1786-2008
4. CLEAR COVER FOR REINFORCEMENT
   A) COLUMN = 40MM
5. THIS DRG. READ IN CONJECTION WITH ARCH.DRGS.
6. COLUMNS AND FOOTINGS ARE DESIGNED FOR S+5 FLOOR LOADS AND SBC OF THE SOIL IS 150KN/ SQM. @ 3000 BELOW N.G.L.
7. STRUCTURAL ENGINEER RESPONSIBILITY IS LIMITED TO DESIGN & ISSUAL OF STRUCTURAL DRAWING ONLY
8. ONUS OF CONSTRUCTION LIES WITH CONTRACTOR/OWNER

REV.	DESCRIPTIONS	DRN	CHK	DATE
00	ISSUE FOR APPROVAL	M.A	R.M	05.04.23

PROJECT :-

PLAN SHOWING THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

DRAWING TITLE :-

LAYOUT & R.C DETAILS OF COLUMN

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 130	——	001

ARCHITECT / CONSULTANTS :-

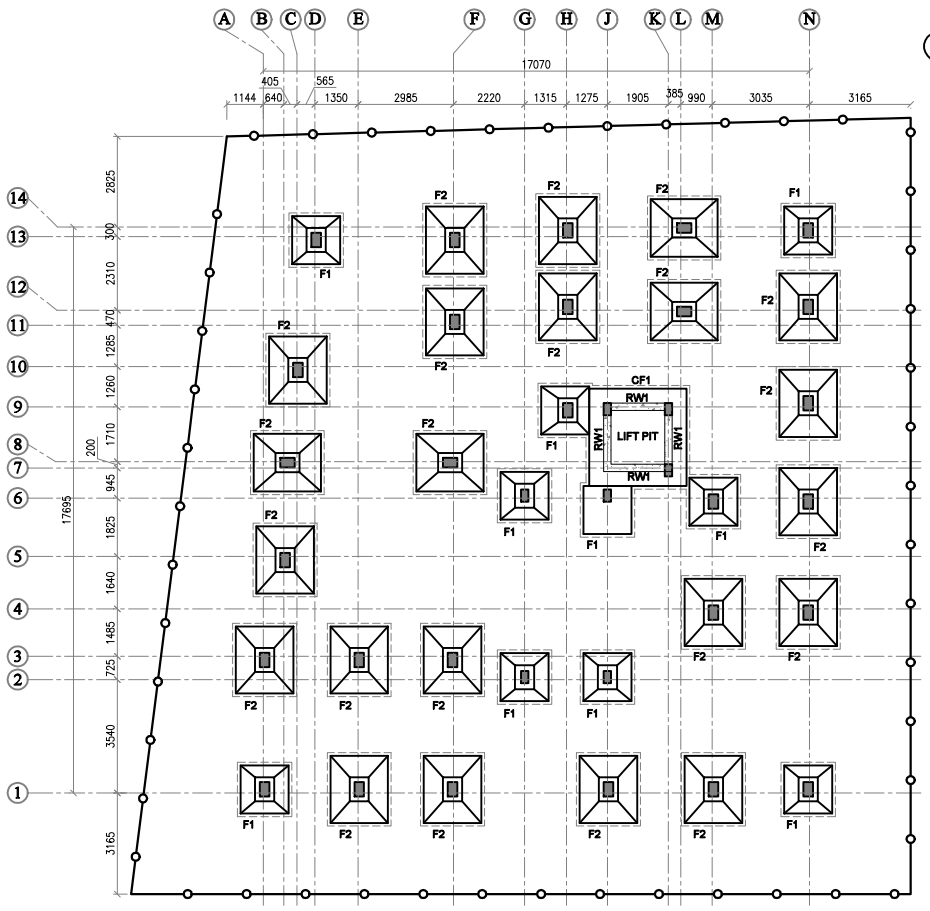
STRUCTURAL CONSULTANTS :-

AURA ENGINEERING CONSULTANTS
#8, 50th St, Saravanagalai Colony, 7th Avenue, ASHOK NAGAR, CHENNAI- 600 083.
Fixed Line :- 044 4859 7100
projects.auraengg@gmail.com
projects@auraengineers.co.in
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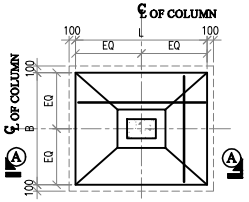
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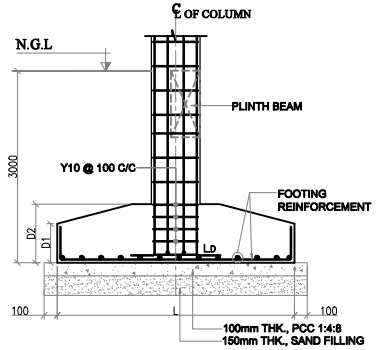
FOR APPROVAL ONLY



LAYOUT OF FOOTINGS



PLAN OF SLOPED FOOTING



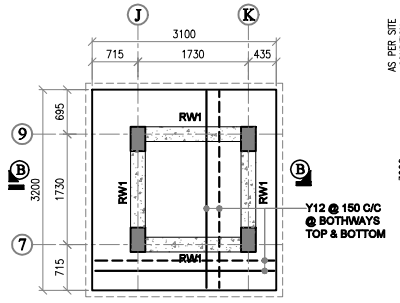
SECTION "A-A"

COMBINED FOOTINGS SCHEDULE:-

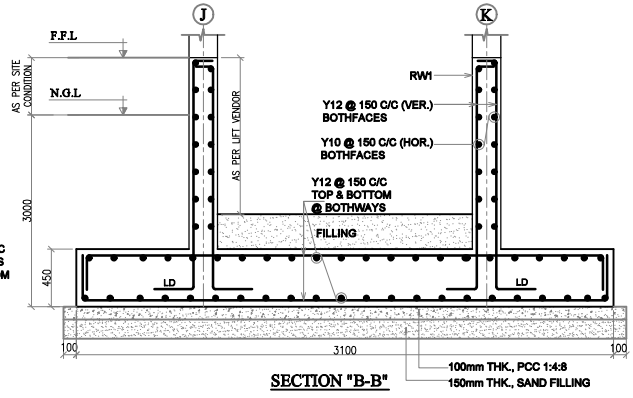
SI.	FOOTING NAME	FOOTING SIZE	FOOTING REINFORCEMENT
01	CF1	3200 X 3000 D=450	Y12 @ 150 C/C @ BOTHWAYS TOP & BOTTOM

FOOTINGS SCHEDULE:-

SI.	FOOTING NAME	FOOTING SIZE	FOOTING REINFORCEMENT
01	F1	1500 X 1500 SLOPED FOOTING D1=250 , D2=450	Y10 @ 150 C/C BOTHWAYS @ BOTTOM
02	F2	1800 X 2100 SLOPED FOOTING D1=300 , D2=550	Y12 @ 150 C/C BOTHWAYS @ BOTTOM



PLAN OF COMBINED FOOTING



SECTION "B-B"

GENERAL NOTES:

REMOVAL OF FORMWORK(SHUTTERING) :

SHUTTERING OF ANY PART OF THE STRUCTURE SHALL NOT BE REMOVED UNTIL THE CONCRETE ATTAINS 70% THE SPECIFIED STRENGTH OF CONCRETE. HOWEVER THE FOLLOWING MINIMUM PERIOD (AFTER FINAL POUR) SHALL BE ALLOWED BEFORE REMOVAL OF SHUTTERING AS PER CLAUSE 11.3.1, IS 456:2000

A) VERTICAL SIDE OF SHUTTERING OF COLUMNS,WALLS

LAP JOINTS FOR REINFORCEMENT BARS :

I) AT ANY CROSS SECTION OF THE MEMBER,NOT MORE THAN 50% THE BARS SHALL BE LAPPED.

II) LAPS SHALL BE STAGGERED WITH A MINIMUM CENTRE TO CENTRE DISTANCE OF1.3 TIMES LAP LENGTH OF THE BAR FOR TENSION & COMPRESSION MEMBERS.

III) DEVELOPMENT LENGTH (Ld) SHALL BE AS PER (SP:34-1999) TABLE BELOW:

A) DEVELOPMENT LENGTH FOR COLUMN BARS -40D
B) DEVELOPMENT LENGTH FOR BEAM BARS -50D

IV) LAPS IN COLUMNS SHALL BE PROVIDED AT MID HEIGHT OF FLOOR AND NOT AT SLAB LEVEL .

