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Verified RealEstate™
3/350/2 Mettukuppam,
OMR Thoraipakkam,
Chennai 600 097, India

hello@verified.realestate
+91 9996 12 9996
verified.realestate

HARMONY RESIDENCES PVT. LTD.

DESIGN CALCULATION

FOR

RESIDENTIAL BUILDING

STILT + 5 FLOORS 20 DWELLING UNITS IN SINGLE BLOCK AVAILING PREMIUM FSI WITH TOD BENEFITS

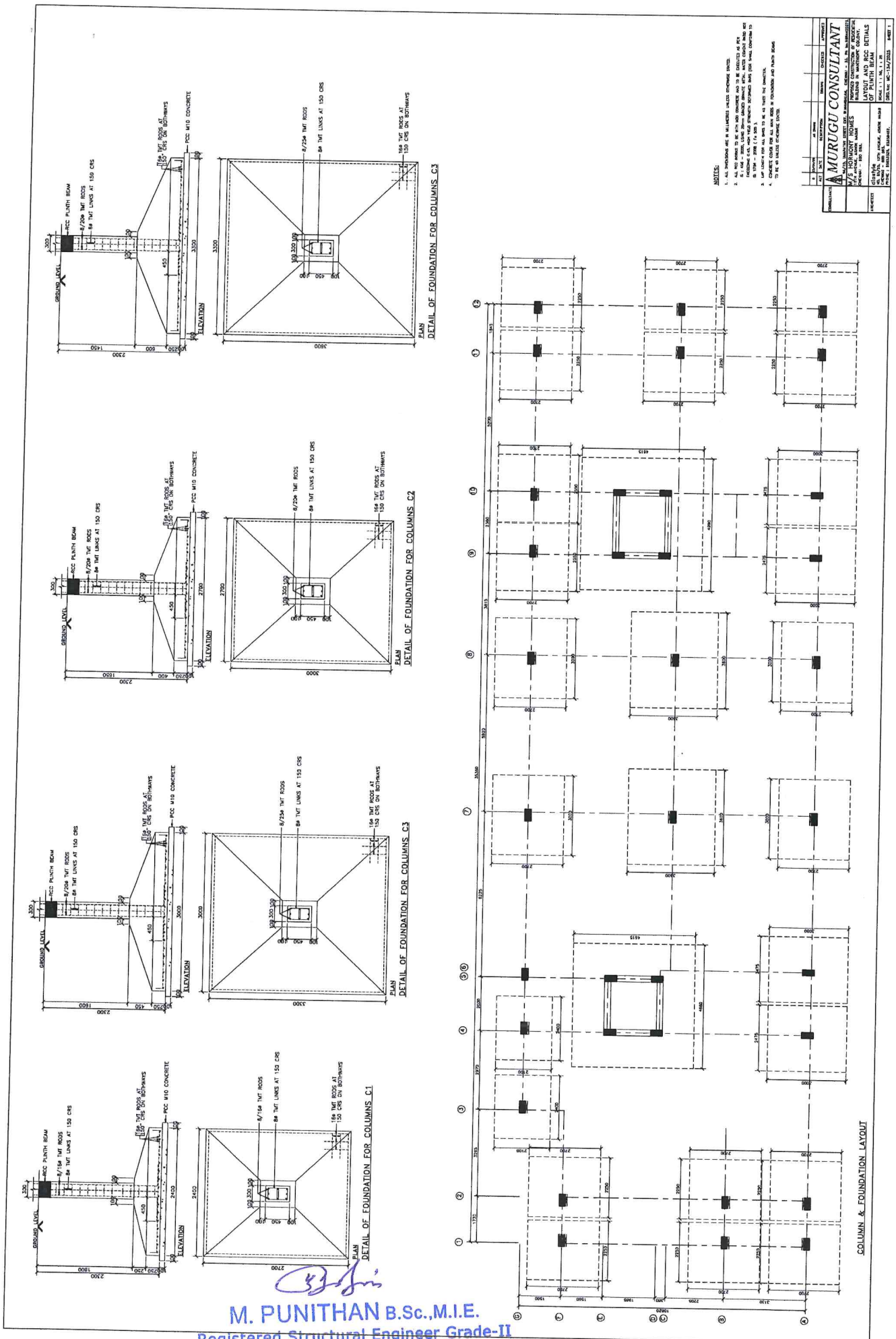
AT BLOCK NO. H20 & H21, MONTHOPE COLONY, 46TH STREET/ 8TH AVENUE, ASHOK NAGAR CHENNAI – 600 083 COMPRISED IN OLD S. NO. 146 PART, 148/1A PART, 149/1 PART, 149/2 PART & 150PART, T. S. NO. 3/14, BLOCK NO.48 OF KODAMBAKKAM PART 2 VILLAGE, MAMBALAM TALUK WITHIN THE LIMITS OF GREATER CHENNAI CORPORATION, OLD DIVISION 132, DIVISION NO. N135, ZONE 10, TAMILNADU.

