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CLIENT: Er. P. PREMA

PROJECT: GEOTECHNICAL INVESTIGATIONS FOR THE PROPOSED CONSTRUCTION OF RCC ROOF RESIDENTIAL APARTMENT BUILDING IN S.F NO.256/2B2 AT KRISHNAGIRI TO HODUR NH-44 ROAD, SIKKARIMEDU OF KURUBARAPALLI VILLAGE &PANCHAYAT, VEPPANAPALLI PANCHAYAT UNION, KRISHNAGIRI TALUK &DISTRICT.

RGEO-231016

02nd November 2023

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GEOTECHNICAL INVESTIGATIONS ON

PROPOSED CONSTRUCTION OF RCC ROOF RESIDENTIAL APARTMENT BUILDING IN S.F NO.256/2B2 AT KRISHNAGIRI TO HODUR NH-44 ROAD, SIKKARIMEDU OF KURUBARAPALLI VILLAGE &PANCHAYAT, VEPPANAPALLI PANCHAYAT UNION, KRISHNAGIRI TALUK & DISTRICT.

RGEO-231016

Issue No.	Date	Description	Prepared	Checked
1	02/11/2023	Final	NVT	AK

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ABBREVIATIONS

EGL	Existing Ground Level
SPT	Standard Penetration Test
GWT	Ground Water Table
TP	Trial Pits
BH	Bore Holes
CBR	California Bearing Ratio
IS	Indian Standard
1V : 2H	1 Vertical : 2 Horizontal
MDD	Maximum Dry Density
OMC	Optimum Moisture Content
m	Meter
mm	Millimetre
m3	Cubic Meter
MSL	Mean Sea Level
KN/m3	Kilo Newton per Cubic Meter
Sec/mm	Seconds per Millimetre
SPT 'N'	Standard Penetration Test 'N' value
pH	Hydrogen Ion Concentration
qa	Allowable Bearing Pressure
IC	In-situ CBR
SBC	Safe Bearing Capacity
SO4	Sulphate
C	Cohesion
Phi	Angle of Internal Friction
RL	Reduced Level
CR	Core Recovery
RQD	Rock Quality Designation
TCR	Total Core Recovery
Kpa	Kilo Pascal
D	Depth
Kg	Kilograms
Kg/cm2	Kilograms per square centimetre
CS	Cross Section

1. INTRODUCTION

Er. P.Prema. entrusted M/s. Alcon Consulting Engineers (India) Pvt Ltd, Bengaluru to conduct Geotechnical investigations for the Proposed Construction of RCC Roof Residential Apartment Building in S.F No.256/2B2 at Krishnagiri to Hodur NH-44 Road, Sikkarimedu of Kurubarapalli Village & Panchayat, Veppanapalli Panchayat Union, Krishnagiri Taluk & District.

The purpose of this investigation is to perform geotechnical investigations and to provide foundation support recommendations.

Broad objectives of the investigations are as follows.

(a) To evaluate the parameters of soil/rock at the proposed site.

(b) To assess the engineering parameters and to estimate the safe bearing capacity of soil.

2. SCOPE OF WORK

The scope of work includes drilling of Three (03) boreholes at the Proposed Construction of RCC Roof Residential Apartment Building in S.F No.256/2B2 at Krishnagiri to Hodur NH-44 Road, Sikkarimedu of Kurubarapalli Village & Panchayat, Veppanapalli Panchayat Union, Krishnagiri Taluk & District. "Borehole Locations" sketch are provided by **Er. P.Prema.**

The scope also includes carrying out field tests like Standard Penetration Test (SPT) in boreholes, collection of disturbed and undisturbed soil samples and carrying out the relevant laboratory tests on collected soil samples.

3. FIELD INVESTIGATION

The field investigation was carried out on 28th October 2023. Three (03) exploratory boreholes were drilled up to a depth of 6.0 m or refusal (N>50), whichever is shallower, below the existing ground level. The boreholes were sunk by using Hand Auger. During drilling, Standard Penetration Tests SPT were conducted in accordance with IS 2131 – 1981. The test involves driving a 50mm external diameter thick walled tube (Split-Barrel) Sampler into the bottom of the borehole with successive blows from a 63.5kg hammer, falling freely through 760mm height. The sampler is driven in three intervals of 150mm each and the number of blows required to penetrate each interval is recorded. The initial