VII) DETAILING OF REBARS SHALL CONFIRM TO IBC

EXCAVATION AND BACKFILL

I) BACKFILL FOR FOUNDATIONS AND PITS SHALL BE PLACED EVENLY LAYERS OF 300 MM THICKNESS AND SHALL BE WELL COMPACTED. THE FILL SHALL CONTAIN NO STONES WITH A DIAMETER GREATER THAN 100 MM. THE DEGREE OF COMPACTION OF BACKFILL SHALL BE 95% OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT OBTAINED BY THE MODIFIED PROCTOR COMPACTION TEST.

II) UNSUITABLE MATERIALS LIKE ROCKS/ EXISTING FOOTINGS ARE DETECTED BELOW FOUNDATION LEVEL IN AREAS OF CUT SHALL BE EXCAVATED TO FIRM GROUND. THE RESULTANT EXCAVATION SHALL BE FILLED TO THE REQUIRED LEVELS WITH MASS CONCRETE GRADE M10. ALSO WHEREVER EXCAVATION OCCURS BELOW THE FOUNDATION LEVEL STATED ON THE DESIGN DRAWING THE RESULTING VOID SHALL BE BACK FILLED WITH MASS CONCRETE GRADE-10.

III) BEFORE CASTING FOUNDATIONS, ANY IMMEDIATE OR ADJACENT AREAS OF DISTURBED OR SOFT SOIL MUST BE REMOVED AND REPLACED WITH MASS CONCRETE OF GRADE 10

IV) A LAYER OF BLINDING CONCRETE 100 MM THICK SHALL BE PLACED UNDER ALL FOUNDATIONS. BLINDING CONCRETE SHALL HAVE A CHARACTERISTIC STRENGTH OF 10 N/MM AT 28 DAYS.

V) EXCAVATED FINE-GRAINED SOIL ENCOUNTERED AT SHALLOW DEPTHS SHALL BE IN NO CASE USED FOR BACK FILLING PURPOSES.

GENERAL NOTES:

1. ALL DIMENSIONS ARE IN MM.
2. CONCRETE GRADE SHALL BE M25 (1:1.5:3) CONFIRMING TO IS 456:2000
3. GRADE OF REINFORCEMENT MARKED THUS (Y) SHALL BE Fe 500 CONFIRMING TO IS 1786-2008
4. CLEAR COVER FOR REINFORCEMENT
   A) FOOTING = 50MM
5. THIS DRG. READ IN CONJECTION WITH ARCH.DRGS.
6. COLUMNS AND FOOTINGS ARE DESIGNED FOR S+5 FLOOR LOADS AND SBC OF THE SOIL IS 150KN/ SQM. @ 3000 BELOW N.G.L.
7. STRUCTURAL ENGINEER RESPONSIBILITY IS LIMITED TO DESIGN & ISSUAL OF STRUCTURAL DRAWING ONLY
8. ONUS OF CONSTRUCTION LIES WITH CONTRACTOR/OWNER

REV.	DESCRIPTIONS	DRN	CHK	DATE
00	ISSUE FOR APPROVAL	M.A	R.M	05.04.23

PROJECT :-

PLAN SHOWING THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

DRAWING TITLE :-

LAYOUT & R.C DETAILS OF FOOTING

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 130	———	002

ARCHITECT / CONSULTANTS :-

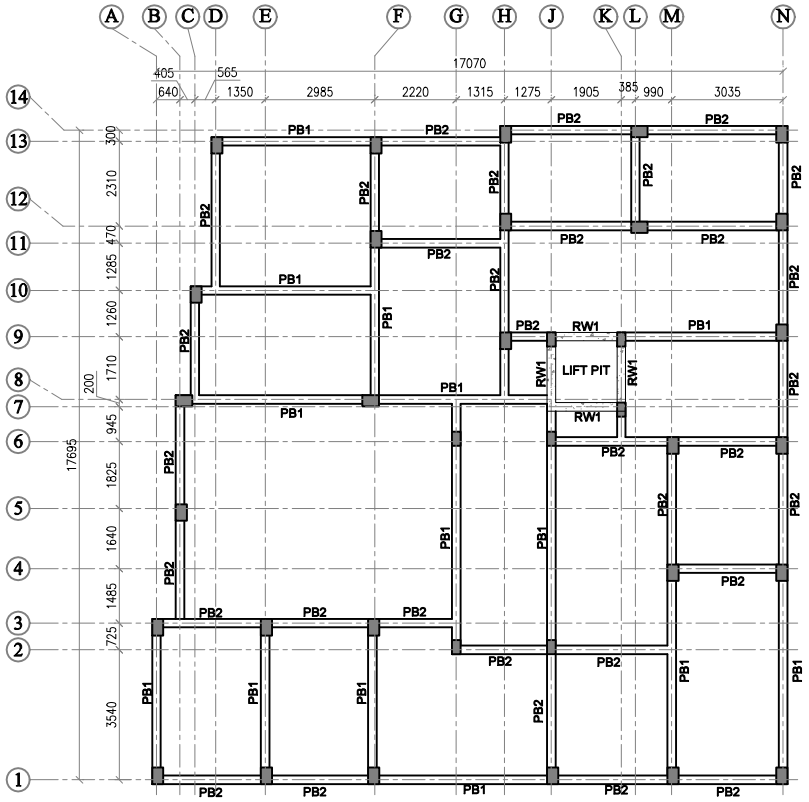
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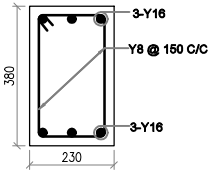
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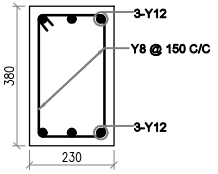
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LAYOUT OF PLINTH BEAM

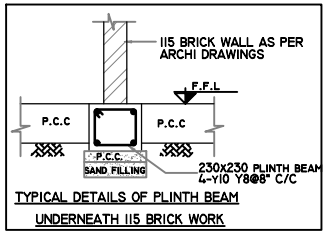


C.S. OF BEAM "PB1"
(230 x 380)

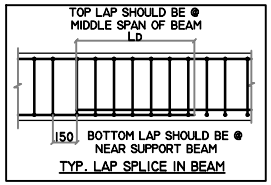


C.S. OF BEAM "PB2"
(230 x 380)

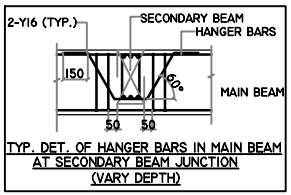
BEAM SCHEDULE		
SL.NO	NAME	SIZE
01	PB1	230 x 380
02	PB2	230 x 380



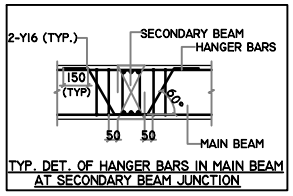
TYPICAL DETAILS OF PLINTH BEAM
UNDERNEATH 115 BRICK WORK



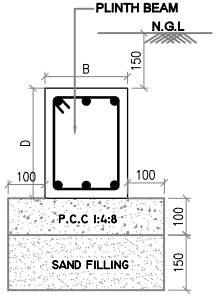
TYP. LAP SPLICE IN BEAM



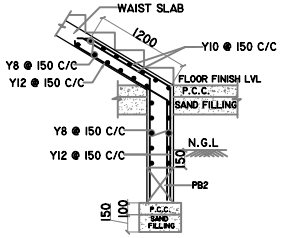
TYP. DET. OF HANGER BARS IN MAIN BEAM AT SECONDARY BEAM JUNCTION (VARY DEPTH)



TYP. DET. OF HANGER BARS IN MAIN BEAM AT SECONDARY BEAM JUNCTION



TYPICAL DETAIL OF PLINTH BEAM



TYP. DETAIL OF STAIRCASE DOWELS

GENERAL NOTES:

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REV.	DESCRIPTIONS	DRN	CHK	DATE
00	ISSUE FOR APPROVAL	M.A	R.M	05.04.23

