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Proposed Residential Building, Eeshanya's Synchrony *Coimbatore*

TYPICAL STRUCTURAL DESIGN CALCULATION

GENERAL:

The ***Proposed Construction for Eeshanya's Synchrony Residential Building (Stilt+05) At Vellakinar village Coimbatore*** designed as per the following floor requirements.

1. Proposed Construction For Residential Purpose - Stilt + Five Floors Only

DESIGN BASIS:

Latest Design codes of IS-456, IS-875 and IS-1893 are followed with SP-16 Design Aid books are used for the purpose of calculations.

Since the maximum storey of building is Four Floors, wind load is also taken into account. The structural element is designed for M25 grade of concrete for Column, M20 Grade for Slab & Beams and Fe-500 grade of steel is used for main and secondary reinforcement.

The following are the Clear cover to the main steel adopted:

1. For Footing – 50 mm
2. For Columns – 40 mm
3. For Beams – 25 mm
4. For Slab – 20 mm

CALCULATION OF LOADS:

1. EARTHQUAKE LOAD:

The Earthquake load for this structure is calculated as per IS:1893 as follows:

Design horizontal seismic coefficient, $A_h = ZI\,S_a/2Rg$

Z = Zone factor	= 0.16 (for Chennai)
I = Importance factor	=1
R = Response reduction factor	=5.00
S_a/g, Average response acceleration co-efficient	= 1.0

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2. LIVE LOAD:

The Live load for this structure is calculated as per IS:875 (Part 2) as follows:

The structure is designed for Live loads of 2 KN/Sq.m for Residential.

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3. DEAD LOAD:

Loads from self weight of Slab, Beams, Columns, Walls, Staircase and Water Tank are taken as Dead load of the structure.

a. Dead Load due to Slab:

The dead load of slab can be calculated based on the slab thickness and that should be calculated based on span.

Maximum Slab Thickness provided for this structure is 150 mm.

Therefore,	Load due to slab = 0.15 x 25	= 3.750 KN/Sq.m.
	Load due to Floor Finish	= 0.500 KN/Sq.m.
	Load due to Unknown Partition wall	= 1.000 KN/Sq.m.
	Total Load	= 5.250 KN/Sq.m.

b. Dead Load due to Wall:

The dead load of the wall can be calculated based on the wall thickness.

Maximum Wall Thickness provided for this structure is 230 mm.

Therefore,

Load due to 230 mm Thk solid block masonry = 0.230 x 20 x 1.925 = 8.855 KN/m

Load due to 115 mm Thk solid block masonry = 0.115 x 20 x 1.925 = 4.428 KN/m

c. Dead Load due to Self Weight of Beams:

The dead load of the beam can be calculated based on the size of the beams provided and that should be decided based on the span.

Maximum Beam Size in this structure provided is 300 x 600 mm.

Therefore, Load due to Beam Self weight = 0.3 x 0.600 x 25 = 4.5 KN/m

d. Dead Load due to Self Weight of Column:

The dead load of the column can be calculated based on the size of the columns provided.

Maximum Column Size in this structure provided is 300*600 mm.

Therefore,

Load due to Column Self weight = 0.3 x 0.6 x 1.925 x 25 =8.663 KN/floor

e. Dead Load due to Water Tank:

The dead load of the water Tank is calculated as follows.

Load due to base slab	= 0.15 x 25	= 3.750 KN/Sq.m.
Load due to cover slab	= 0.125 x 25	= 3.125 KN/Sq.m.
Load due to Floor Finish		= 1.000 KN/Sq.m.
Load due to water	= 1.35 x 10	= 13.5 KN/Sq.m.
Load due to Imposed load		= 0.750 KN/Sq.m.
Total Load		=22.125 KN/Sq.m.

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f. Dead Load due to Fillings:

The dead load of the Fillings on sunken area is calculated as follows.

Load due to filling on Toilet area = 0.3 x 20 = 6.00 KN/Sq.m.

LOAD COMBINATIONS:

The following are the load combinations provided for the structure for design purpose as per code.