150mm is intended to ensure "seating" of the sampler such that it penetrates beyond the zone of influence of any soil disturbance at the base of the borehole. The total number of blows to drive the sampler over the final 300mm is termed as the "N" value, and is considered indicative of the in-situ relative soil density. In a very dense and/or cemented soil layers it is often not possible to ensure complete penetration of the SPT sampler, due to driving refusal, or the risk of damage to sampling equipment. In such cases, where a penetration of 300mm was not achieved, the distance driven and number of blows are recorded on the Borehole Logs. SPT was performed wherever possible and the N-values are reported. Disturbed SPT and bulk samples collected during drilling were retained in sealed, labeled air tight containers. Relatively undisturbed soil samples, if possible, were taken from cohesive soils using Shelby tube samplers, later transported to our laboratory for further classifications and determination of engineering properties. The details of the strata encountered along with the field tests results are presented in Appendix B.

4. LABORATORY TESTING

The laboratory testing was performed in our laboratory on soil samples obtained during the field investigation. The laboratory testing was performed as per the relevant parts of IS 2720. The test results were used to classify the soils and determine the physical and strength properties of soil.

The following tests were conducted:

1) Wet Sieve analysis/Hydrometer Test (IS 2720, Part 4, 1975)
2) Liquid and Plastic limit tests (IS 2720, Part 5, 1970)
3) Natural Moisture Content (IS 2720, Part 2, 1973)
4) Direct Shear Test (IS 2720, Part 13)

The laboratory test results are presented in APPENDIX – C

5. GENERALISED GEOLOGICAL PROFILE

A generalized geological profile is prepared for individual boreholes. The description of the various layers is based on the laboratory test results and on IS classification. For the purpose of clarity, existing level is considered as 0.0m, datum for boreholes in the individual geological profile.

5.1 GEOLOGICAL PROFILE

Bore logs for the Proposed Construction of RCC Roof Residential Apartment Building in S.F No.256/2B2 at Krishnagiri to Hodur NH-44 Road, Sikkarimedu of Kurubarapalli Village & Panchayat, Veppanapalli Panchayat Union, Krishnagiri Taluk & District. Shows the soil strata as per field observation & bore log. The geology of the proposed site is varying with respect to the thickness of each layer. BH-01 consists of Yellowish Brown with Greyish White Clayey Silty Sand (0.00 to 0.50m), below this level it consists Greyish White Silty Sand (0.50 to 1.70m).

6. GROUND WATER TABLE

Ground water table was Not Encountered during the time of Investigation.

7. GENERAL DISCUSSIONS FOR THE CHOICE OF SUITABLE FOUNDATION

Based on the field observations and laboratory test results the founding levels and the Allowable bearing capacity which can be considered at that level shall be determined by considering the basic criteria as

- The foundation should carry the maximum loads coming from the structure with margin of factor of safety.
- The probable maximum and differential settlement of the soil under any part of the foundation must be limited to safe and tolerable limits.

8. CONCLUSIONS AND RECOMMENDATIONS

According to the field and laboratory investigation, subsurface conditions, engineering analysis and practical experience, it can be concluded that proposed structures/ facilities can be satisfactorily supported by the ground at site, provided that the following suggestions are taken into considerations:

- Isolated footing may be designed with the below mentioned Net Safe Bearing Capacity for minimum depth of 1.50m from Existing Ground Level and all the Columns should be proportioned for equal settlement. The above mentioned net safe bearing pressure can be considered for design of foundations for the proposed structure with an allowable settlement of 25mm.

- In case any change in strata is observed during foundation excavation, the same shall be brought to the notice of Geotechnical Consultant for the review of recommended net SBC.

  In case, Seepage encountered, it is recommended to do proper dewatering shall be adapted to drain out the seepage water at the time of foundation excavation.
- When excavating to design level, care should be taken to prevent any disturbance to the exposed strata. If any disturbance occurs, or if any soft spots are encountered, the material should be removed and replaced by selected granular fill, which should be compacted to not less than 95% of maximum dry density (MDD) as determined by modified proctor test.
- All excavation shall be performed in accordance with the National Building Code of India, 1983 and relevant IS Codes. The contractor shall be responsible for all necessary arrangements for drainage and support for any excavation and/or construction. Variable site conditions always include the possibility of encountering very soft soils, ground water. The variable site conditions may include waste dumped soil, loosely filled up soil, boulders, or other adverse soil conditions in native ground, that were not identified in this investigation.