PROJECT :-

PLAN SHOWING THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

DRAWING TITLE :-

LAYOUT & R.C DETAILS OF PLINTH BEAM

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 120	——	003

ARCHITECT / CONSULTANTS :-

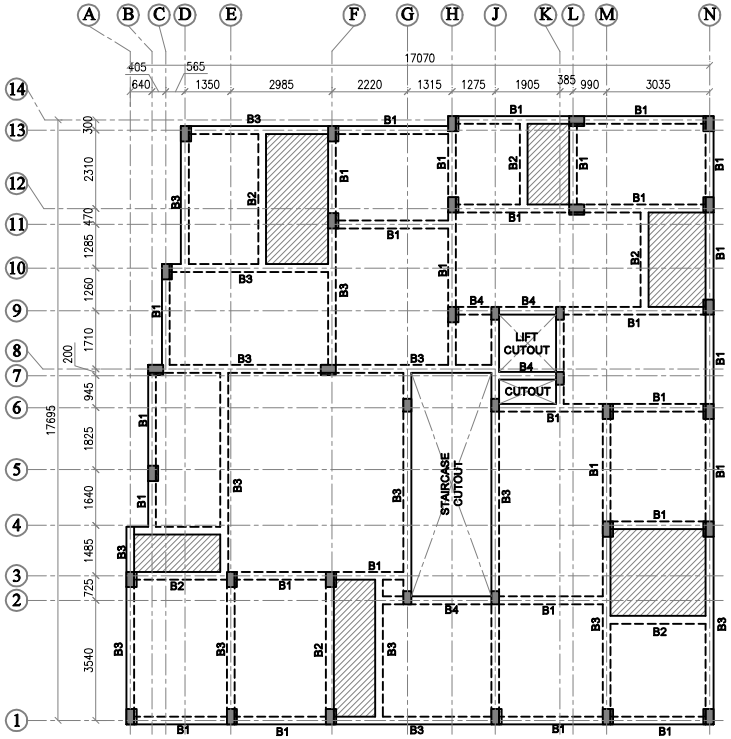
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A3

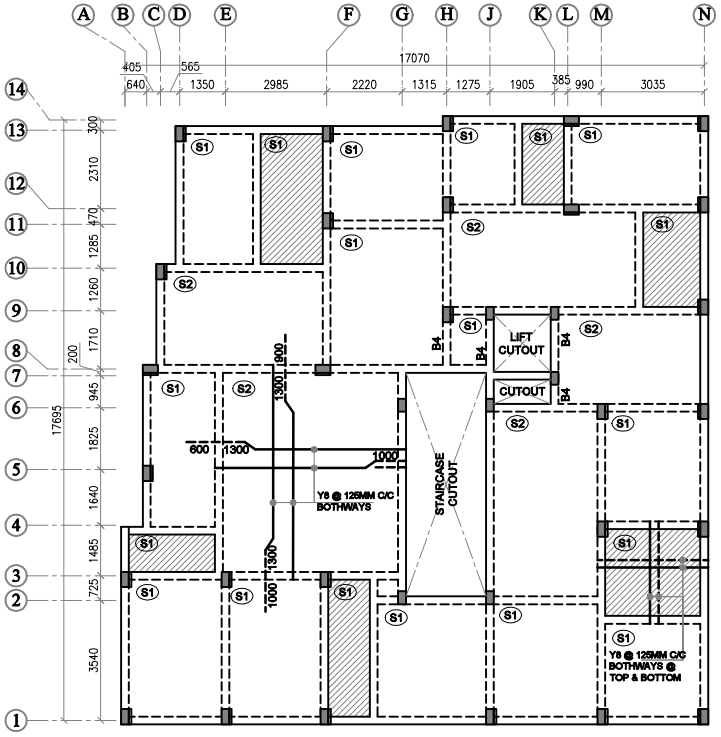
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NUMERATION LAYOUT OF STILT FLOOR

ROOF SLAB & BEAM



R.C LAYOUT OF STILT FLOOR

ROOF SLAB & BEAM

NOTE:

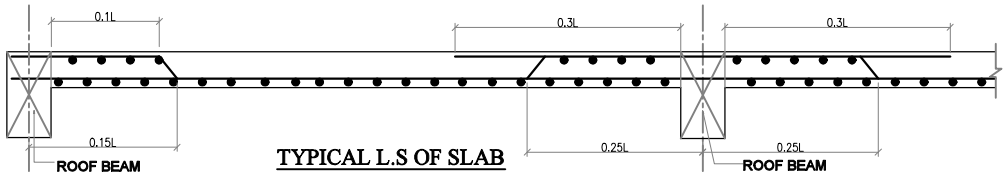
- SLAB SUNK BY 0'-10"
- PROVIDE Y8 @ 8" C/C AS DISTRIBUTORS WHERE EVER REQUIRED
- COLUMN CURTAILED TO BE BEND IN TO TOP OF THE SLAB
- FOR BEAM REINFORCEMENT DETAILS REFER DWG.NO : 005

BEAM SCHEDULE

SL.NO	NAME	SIZE
01	B1	230 x 450
02	B2	230 x 450
03	B3	230 x 530
04	B4	230 x 450

SLAB SCHEDULE

SL.NO	NAME	THICK
01	S1	150 THK.
02	S2	150 THK.



TYPICAL L.S OF SLAB

GENERAL NOTES:

REMOVAL OF FORMWORK(SHUTTERING) :

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iii) BEFORE CASTING FOUNDATIONS, ANY IMMEDIATE OR ADJACENT AREAS OF DISTURBED OR SOFT SOIL MUST BE REMOVED AND REPLACED WITH MASS CONCRETE OF GRADE 10

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GENERAL NOTES:

1. ALL DIMENSIONS ARE IN MM.
2. CONCRETE GRADE SHALL BE M25 (1:1.5:3) CONFIRMING TO IS 456:2000
3. GRADE OF REINFORCEMENT MARKED THUS (Y) SHALL BE Fe 500 CONFIRMING TO IS 1786-2008
4. CLEAR COVER FOR REINFORCEMENT
   A) BEAM = 30MM B) SLAB = 20MM
5. THIS DRG. READ IN CONJECTION WITH ARCH.DRGS.
6. STRUCTURAL ENGINEER RESPONSIBILITY IS LIMITED TO DESIGN & ISSUAL OF STRUCTURAL DRAWING ONLY
7. ONUS OF CONSTRUCTION LIES WITH CONTRACTOR/OWNER

REV.	DESCRIPTIONS	DRN	CHK	DATE
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DRAWING TITLE :-

NUMERATION LAYOUT & R.C LAYOUT OF STILT FLOOR ROOF SLAB & BEAMS

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 130	-------	004

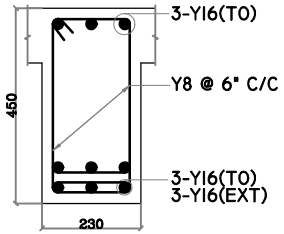
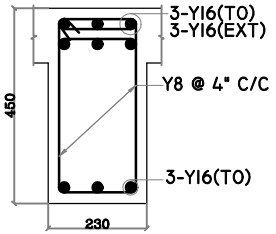
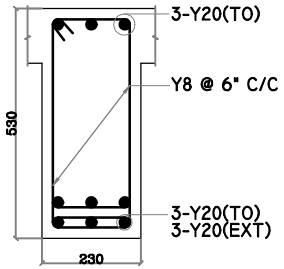
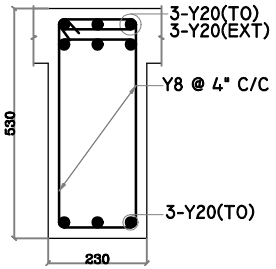
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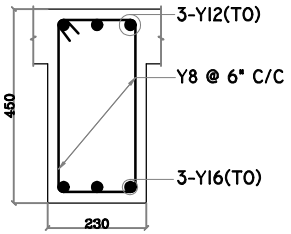
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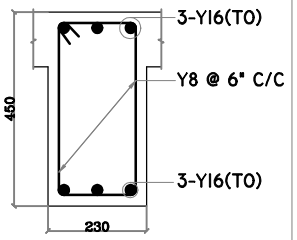
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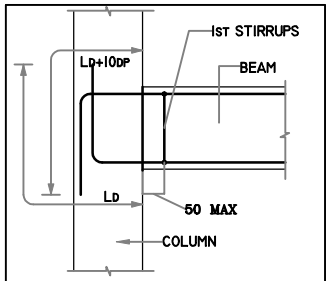
NAME	MID SPAN	END SUPPORT
B1 (230 X 450)		
B3 (230 X 530)		