By considering Earthquake:

1. 1.5DL+1.5LL
2. 1.5DL+1.5ELX
3. 1.5DL+1.5EL-X
4. 1.5DL+1.5ELZ
5. 1.5DL+1.5EL-Z
6. 0.9DL+1.5ELX
7. 0.9DL+1.5EL-X
8. 0.9DL+1.5ELZ
9. 0.9DL+1.5EL-Z
10. 1.2DL+1.2ELX+1.2LL
11. 1.2DL+1.2EL-X+1.2LL
12. 1.2DL+1.2ELZ+1.2LL
13. 1.2DL+1.2EL-Z+1.2LL
14. DL+LL
15. 1.0DL+1.0ELX
16. 1.0DL+1.0EL-X
17. 1.0DL+1.0ELZ
18. 1.0DL+1.0EL-Z
19. 1.0DL+0.8ELX+0.8LL
20. 1.0DL+0.8EL-X+0.8LL
21. 1.0DL+0.8ELZ+0.8LL
22. 1.0DL+0.8EL-Z+0.8LL

By considering WindLoad:

23. 1.5DL+1.5WLX
24. 1.5DL+1.5WL-X
25. 1.5DL+1.5WLZ
26. 1.5DL+1.5WL-Z
27. 0.9DL+1.5WLX
28. 0.9DL+1.5WL-X
29. 0.9DL+1.5WLZ
30. 0.9DL+1.5WL-Z
31. 1.2DL+1.2WLX+1.2LL
32. 1.2DL+1.2WL-X+1.2LL
33. 1.2DL+1.2WLZ+1.2LL
34. 1.2DL+1.2WL-Z+1.2LL
35. 1.0DL+1.0ELX
36. 1.0DL+1.0EL-X
37. 1.0DL+1.0ELZ
38. 1.0DL+1.0EL-Z
39. 1.0DL+0.8ELX+0.8LL
40. 1.0DL+0.8EL-X+0.8LL
41. 1.0DL+0.8ELZ+0.8LL

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42.1.0DL+0.8EL-Z+0.8LL

DESIGN CALCULATION

The ***Proposed Construction for Eeshanya's Synchrony Residential Building (Stilt+05) At Vellakinar village Coimbatore*** A model calculation for the design of the structural elements like Beam, Slab, Column and Foundation is submitted here.

DESIGN OF SLAB

INP
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Slab thickness	=	125	mm
Longer Span	=	3.50	m
Shorter Span	=	2.815	m
Grade of concrete	=	30	N/mm^2
Grade of steel	=	500	N/mm^2
Clear cover	=	20	mm
Load factor	=	1.5	
Boundary condition	=	One long edge discontinuos	

EFFECTIVE SPAN

Longer Span	l_y	=	3.595	m
Shorter Span	l_x	=	2.920	m
Span Ratio		=	1.23	TWO WAY SLAB

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Effective Depth:	Longer Span side	=	95	mm
	Shorter Span side	=	105	mm

LOAD CALCULATION

Self weight of slab	= 25 x (125/1000)	=	3.13	kN/m^2
Floor finish and Partition	=	1	kN/m^2	
Live Load	=	2	kN/m^2	
TOTAL LOAD (w)	= 6.13	kN/m^2		

MOMENT CALCULATION

Factd Bending Moment = $1.5\ w\ \alpha\ l_x^2$

Shorter Span:	-ve	mmt coefficient	=	0.054		
	+ve	mmt coefficient	=	0.041		
-ve	B.M	= 1.5 x 6.75 x 0.068 x 3.483^2	=	4.23 kN-m	kN-m	
+ve	B.M	= 1.5 x 6.75 x 0.051 x 3.483^2	=	3.21 kN-m	kN-m	
Longer Span:	-ve	mmt coefficient	=			
	+ve	mmt coefficient	=	0.043		
-ve	B.M	= 1.5 x 6.75 x 0.047 x 3.483^2	=	2.90 kN-m	kN-m	
+ve	B.M	= 1.5 x 6.75 x 0.035 x 3.483^2	=	2.19 kN-m	kN-m	

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DEPTH CHECK

i) W.r.t to moment:

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Mu = 0.1389 fck b d^2

d = sqrt(8.35 x 10^6 / (0.1389 x 25 x 150)) = 32 mm

D = 52 mm

ii) W.r.t to deflection:

d = Span / (Span-depth ratio x kt) = 44 mm

D = 64 mm

REINFORCEMENT REQUIRED

			Mu/bd^2	Pt reqd	Ast reqd	Min Ast Reqd
				%	mm^2	
Shorter Span	-ve		0.38	0.090	94	150
	+ve		0.29	0.068	71	150
Longer span	-ve		0.38	0.075	71	150
	+ve		0.28	0.056	54	150

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REINFORCEMENT PROVIDED

Ast provide

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								d (mm^2)
Shorter Span:	Support			Y-	10	@	150	524
	Span			Y-	10	@	150	524