8.1 BEARING CAPACITY

Based on the information obtained from samples collected at site, the soil profiles, the lithology encountered and recommended Net Safe Bearing Pressure are summarized below.

Isolated Footings

Isolated footings may be designed with the below mentioned Net Safe Bearing Pressure and all the Columns should be proportioned for equal settlement. The following net safe bearing pressure can be considered for design of foundations for the proposed structure with an allowable settlement of 25mm. The calculations are based on equation recommended by IS Code: 6403 which takes care of L/B ratio (shape), foundation depth etc., along with other parameters based on local shear failure.

Table1. Bearing Capacity for the Proposed Structure

Sl. No	Net Safe Bearing Pressure, T/m²		Factor of safety	Width of the footing (m)
	1.50m (BEGL)	1.70m (BEGL)	2.50	2
BH-01 to BH-03	27	30		

**** BEGL - Below the existing Ground Level.***

Settlement as per IS: 8009(Part I) for Isolated Footing

As Per Fig.9 of IS: 8009 (Part 1)

Settlement = 3.8×10^{-3}(m) X 2.7X100X10

= 10.26mm < 25mm

Hence Adopt **SBC 27 T/m2** (SAFE) at 1.50m depth.

8.2 EXCAVATION

All excavation shall be performed in accordance with the National Building Code of India, 1983 and relevant IS Codes. The contractor shall be responsible for all necessary arrangements for drainage and support for any excavation or construction. Variable site conditions always include the possibility of encountering very soft soils and ground water.

8.3 LATERAL EARTH PRESSURE

The proposed construction may involve below grade structures and therefore lateral earth pressure parameters are required for the soils within the excavated depths. These parameters are derived from laboratory tests, field tests and published data. The following table summarizes lateral earth pressure parameters for the various soils encountered.

Soil Parameters		
Bulk Density (g/cc)		2.00
Angle of Internal Friction Φ(degrees)		36
Coefficients of lateral earth pressure	Ko	0.41
	Ka	0.26
	Kp	3.85

Report Limitations

The depicted subsurface conditions of this site are solely based on the exploratory bore holes which are specific to their locations and provide information about a relatively small column of the soils/rock and the possibility of actual conditions differing must be recognized.

Remarks:

1. The result is of sample of soil collected through shear strength parameters.
2. The sample will be preserved in our laboratory for a period of fifteen day's only.
3. This report cannot be produced for legal purposes.
4. This report is subject to Bangalore Jurisdiction only.
5. This report should not be treated as the endorsement of the sample submitted for testing.