C.S. OF BEAM "B2"
(230 X 450)



C.S. OF BEAM "B4"
(230 X 450)



TYP. EXTERNAL JOINT DETAIL AT BEAM-COLUMN JUNCTION

BEAM SCHEDULE

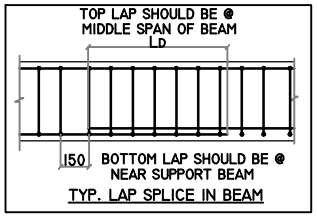
SL.NO	NAME	SIZE
01	B1	230 x 450
02	B2	230 x 450
03	B3	230 x 530
04	B4	230 x 450

GENERAL NOTES:

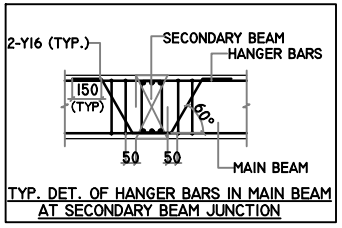
1. ALL DIMENSIONS ARE IN FOOT UNITS.
2. CONCRETE GRADE SHALL BE M25 CONFIRMING TO IS 456:2000
3. GRADE OF REINFORCEMENT MARKED THUS (Y) SHALL BE Fe 500 CONFIRMING TO IS 1786-2008
4. CLEAR COVER FOR REINFORCEMENT
   A) COLUMN = 1½" B) BEAM = 1"
   C) SLAB = ¾"
5. THIS DRG. READ IN CONJECTION WITH ARCH.DRGS.
6. STRUCTURAL ENGINEER RESPONSIBILITY IS LIMITED TO DESIGN & ISSUAL OF STRUCTURAL DRAWING ONLY
7. DEVELOPMENT LENGTH (Ld) SHALL BE AS PER (SP:34-1999) TABLE BELOW:

DEVELOPMENT LENGTH FOR COLUMNS	= 40D
DEVELOPMENT LENGTH FOR BEAMS	= 50D

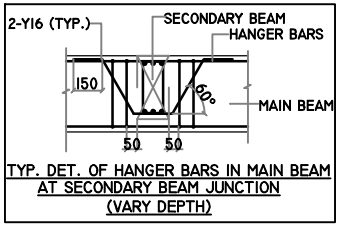
8. DETAILING OF REBARS SHALL CONFIRM TO SP-34 & IS 13920.



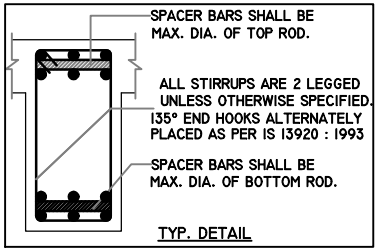
TYP. LAP SPLICE IN BEAM



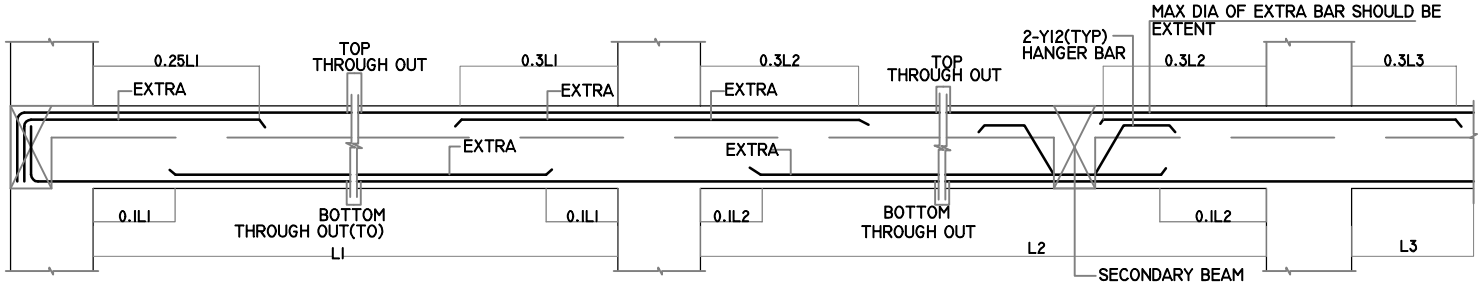
TYP. DET. OF HANGER BARS IN MAIN BEAM AT SECONDARY BEAM JUNCTION



TYP. DET. OF HANGER BARS IN MAIN BEAM AT SECONDARY BEAM JUNCTION (VARY DEPTH)



TYP. DETAIL



TYPICAL L.S OF ROOF BEAM

REV.	DESCRIPTIONS	DRN	CHK	DATE
00	ISSUE FOR APPROVAL	M.A	R.M	05.04.23

PROJECT :-

PLAN SHOWING THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

DRAWING TITLE :-

R.C. DETAILS OF STILT FLOOR ROOF SLABS & BEAMS

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 130	——	005

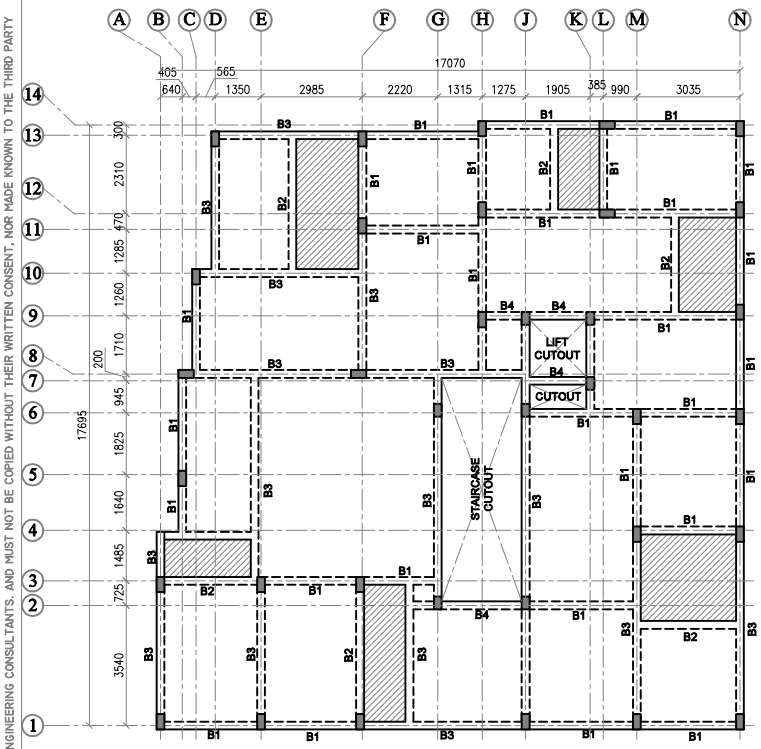
ARCHITECT / CONSULTANTS :-

STRUCTURAL CONSULTANTS :-

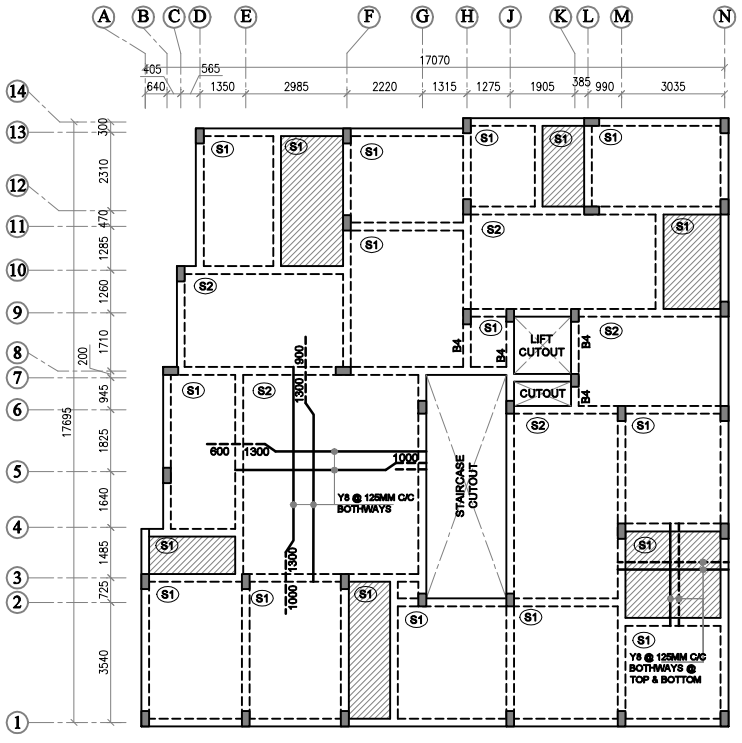
AURA ENGINEERING CONSULTANTS
#8, 80th St, Saravanangala Colony, 7th Avenue, ASHOK NAGAR, CHENNAI- 600 083.
Fixed Line :- 044 4859 7100
projects.auraengg@gmail.com
projects@auraengineers.co.in
http://auraengineers.co.in/
aura Engineering Consultants