Longer span:	Support			Y-	10	@	150	524
	Span			Y-	10	@	150	524

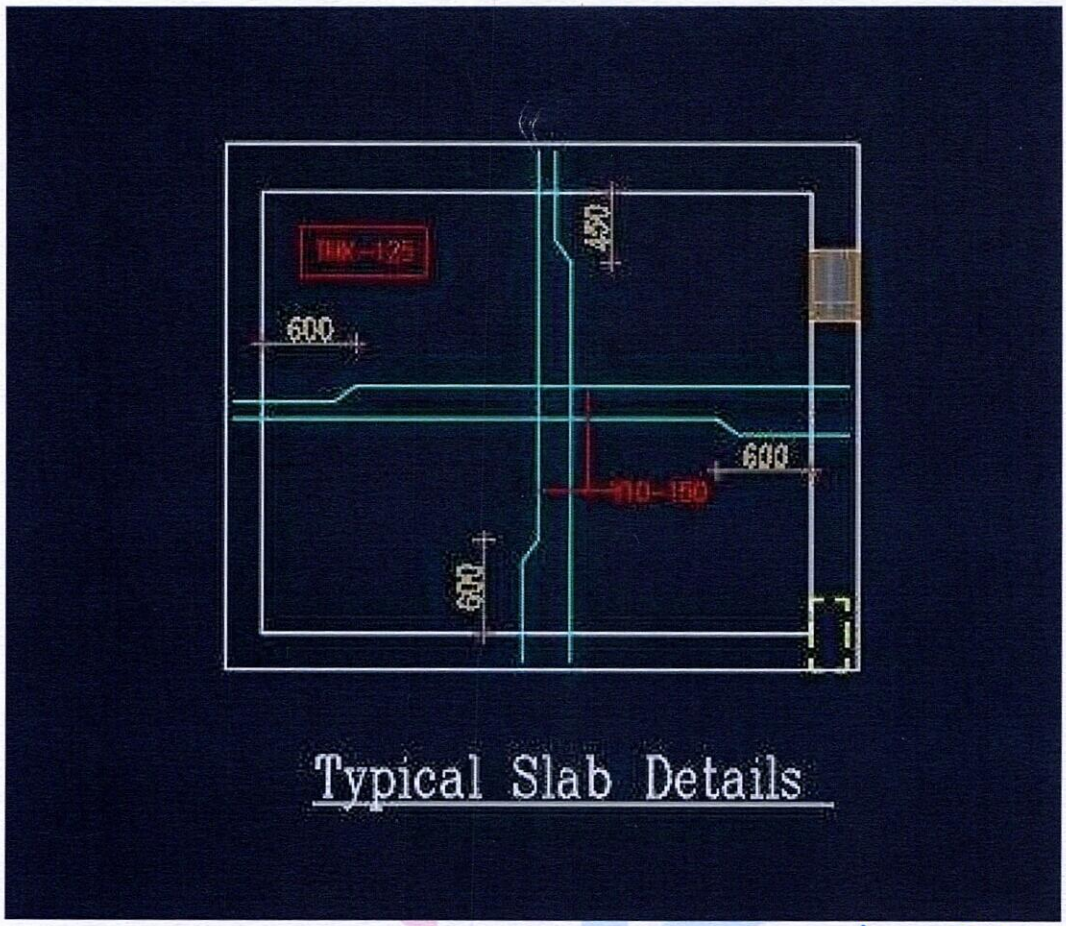
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Typical Slab Details

A model calculation of Beam is given below.

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General Data:

Breadth of beam,	b	=	300	mm
Depth of beam,	D	=	600	mm
Clear cover for reinforcement		=	25	mm
Diameter of main bars,	Y	=	25	mm
Diameter of stirrups,	Y1	=	12	mm
Effective depth of beam,	d	=	562.5	mm
Dist from beam edge to centre of main steel,	d'	=	37.5	mm
Ratio,	d' / d		0.06	
Characteristic strength of concrete,	fck	=	20	N/mm²
Characteristic strength of steel,	fy	=	500	N/mm²

Results from STAAD Analysis:

The following are the results taken from the STAAD analysis,

Critical values due to combination of loads by Earth quake and Wind is taken for the calculation

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Maximum positive moment	=	507	kN-m
Negative moment at support	=	320	kN-m
Maximum shear force	=	620	kN

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Design for Positive moment

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Ultimate positive moment,		Mu	=	507	kN-m
		Mu / bd^2	=	4.158	N/mm^2
Percentage of reinforcement,		pt	=	1.145	
Percentage of compressive reinforcement,		pc	=	0.42	
Area of tensile steel,		Ast	=	2319	mm^2
Provide	**6 nos Y 25 mm as Tensile steel**		=	**3000**	mm^2