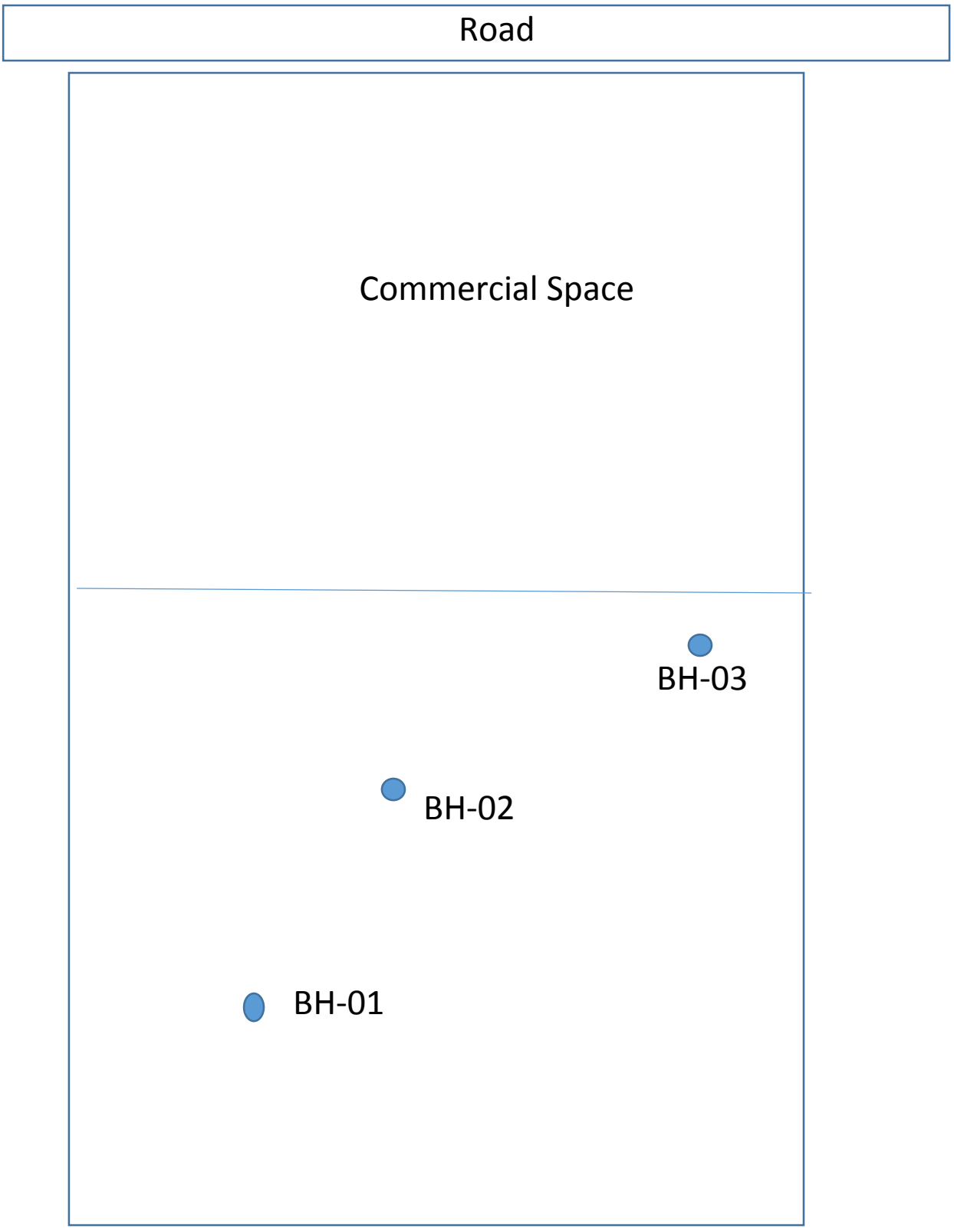
Note

1. The test results relate only to the item(s) tested.
2. Publication or reproduction of this report in any form other than by complete set of the whole report and in the language written is not permitted without the written approval.
3. Corrections / erasing invalidate the test report.
4. Any anomaly / discrepancy in the test report should be brought to our notice within 10 days from the date of issue.
5. # indicates that for such tests there is no formal NABL accreditation and the tests are conducted as per the relevant applicable National / International standard or as per the specific customer requirement.

REFERENCES

1. IS 1498, 1970 - Classification and Identification of Soils for General Engineering Purposes,
2. IS 2720, Part 40, 1977 - Method of Test for Soils: Determination of Free Swell Index of Soils,
3. IS 2720, Part 1, 1972 - Preparation of Dry Samples for Various Tests,
4. IS 2720, Part 2 - Determination of Water Content,
5. IS 2720, Part 4, 1975, - Grain Size Analysis,
6. IS 2720, Part 5, 1970, - Determination of Liquid and Plastic Limits,
7. IS 1892:1979 - Code of Practice for Sub-Surface Investigation for Foundations,
8. IS 2131:1981 - Method of Standard Penetration Test for Soils (First Revision),
9. IS 6403-1981 - Determination of Allowable Bearing on Shallow Foundations,
10. National Building Code of India, Bureau of Indian Standard

APPENDIX A – PROJECT LOCATION MAP


Road
Commercial Space
BH-03
BH-02
BH-01

APPENDIX B – BORE LOGS

BOREHOLE LOG

ALCON CONSULTING ENGINEERS (INDIA) PVT LIMITED.

BH. N0. **1(N12 35' 16",E78 8'36'')**

Sheet **1**

Project: PROPOSED CONSTRUCTION OF RCC ROOF RESIDENTIAL APARTMENT BUILDING IN S.F NO.256/2B2 AT KRISHNAGIRI TO HODUR NH-44 ROAD, SIKKARIMEDU OF KURUBARAPALLI VILLAGE &PANCHAYAT, VEPPANAPALLI PANCHAYAT UNION, KRISHNAGIRI TALUK &DISTRICT.