A3

FOR APPROVAL ONLY



NUMERATION LAYOUT OF TYPICAL FLOOR
ROOF SLAB & BEAM



R.C LAYOUT OF TYPICAL FLOOR
ROOF SLAB & BEAM

NOTE:

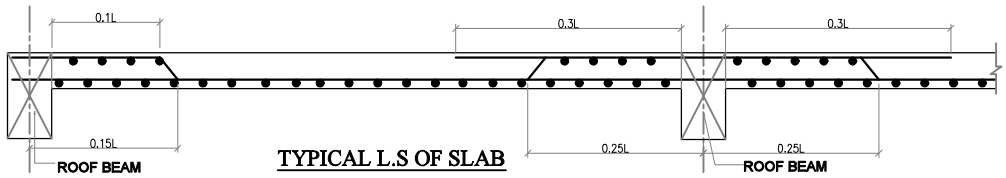
SLAB SUNK BY 0'-10"

PROVIDE Y8 @ 8" C/C AS DISTRIBUTORS WHERE EVER REQUIRED

FOR BEAM REINFORCEMENT DETAILS REFER DWG.NO : 007

BEAM SCHEDULE		
SL.NO	NAME	SIZE
01	B1	230 x 450
02	B2	230 x 450
03	B3	230 x 530
04	B4	230 x 450

SLAB SCHEDULE		
SL.NO	NAME	THICK
01	S1	150 THK.
02	S2	150 THK.



TYPICAL L.S OF SLAB

GENERAL NOTES:

REMOVAL OF FORMWORK(SHUTTERING) :

SHUTTERING OF ANY PART OF THE STRUCTURE SHALL NOT BE REMOVED UNTIL THE CONCRETE ATTAINS 70% THE SPECIFIED STRENGTH OF CONCRETE. HOWEVER THE FOLLOWING MINIMUM PERIOD (AFTER FINAL POUR) SHALL BE ALLOWED BEFORE REMOVAL OF SHUTTERING AS PER CLAUSE 11.3.1, IS 456:2000

A) VERTICAL SIDE OF SHUTTERING OF COLUMNS,WALLS

LAP JOINTS FOR REINFORCEMENT BARS :

i) AT ANY CROSS SECTION OF THE MEMBER,NOT MORE THAN 50% THE BARS SHALL BE LAPPED.

ii) LAPS SHALL BE STAGGERED WITH A MINIMUM CENTRE TO CENTRE DISTANCE OF1.3 TIMES LAP LENGTH OF THE BAR FOR TENSION & COMPRESSION MEMBERS.

iii) DEVELOPMENT LENGTH (Ld) SHALL BE AS PER (SP:34-1999) TABLE BELOW:

A) DEVELOPMENT LENGTH FOR COLUMN BARS -40D
B) DEVELOPMENT LENGTH FOR BEAM BARS -50D

iv) LAPS IN COLUMNS SHALL BE PROVIDED AT MID HEIGHT OF FLOOR AND NOT AT SLAB LEVEL .

vii) DETAILING OF REBARS SHALL CONFIRM TO IBC

EXCAVATION AND BACKFILL

I) BACKFILL FOR FOUNDATIONS AND PITS SHALL BE PLACED EVENLY LAYERS OF 300 mm THICKNESS AND SHALL BE WELL COMPACTED. THE FILL SHALL CONTAIN NO STONES WITH A DIAMETER GREATER THAN 100 mm. THE DEGREE OF COMPACTION OF BACKFILL SHALL BE 95% OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT OBTAINED BY THE MODIFIED PROCTOR COMPACTION TEST.

II) UNSUITABLE MATERIALS LIKE ROCKS/ EXISTING FOOTINGS ARE DETECTED BELOW FOUNDATION LEVEL IN AREAS OF CUT SHALL BE EXCAVATED TO FIRM GROUND. THE RESULTANT EXCAVATION SHALL BE FILLED TO THE REQUIRED LEVELS WITH MASS CONCRETE GRADE M10. ALSO WHEREVER EXCAVATION OCCURS BELOW THE FOUNDATION LEVEL STATED ON THE DESIGN DRAWING THE RESULTING VOID SHALL BE BACK FILLED WITH MASS CONCRETE GRADE-10.

III) BEFORE CASTING FOUNDATIONS, ANY IMMEDIATE OR ADJACENT AREAS OF DISTURBED OR SOFT SOIL MUST BE REMOVED AND REPLACED WITH MASS CONCRETE OF GRADE 10

IV) A LAYER OF BLINDING CONCRETE 100 mm THICK SHALL BE PLACED UNDER ALL FOUNDATIONS. BLINDING CONCRETE SHALL HAVE A CHARACTERISTIC STRENGTH OF 10 N/mm AT 28 DAYS.

V) EXCAVATED FINE-GRAINED SOIL ENCOUNTERED AT SHALLOW DEPTHS SHALL BE IN NO CASE USED FOR BACK FILLING PURPOSES.

GENERAL NOTES:

1. ALL DIMENSIONS ARE IN MM.
2. CONCRETE GRADE SHALL BE M25 (1:1.5:3) CONFIRMING TO IS 456:2000
3. GRADE OF REINFORCEMENT MARKED THUS (Y) SHALL BE Fe 500 CONFIRMING TO IS 1786-2008
4. CLEAR COVER FOR REINFORCEMENT
   A) BEAM = 30MM B) SLAB = 20MM
5. THIS DRG. READ IN CONJECTION WITH ARCH.DRGS.
6. STRUCTURAL ENGINEER RESPONSIBILITY IS LIMITED TO DESIGN & ISSUAL OF STRUCTURAL DRAWING ONLY
7. ONUS OF CONSTRUCTION LIES WITH CONTRACTOR/OWNER

REV.	DESCRIPTIONS	DRN	CHK	DATE
00	ISSUE FOR APPROVAL	M.A	R.M	05.04.23

PROJECT :-

PLAN SHOWING THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

DRAWING TITLE :-

NUMERATION LAYOUT & R.C LAYOUT OF TYPICAL FLOOR ROOF SLAB & BEAMS

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 130	-------	006

ARCHITECT / CONSULTANTS :-

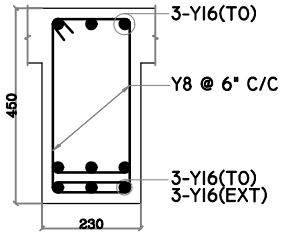
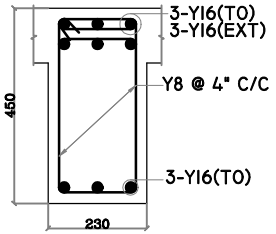
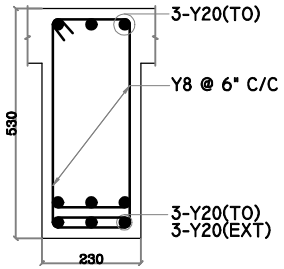
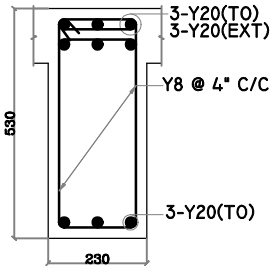
STRUCTURAL CONSULTANTS :-