Design for Negative moment

Ultimate Negative moment at support			=	320	kN-m
		Mu / bd^2	=	2.45	
Percentage of reinforcement,		pt	=	0.758	
Percentage of reinforcement,		pc	=	0.003	
Area of tensile steel,		Ast	=	1354	mm^2
Provide	**3 nos Y 25mm as Compressive Steel**		=	**1500**	mm^2

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Design for shear

Ultimate shear,	Vu	=	620	kN
Shear stress,	**tvc**	=	3.24	N/mm^2
Area of tensile steel provided,	Ast	=	3000	mm^2

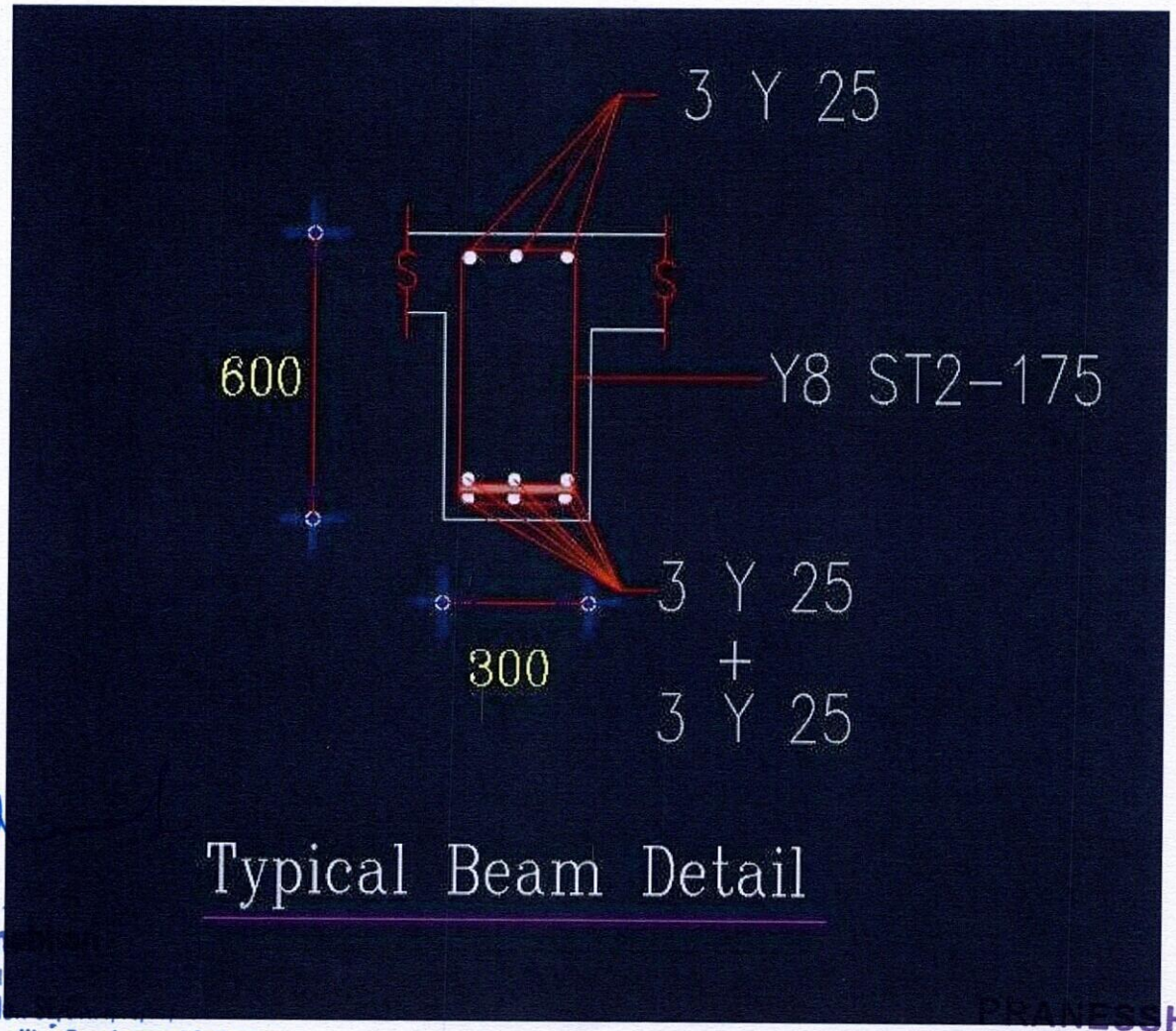
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For p = 100 As/ bd	p	=	1.56
	tc	=	0.73 N/mm²
Allowable shear stress, Shear force to be taken by stirrups,	Vus	=	(**tvc** - **tc**) x bd / 1000
		=	480 kN
Maximum spacing @ 0.75d		225	Mm