Logged by: NVT
Checked by: AK

Date Started: **28/10/2023**
Date Completed: **28/10/2023**

Equipment: : Auger
BH Depth:1.70 Meters

BH. DIA.: 150 **mm**
Slope: -90

Ground Lvl: **0.00m**
Water Table: **Not Encounterd**

Drilling Info					Description of Strata	SPT, Blows				Sample				TCR %	SCR %	RQD %	Notes
Method	Depth (m)		Water	Log		15 cm	30 cm	45 cm	N	From	To	No.	Type				Structure, additional info.
Auger Boring	0.0				Brownish white Clayey silty sand					0.00	0.50		DS				
	0.2																
	0.4																
	0.6																
	0.8																
	1.0																
	1.2																
	1.4				Greyish White with light Brown Clayey Silty Sand	12	18	35	**53**	0.50	1.50		SPT				
	1.6																
	1.8				Greyish White with light Brown Silty Sand(S/R)	100 for 5cm			**>100**	1.50	1.70		SPT				
	2.0				END OF DRILLING 1.70m BEGL (END OF EXPLORATION)												
	2.2																
	2.4																
	2.6																
	2.8																
	3.0																
	3.2																
	3.4																
	3.6																
	3.8																
	4.0																
	4.2																
	4.4																
	4.6																
	4.8																
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	7.0																
	7.2																
	7.4																
	7.6																
	7.8																
	8.0																
	8.2																
	8.4																
	8.6																
	8.8																
	9.0																
	9.2																
	9.4																
	9.6																
	9.8																
	10.0																

Method

AD	Auger drilling
RR	Rotary Drilling
PD	Percussion Drilling
CT	Cable TOOL
HA	Hand auger
B	Blank bit
V	V bit
T	TC bit
A	Air
W	Water

Notes, samples, tests

U	undisturbed sample
D	disturbed samples
N	standard penetration test (SPT)
N*	SPT – sample recovered
Nc	SPT with solid cone
V	vane shear
B	bulk sample
E	environment sample
W/C	chiseled and washed sample
R	refusal
TCR	Total Core Recovery

Moisture

D	Dry
M	Moist
V	Wet

Density Index

VS	very soft
S	soft
F	firm
St	stiff
VSt	very stiff
H	hard
Fb	friable
VL	very loose
L	loose
MD	medium dense

Weathering

FR	fresh
SW	slightly weathered
MW	moderately weathered
HW	highly weathered
XW	extremely weathered
DW	distinctly weathered
Water	Water Table

BOREHOLE LOG

ALCON CONSULTING ENGINEERS (INDIA) PVT LIMITED.

BH. N0. **2(N12 35'17'',E78 8'36'')**

Sheet **1**

Project: PROPOSED CONSTRUCTION OF RCC ROOF RESIDENTIAL APARTMENT BUILDING IN S.F NO.256/2B2 AT KRISHNAGIRI TO HODUR NH-44 ROAD, SIKKARIMEDU OF KURUBARAPALLI VILLAGE &PANCHAYAT, VEPPANAPALLI PANCHAYAT UNION, KRISHNAGIRI TALUK &DISTRICT.

Logged by: NVT
Checked by: AK

Date Started: **28/10/2023**
Date Completed: **28/10/2023**

Equipment: : Auger
BH Depth:1.30 Meters

BH. DIA.: 150 **mm**
Slope: -90

Ground Lvl: **0.00m**
Water Table: **Not Encounterd**

Drilling Info					Description of Strata	SPT, Blows				Sample				TCR %	SCR %	RQD %	Notes
Method	Depth (m)		Water	Log		15 cm	30 cm	45 cm	N	From	To	No.	Type				Structure, additional info.
Auger Boring	0.0				Brownish white Clayey silty sand					0.00	0.50		DS				
	1.0				Greyish White with light Brown Silty Sand(S/R)	100 for 8cm			**>100**	0.50	1.30		SPT				
	2.0				END OF DRILLING 1.30m BEGL (END OF EXPLORATION)												
	3.0																
	4.0																
	5.0																
	6.0																
	7.0																
	8.0																
	9.0																
	10.0																

Method

AD	Auger drilling
RR	Rotary Drilling
PD	Percussion Drilling
CT	Cable TOOL
HA	Hand auger
B	Blank bit
V	V bit
T	TC bit
A	Air
W	Water

Notes, samples, tests

U	undisturbed sample
D	disturbed samples
N	standard penetration test (SPT)
N*	SPT – sample recovered
Nc	SPT with solid cone
V	vane shear
B	bulk sample
E	environment sample
W/C	chiseled and washed sample
R	refusal
TCR	Total Core Recovery

Moisture

D	Dry
M	Moist
V	Wet

Density Index

VS	very soft
S	soft
F	firm
St	stiff
VSt	very stiff
H	hard
Fb	friable
VL	very loose
L	loose
MD	medium dense

Weathering

FR	fresh
SW	slightly weathered
MW	moderately weathered
HW	highly weathered
XW	extremely weathered
DW	distinctly weathered
Water	Water Table

BOREHOLE LOG

ALCON CONSULTING ENGINEERS (INDIA) PVT LIMITED.