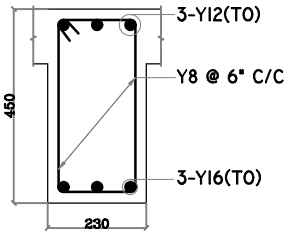
AURA ENGINEERING CONSULTANTS
#8, 50th St, Sarvamangala Colony, 7th Avenue, ASHOK NAGAR, CHENNAI- 600 083.
Fixed Line :- 044 4859 7100
projects.auraengg@gmail.com
projects@auraengineers.co.in
http://auraengineers.co.in/
aura Engineering Consultants

A3

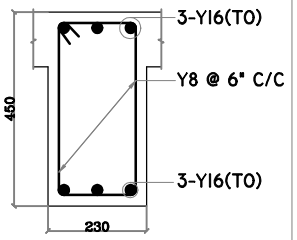
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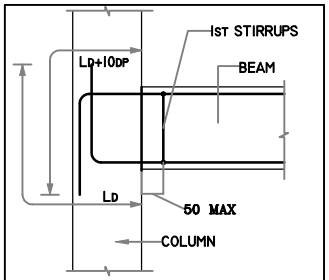
NAME	MID SPAN	END SUPPORT
B1 (230 X 450)		
B3 (230 X 530)		



C.S. OF BEAM "B2"
(230 X 450)



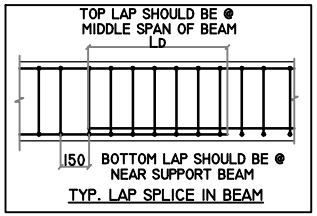
C.S. OF BEAM "B4"
(230 X 450)



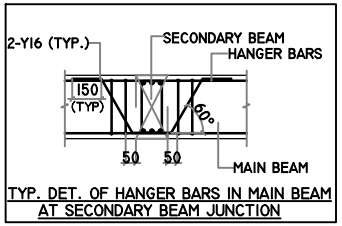
TYP. EXTERNAL JOINT DETAIL AT BEAM-COLUMN JUNCTION

BEAM SCHEDULE

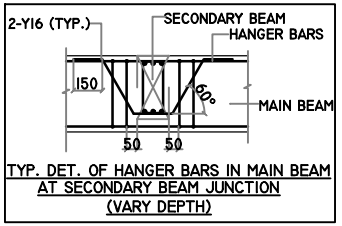
SL.NO	NAME	SIZE
01	B1	230 x 450
02	B2	230 x 450
03	B3	230 x 530
04	B4	230 x 450



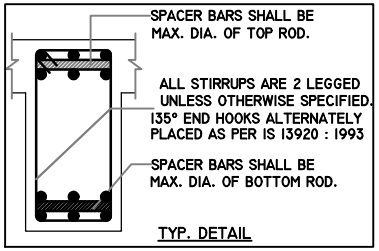
TYP. LAP SPLICE IN BEAM



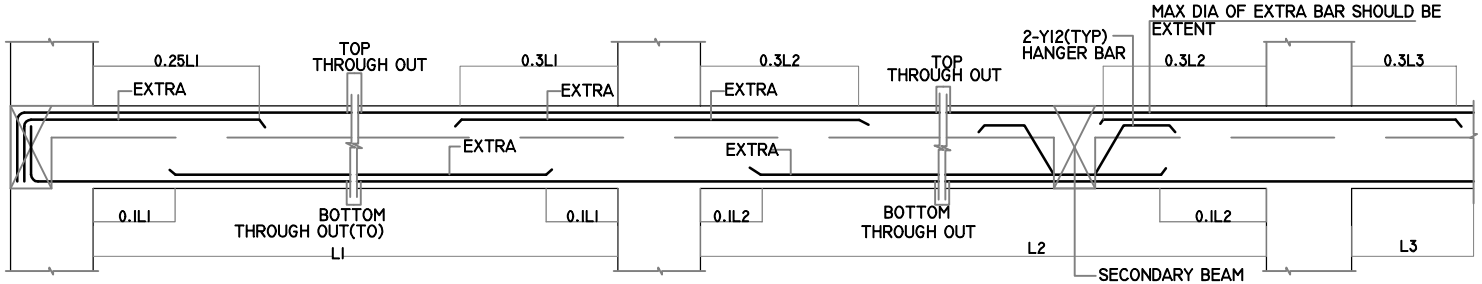
TYP. DET. OF HANGER BARS IN MAIN BEAM AT SECONDARY BEAM JUNCTION



TYP. DET. OF HANGER BARS IN MAIN BEAM AT SECONDARY BEAM JUNCTION (VARY DEPTH)



TYP. DETAIL



TYPICAL L.S OF ROOF BEAM

GENERAL NOTES:

1. ALL DIMENSIONS ARE IN FOOT UNITS.
2. CONCRETE GRADE SHALL BE M25 CONFIRMING TO IS 456:2000
3. GRADE OF REINFORCEMENT MARKED THUS (Y) SHALL BE Fe 500 CONFIRMING TO IS 1786-2008
4. CLEAR COVER FOR REINFORCEMENT
   A) COLUMN = 1½" B) BEAM = 1"
   C) SLAB = ¾"
5. THIS DRG. READ IN CONJECTION WITH ARCH.DRGS.
6. STRUCTURAL ENGINEER RESPONSIBILITY IS LIMITED TO DESIGN & ISSUAL OF STRUCTURAL DRAWING ONLY
7. DEVELOPMENT LENGTH (LD) SHALL BE AS PER (SP:34-1999) TABLE BELOW:

DEVELOPMENT LENGTH FOR COLUMNS	= 40D
DEVELOPMENT LENGTH FOR BEAMS	= 50D

8. DETAILING OF REBARS SHALL CONFIRM TO SP-34 & IS 13920.

REV.	DESCRIPTIONS	DRN	CHK	DATE
00	ISSUE FOR APPROVAL	M.A	R.M	05.04.23

PROJECT :-

PLAN SHOWING THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

DRAWING TITLE :-

R.C. DETAILS OF TYPICAL FLOOR ROOF SLABS & BEAMS

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 130	——	007

ARCHITECT / CONSULTANTS :-

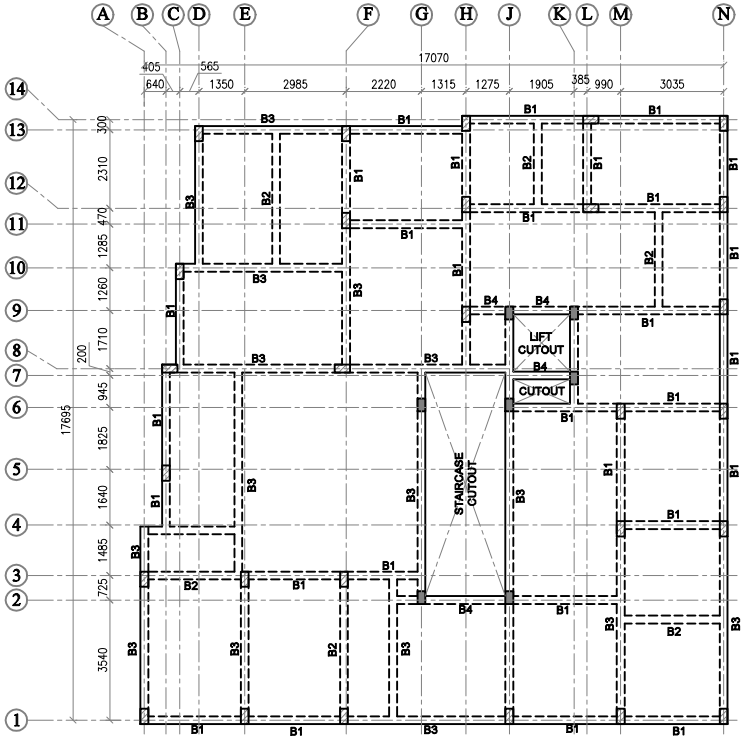
STRUCTURAL CONSULTANTS :-

AURA ENGINEERING CONSULTANTS
#8, 80th St, Saravanangala Colony, 7th Avenue, ASHOK NAGAR, CHENNAI- 600 083.
Fixed Line :- 044 4889 7100
projects.auraengg@gmail.com
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aura Engineering Consultants