Provide 2 Leg Stirrups 8 mm dia at 5" C/C



Typical Beam Detail

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DESIGN OF COLUMN

Grade of concrete		=	M25	=	25	N/mm²
Grade of steel		=	Fe500	=	500	N/mm²
Column Size	B	=		=	300	mm
	D	=		=	600	mm
Unsupported Length	L	=		=	1.925	m
Clear cover				=	40	mm
Dia of the bar				=	25	mm
Axial load on column	P	=		=	1630.00	kN
Bending Moment	Mux	=		=	337.00	kN.m

$$\frac{Pu}{fck \times b \times D} = \frac{1630 \times 1.5 \times 1000}{25 \times 300 \times 600} = 0.55$$

$$\frac{Mux}{fck \times b \times D^2} = \frac{337 \times 1000000 \times 1.5}{25 \times 300 \times 600^{\wedge} 2} = 0.125$$

d' = 52.5

Ref. chart 48 from SP 16

(d'/D = 0.08) = 0.10

Pt = 0.11x25 = 2.77 %

Ast required = 2.77x300x600/100 = 5000 mm²

PROVIDE 12Y25

Ast provided = 5890.5 mm²

Spacing of the Lateral ties is the minimum of the following

(a) L.L.D. = 300 mm

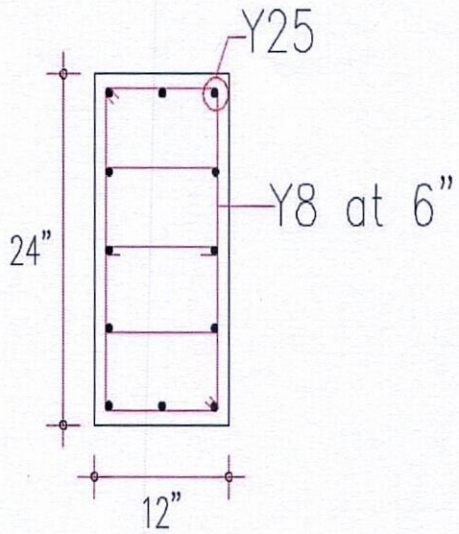
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(b)	16 x d	=	400	mm
(c)	300	=	300	mm

PROVIDE Y8 LATERAL TIES AT 150 mm c/c



Structural Drawing For Column

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DESIGN OF ISOLATED FOOTING

Design of Footing – For a working load of 90t :

COLUMN SIZE	a =	0.3	m
	b =	0.6	m
SBC OF SOIL	w =	250	KN/ m^2
AXIAL LOAD	P =	1600	KN
10 % AXIAL LOAD =		160	KN
TOTAL LOAD	Pu =	1760	KN
AREA REQUIRED A=		7.04	m^2
SIZE OF FOOTING REQUIRED	B =	2.9	m
LET SIZE OF FOOTING	B =	2.9	m
AREA PROVIDED A=		8.41	m^2

CHECK FOR SOIL PRESSURE = P/A = 190.2497 KN/ m^2

SAFE

MOMENT CALCULATION

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Shorter side length B	=	1.225	m
Longer side length L	=	1.335	m
Moment at critical section, Mxx=		**254.3008**	KNm
Depth required	d =	**303.54**	mm
Provide overall depth	D =	**358.54**	

Say overall Depth D = 750 mm

Effective depth	d =	695	mm

REINFORCEMENT

	M/bd^2 =	**0.53**	
% Ast required	pt =	0.164	
Ast required	=	792.8	mm^2
Let diameter of bar	=	12	mm
Spacing of bars using 12 mm		**142.67**	mm
	Say	125	mm

Provide 12 mm @ **125** mm c/c both ways

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No. of bars using 12 mm = 23.8 nos

Ast provide Ast = 2645.136 mm2

PROVIDE 12 mm # 24 both directions

CHECK FOR PUNCHING SHEAR

Area of section As= 1.059 m2

perimeter 4.14 m

Area for shear = 7.351

Shear force Vup = 2097.75 KN

Nominal shear stress Tv = 0.729 N/mm2

Allowable shear sterss Tc = 1.118

SAFE

CHECK FOR BENDING SHEAR

Length of the section 0.64 m

Area of the section A 1.856 m2

Shear force = 529.66 KN

Tv = 0.263 N/mm2

For 100 Ast / bd = 0.13

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Tc = 0.264992

SAFE

SIZE	=	2.9	X	2.9	m	D =	750	mm

PROVIDE	12	mm #	24	nos both directions	

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