BH. N0. **3(N12 35' 17'',E78 8'37'')**

Sheet **1**

Project: PROPOSED CONSTRUCTION OF RCC ROOF RESIDENTIAL APARTMENT BUILDING IN S.F NO.256/2B2 AT KRISHNAGIRI TO HODUR NH-44 ROAD, SIKKARIMEDU OF KURUBARAPALLI VILLAGE &PANCHAYAT, VEPPANAPALLI PANCHAYAT UNION, KRISHNAGIRI TALUK &DISTRICT.

Logged by: NVT
Checked by: AK

Date Started: **28/10/2023**
Date Completed: **28/10/2023**

Equipment: : Auger
BH Depth:1.30 Meters

BH. DIA.: 150 **mm**
Slope: -90

Ground Lvl: **0.00m**
Water Table: **Not Encounterd**

Drilling Info					Description of Strata	SPT, Blows				Sample				TCR %	SCR %	RQD %	Notes
Method	Depth (m)		Water	Log		15 cm	30 cm	45 cm	N	From	To	No.	Type				Structure, additional info.
Auger Boring	0.0				Brownish white Clayey silty sand					0.00	0.50		DS				
	1.0				Greyish White with light Brown Silty Sand(S/R)	100 for 6cm			**>100**	0.50	1.30		SPT				
	2.0				END OF DRILLING 1.30m BEGL (END OF EXPLORATION)												
	3.0																
	4.0																
	5.0																
	6.0																
	7.0																
	8.0																
	9.0																
	10.0																

Method

AD	Auger drilling
RR	Rotary Drilling
PD	Percussion Drilling
CT	Cable TOOL
HA	Hand auger
B	Blank bit
V	V bit
T	TC bit
A	Air
W	Water

Notes, samples, tests

U	undisturbed sample
D	disturbed samples
N	standard penetration test (SPT)
N*	SPT – sample recovered
Nc	SPT with solid cone
V	vane shear
B	bulk sample
E	environment sample
W/C	chiseled and washed sample
R	refusal
TCR	Total Core Recovery

Moisture

D	Dry
M	Moist
V	Wet

Density Index

VS	very soft
S	soft
F	firm
St	stiff
VSt	very stiff
H	hard
Fb	friable
VL	very loose
L	loose
MD	medium dense

Weathering

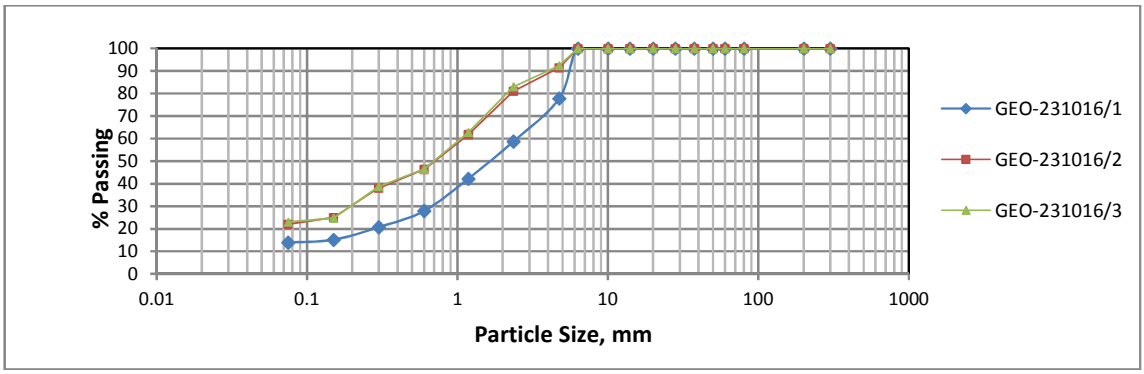
FR	fresh
SW	slightly weathered
MW	moderately weathered
HW	highly weathered
XW	extremely weathered
DW	distinctly weathered
Water	Water Table