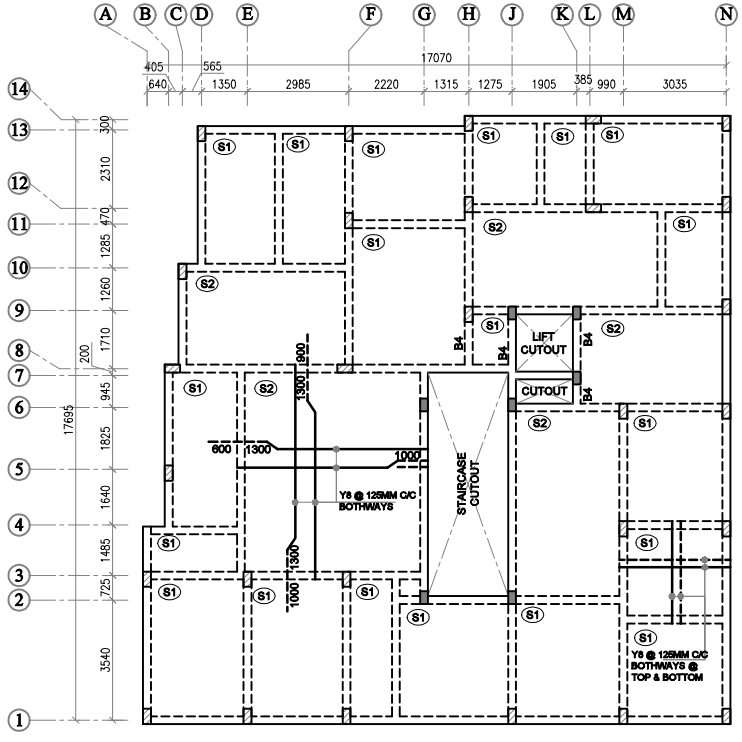
A3

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FOR APPROVAL ONLY



NUMERATION LAYOUT OF TERRACE FLOOR SLAB & BEAM



R.C LAYOUT OF TERRACE FLOOR SLAB & BEAM

NOTE:

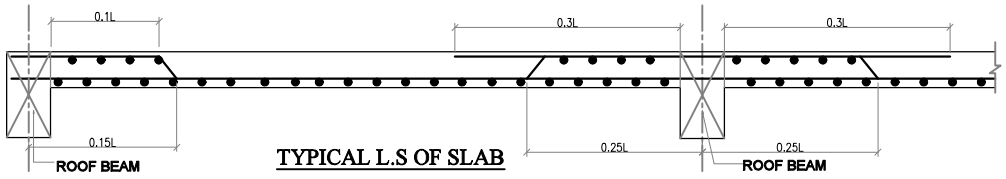
- PROVIDE Y8 @ 8" C/C AS DISTRIBUTORS WHERE EVER REQUIRED
- FOR BEAM REINFORCEMENT DETAILS REFER DWG.NO : 009
- CC - COLUMN CURTAILED & RODS TO BE BENT IN TO TOP OF THE SLAB

BEAM SCHEDULE

SL.NO	NAME	SIZE
01	B1	230 x 450
02	B2	230 x 450
03	B3	230 x 530
04	B4	230 x 450

SLAB SCHEDULE

SL.NO	NAME	THICK
01	S1	150 THK.
02	S2	150 THK.



TYPICAL L.S OF SLAB

GENERAL NOTES:

REMOVAL OF FORMWORK(SHUTTERING) :

SHUTTERING OF ANY PART OF THE STRUCTURE SHALL NOT BE REMOVED UNTIL THE CONCRETE ATTAINS 70% THE SPECIFIED STRENGTH OF CONCRETE. HOWEVER THE FOLLOWING MINIMUM PERIOD (AFTER FINAL POUR) SHALL BE ALLOWED BEFORE REMOVAL OF SHUTTERING AS PER CLAUSE 11.3.1, IS 456:2000

A) VERTICAL SIDE OF SHUTTERING OF COLUMNS,WALLS

LAP JOINTS FOR REINFORCEMENT BARS :

I) AT ANY CROSS SECTION OF THE MEMBER,NOT MORE THAN 50% THE BARS SHALL BE LAPPED.

II) LAPS SHALL BE STAGGERED WITH A MINIMUM CENTRE TO CENTRE DISTANCE OF1.3 TIMES LAP LENGTH OF THE BAR FOR TENSION & COMPRESSION MEMBERS.

III) DEVELOPMENT LENGTH (Ld) SHALL BE AS PER (SP:34-1999) TABLE BELOW:

A) DEVELOPMENT LENGTH FOR COLUMN BARS -40D
B) DEVELOPMENT LENGTH FOR BEAM BARS -50D

IV) LAPS IN COLUMNS SHALL BE PROVIDED AT MID HEIGHT OF FLOOR AND NOT AT SLAB LEVEL .

VII) DETAILING OF REBARS SHALL CONFIRM TO IBC

EXCAVATION AND BACKFILL

I) BACKFILL FOR FOUNDATIONS AND PITS SHALL BE PLACED EVENLY LAYERS OF 300 mm THICKNESS AND SHALL BE WELL COMPACTED. THE FILL SHALL CONTAIN NO STONES WITH A DIAMETER GREATER THAN 100 mm. THE DEGREE OF COMPACTION OF BACKFILL SHALL BE 95% OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT OBTAINED BY THE MODIFIED PROCTOR COMPACTION TEST.

II) UNSUITABLE MATERIALS LIKE ROCKS/ EXISTING FOOTINGS ARE DETECTED BELOW FOUNDATION LEVEL IN AREAS OF CUT SHALL BE EXCAVATED TO FIRM GROUND. THE RESULTANT EXCAVATION SHALL BE FILLED TO THE REQUIRED LEVELS WITH MASS CONCRETE GRADE M10. ALSO WHEREVER EXCAVATION OCCURS BELOW THE FOUNDATION LEVEL STATED ON THE DESIGN DRAWING THE RESULTING VOID SHALL BE BACK FILLED WITH MASS CONCRETE GRADE-10.

III) BEFORE CASTING FOUNDATIONS, ANY IMMEDIATE OR ADJACENT AREAS OF DISTURBED OR SOFT SOIL MUST BE REMOVED AND REPLACED WITH MASS CONCRETE OF GRADE 10

IV) A LAYER OF BLINDING CONCRETE 100 mm THICK SHALL BE PLACED UNDER ALL FOUNDATIONS. BLINDING CONCRETE SHALL HAVE A CHARACTERISTIC STRENGTH OF 10 N/mm AT 28 DAYS.

V) EXCAVATED FINE-GRAINED SOIL ENCOUNTERED AT SHALLOW DEPTHS SHALL BE IN NO CASE USED FOR BACK FILLING PURPOSES.

GENERAL NOTES:

1. ALL DIMENSIONS ARE IN MM.
2. CONCRETE GRADE SHALL BE M25 (1:1.5:3) CONFIRMING TO IS 456:2000
3. GRADE OF REINFORCEMENT MARKED THUS (Y) SHALL BE Fe 500 CONFIRMING TO IS 1786-2008
4. CLEAR COVER FOR REINFORCEMENT
   A) BEAM = 30MM B) SLAB = 20MM
5. THIS DRG. READ IN CONJECTION WITH ARCH.DRGS.
6. STRUCTURAL ENGINEER RESPONSIBILITY IS LIMITED TO DESIGN & ISSUAL OF STRUCTURAL DRAWING ONLY
7. ONUS OF CONSTRUCTION LIES WITH CONTRACTOR/OWNER

REV.	DESCRIPTIONS	DRN	CHK	DATE
00	ISSUE FOR APPROVAL	M.A	R.M	05.04.23

PROJECT :-

PLAN SHOWING THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

DRAWING TITLE :-

NUMERATION LAYOUT & R.C LAYOUT OF TERRACE FLOOR SLAB & BEAMS

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 130	-------	008

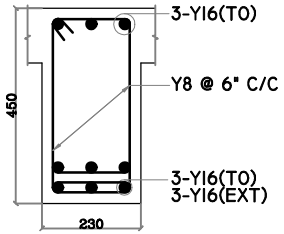
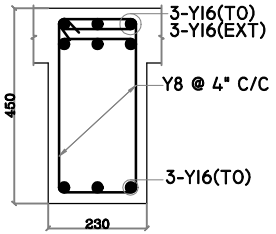
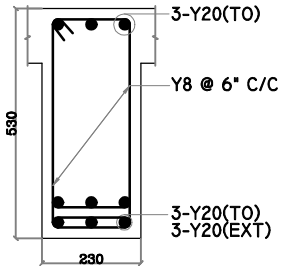
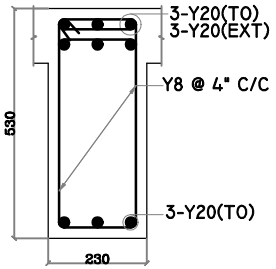
ARCHITECT / CONSULTANTS :-