APPENDIX C – LABORATORY TEST SUMMARY

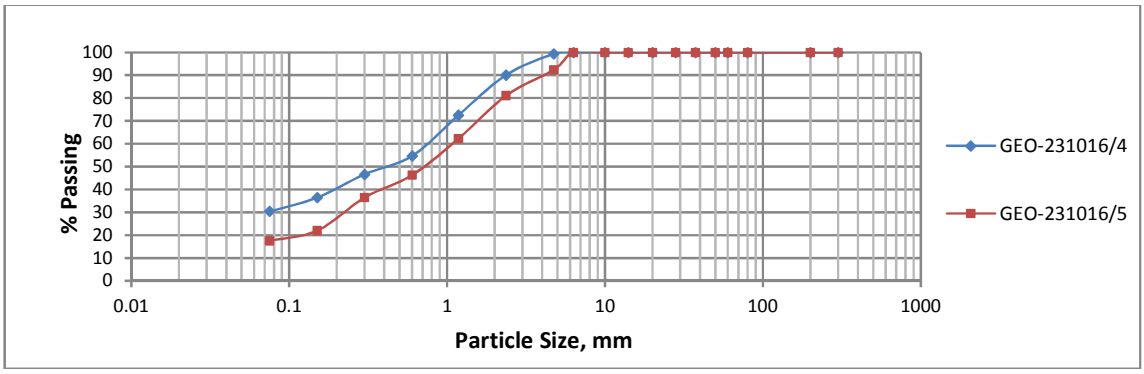
Table 1. Summary of Lab Test Results Boreholes

Sl. No	Borehole No	Depth, (m)	Natural Moisture Content %	Particle Size Analysis			Atterberg's Limits			Shear Parameters	
				Gravel %	Sand %	Silt % & Clay%	Liquid Limit %	Plastic Limit %	PI	c, kPa	phi, deg
1	BH-01	0.50	3.96	22.12	63.95	13.93	-	-	-	-	-
		1.50	7.37	8.51	69.42	22.07	27.47	NP	NP	2	36
		1.70	9.32	7.60	69.35	23.05	-	-	-	-	-
2	BH-02	0.50	7.28	0.60	69.06	30.34	-	-	-	-	-
		1.30	4.50	7.53	74.97	17.50	26.80	NP	NP	0	37
3	BH-03	0.50	6.78	0.58	69.20	30.22	-	-	-	-	-
		1.30	5.08	6.99	73.40	19.61	27.45	NP	NP	0	38

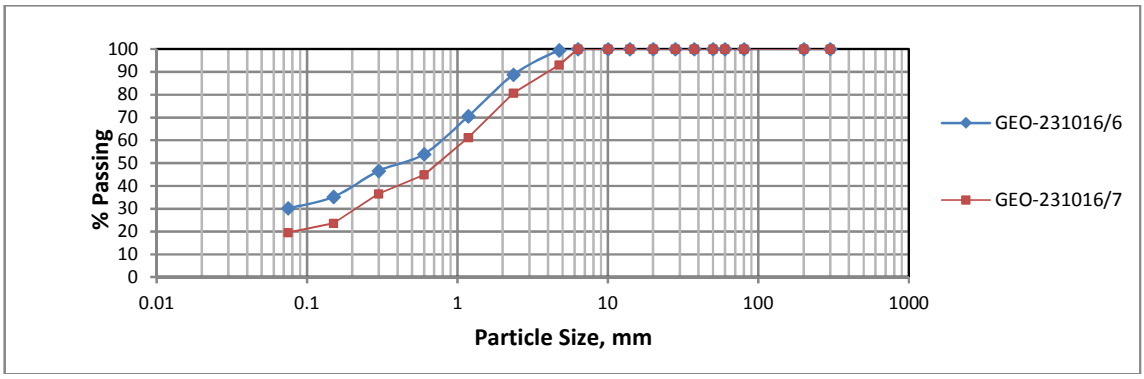
APPENDIX C.1 – GRAIN SIZE DISTRUBUTION CURVES



BH No.	Sample Identification	Boulders	Cobbles	Gravel	Sand	Silt/Clay
BH-01,0.50m	GEO-231016/1	0	0	22.12	63.95	13.93
BH-01,1.50m	GEO-231016/2	0	0	8.51	69.42	22.07
BH-01,1.70m	GEO-231016/3	0	0	7.60	69.35	23.05