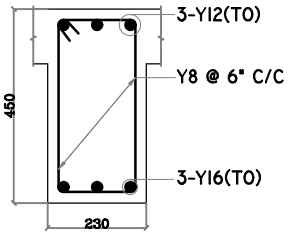
STRUCTURAL CONSULTANTS :-

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#8, 50th St, Sarvamangala Colony, 7th Avenue,
ASHOK NAGAR, CHENNAI- 600 083.
Fixed Line :- 044 4859 7100
projects.auraengg@gmail.com
projects@auraengineers.co.in
http://auraengineers.co.in/
aura Engineering Consultants

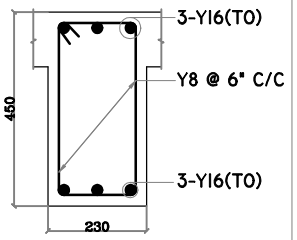
A3

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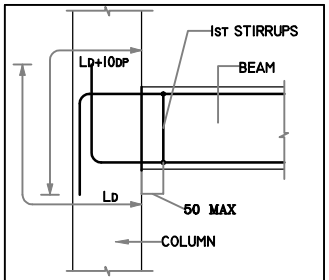
NAME	MID SPAN	END SUPPORT
B1 (230 X 450)		
B3 (230 X 530)		



C.S. OF BEAM "B2"
(230 X 450)



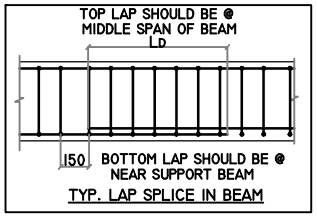
C.S. OF BEAM "B4"
(230 X 450)



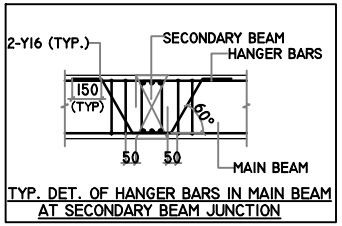
TYP. EXTERNAL JOINT DETAIL AT BEAM-COLUMN JUNCTION

BEAM SCHEDULE

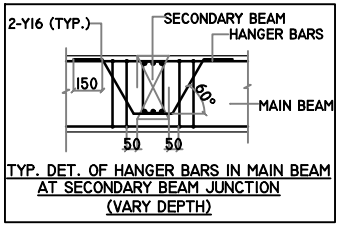
SL.NO	NAME	SIZE
01	B1	230 x 450
02	B2	230 x 450
03	B3	230 x 530
04	B4	230 x 450



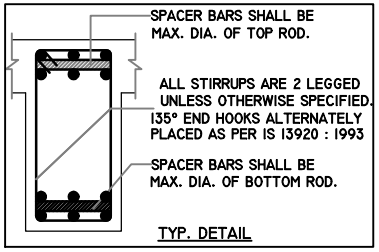
TYP. LAP SPLICE IN BEAM



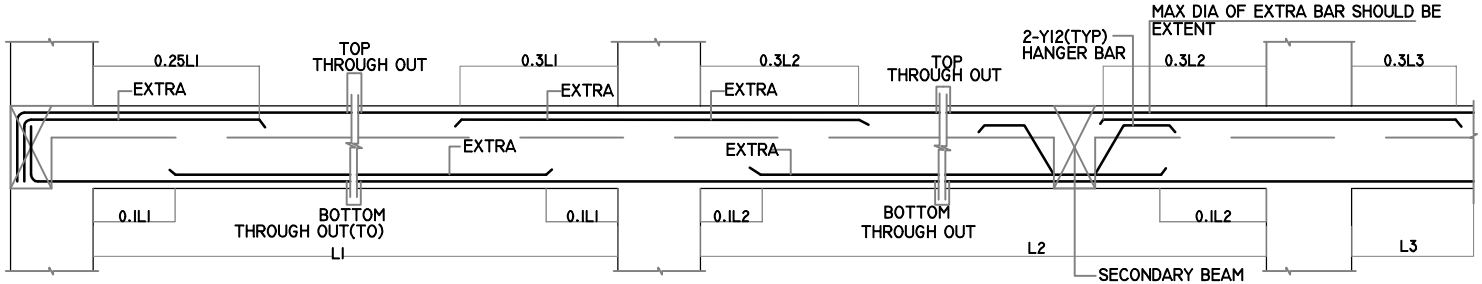
TYP. DET. OF HANGER BARS IN MAIN BEAM AT SECONDARY BEAM JUNCTION



TYP. DET. OF HANGER BARS IN MAIN BEAM AT SECONDARY BEAM JUNCTION (VARY DEPTH)



TYP. DETAIL



TYPICAL L.S OF ROOF BEAM

GENERAL NOTES:

1. ALL DIMENSIONS ARE IN FOOT UNITS.
2. CONCRETE GRADE SHALL BE M25 CONFIRMING TO IS 456:2000
3. GRADE OF REINFORCEMENT MARKED THUS (Y) SHALL BE Fe 500 CONFIRMING TO IS 1786-2008
4. CLEAR COVER FOR REINFORCEMENT
   A) COLUMN = 1½" B) BEAM = 1"
   C) SLAB = ¾"
5. THIS DRG. READ IN CONJECTION WITH ARCH.DRGS.
6. STRUCTURAL ENGINEER RESPONSIBILITY IS LIMITED TO DESIGN & ISSUAL OF STRUCTURAL DRAWING ONLY
7. DEVELOPMENT LENGTH (LD) SHALL BE AS PER (SP:34-1999) TABLE BELOW:

DEVELOPMENT LENGTH FOR COLUMNS	= 40D
DEVELOPMENT LENGTH FOR BEAMS	= 50D

8. DETAILING OF REBARS SHALL CONFIRM TO SP-34 & IS 13920.

REV.	DESCRIPTIONS	DRN	CHK	DATE
00	ISSUE FOR APPROVAL	M.A	R.M	05.04.23

PROJECT :-

PLAN SHOWING THE PROPOSED CONSTRUCTION OF A RESIDENTIAL BUILDING WITH STILT+5 FLOORS @ PLOT NO:7 & 10, RAJAJI SALAI, SENTHIL NAGAR, THIRUMULAIVOYAL, CHENNAI - 600062.

DRAWING TITLE :-

R.C. DETAILS OF TERRACE FLOOR SLABS & BEAMS

DATE	SCALE	JOB.NO	DRG.NO
05.04.23	1 : 130	——	009

ARCHITECT / CONSULTANTS :-

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A3



our services doesn't just end there

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Property Development & Approvals

Building plan approvals, construction, layout and sub-division approvals, zone conversion, and occupancy certification.

NRI & Investment Services

Escrow Services, Rentals, Litigation Solutions, Property Valuation and Property Management

Compliance & Special Services

Demolition, Vaasthu checks, regularization of unauthorized buildings, and office space solutions.



empowering your real estate decisions with smart tools

Property Valuation & Cost Estimation

Guideline Value, Apartment Composite Value, Stamp Duty & Registration Fees Calculator, Building Value Calculator, Capital Gains Calculator, Area Unit Converter, Length Unit Converter

Ownership, Compliance & Legal Verification

Encumbrance Certificate (EC) Search, Find Your SRO, Temple Property Check, WAQF Property Check, View Patta/Chitta, View FMB, Patta Analyzer, EC Analyzer

Zoning & Land Use Analysis

Land Use Zone Finder, CRZ Zone Check, Aquifer Zone Check, CBA Check, EWS Check, Road Width Check, Proposed Road Widening Check, FSI Calculator

Property Documentation & Workflow Tools

Forms & Templates, Document Generator, Document Translation, Dictionary

Vastu & Property Management Tools

Home Vastu Shastra, Plot Vastu Shastra, Property Management

Verified RealEstate™

We're honored to be part of your real estate journey

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