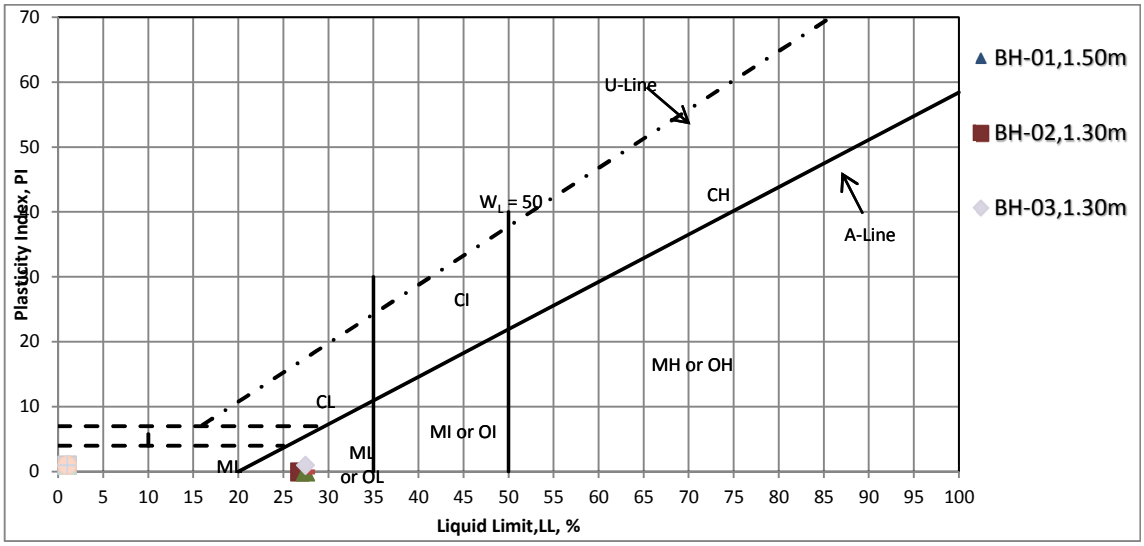


BH No.	Sample Identification	Boulders	Cobbles	Gravel	Sand	Silt/Clay
BH-02,0.50m	GEO-231016/4	0	0	0.60	69.06	30.34
BH-02,1.30m	GEO-231016/5	0	0	7.53	74.97	17.50



BH No.	Sample Identification	Boulders	Cobbles	Gravel	Sand	Silt/Clay
BH-03,0.50m	GEO-231016/6	0	0	0.58	69.20	30.22
BH-03,1.30m	GEO-231016/7	0	0	6.99	73.40	19.61

APPENDIX C.2 – ATTERBERG LIMITS RESULTS



Sl. No.	Location	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index
1	BH-01,1.50m	27.47	NP	NP
2	BH-02,1.30m	26.80	NP	NP
3	BH-03,1.30m	27.45	NP	NP



our services doesn't just end there

Buying & Selling Services

Property Buying Assistance, Property Selling Support, Market Price Analysis, Owner Outreach and Negotiation.

Legal & Documentation Services

Certified EC, Patta, legal opinions, property verification, due diligence, litigation solutions, and legal heir certificates.

Property Development & Approvals

Building plan approvals, construction, layout and sub-division approvals, zone conversion, and occupancy certification.

NRI & Investment Services

Escrow Services, Rentals, Litigation Solutions, Property Valuation and Property Management

Compliance & Special Services

Demolition, Vaasthu checks, regularization of unauthorized buildings, and office space solutions.



empowering your real estate decisions with smart tools

Property Valuation & Cost Estimation

Guideline Value, Apartment Composite Value, Stamp Duty & Registration Fees Calculator, Building Value Calculator, Capital Gains Calculator, Area Unit Converter, Length Unit Converter

Ownership, Compliance & Legal Verification

Encumbrance Certificate (EC) Search, Find Your SRO, Temple Property Check, WAQF Property Check, View Patta/Chitta, View FMB, Patta Analyzer, EC Analyzer

Zoning & Land Use Analysis

Land Use Zone Finder, CRZ Zone Check, Aquifer Zone Check, CBA Check, EWS Check, Road Width Check, Proposed Road Widening Check, FSI Calculator

Property Documentation & Workflow Tools

Forms & Templates, Document Generator, Document Translation, Dictionary

Vastu & Property Management Tools

Home Vastu Shastra, Plot Vastu Shastra, Property Management

Verified RealEstate™

We're honored to be part of your real estate journey

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