ARCHITECTS:
M/S DIASTYLE
NO. 65/55, 12TH AVENUE,
ASHOK NAGAR,
CHENNAI .-600083
PHONE:65853326

ENGINEERS:
M/S MURUGU CONSULTANT
NO.9A/10, UMAPATHI STREET EXT,
WEST MAMBALAM,
CHENNAI – 600 033.
PHONE : 9884400273.



M. PUNITHAN B.Sc.,M.I.E.
Registered Structural Engineer Grade-II
CMDA Reg. No. SE/GR-II/19/04/044
Renewed 10-04-2024. Valid upto 09-04-2029
52/141, Flat C2, PJM Apartments, East Vanniar Street,
West K.K. Nagar, Chennai-600 078.
Mobile: 9884400273, Email: mpunithan@yahoo.com



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DESIGN OF PLINTH BEAM

Span considered	=	5.486 m									
Assuming beam size to be		230 x	375	mm							
Load /m :											
Self wt. of beam	=	2.5 x	0.23	x	0.375	=	0.22 t				
Wall	=	2.0 x	0.23	x	4.191	=	1.93 t				
							2.14 t				
Effective Depth of beam	=	375 -	25	-	8	=	342 mm				
Design negative B.M	=	2.14 x	5.486	x	5.486	x	0.100 =	6.45 t-m			
Mu/ fck bdd	=	6.45 x	1.5	x	100000		=	0.180			
		200 x	23	x	34.20	x	34.20				
Ast required	=	0.2608 x	200	x	23.00	x	34.20 =	8.21 sqcm			
			5000								
Use 6/16 dia hsd rods,			**Ast**	=	**10.30 sqcm**		>	**8.21 sqcm**			
Design Positive B.M	=	2.14 x	5.486	x	5.486	x	0.1 =	6.45 t-m			
Mu/ fck bdd	=	6.45 x	1.5	x	100000		=	0.180			
		200 x	23	x	34.20	x	34.20				
Ast required	=	0.2608 x	200	x	23.00	x	34.20 =	8.21 sqcm			
			5000								
Use 6/16 dia hsd rods,			**Ast**	=	**10.30 sqcm**		>	**8.21 sqcm**			
Shear force	=	2.14 x	5.486	/	2.00	=	5.88 t				
Shear stress	=	5.88 x	1.50	x	10000	/	230 x	342 =	1.12 N/sqmm		
100As/ bd	=	100 x	10.30	/	23.00	x	34.20 =	1.31			
Permissible shear stress	=	0.44 N/sqmm		<	1.12 N/sqmm						
Unbalanced shear stress	=	1.12 -	0.442	=	0.68 N/sqmm						
Unbalanced shear force	=	0.68 x	230	x	342	/	10000 =	5.34 t			
Using 8 dia TMT 2 legged stirrups at			150	crs							
Shear Capacity	=	0.87 x	500	x	100.48	x	342 /	150 x	10000		
	=	10.0 t	>	5	t						

RESULT :

Beam section	=	**230 X 375 mm**
Use 6/16 dia hsd rods,		**at top**
Use 6/16 dia hsd rods,		**at bottom**
Using 8 dia TMT 2 legged stirrups at		**150 crs**

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DESIGN OF PLINTH BEAM

Span considered	=	9.900 m									
Assuming beam size to be		230 x	600	mm							
Load /m :											
Self wt. of beam	=	2.5 x	0.23	x	0.6	=	0.35 t				
Wall	=	2.0 x	0.23	x	5.000	=	2.30 t				
							2.65 t				
Effective Depth of beam	=	600 -	25	-	8	=	567 mm				
Design negative B.M	=	2.65 x	9.900	x	9.900	x	0.083 =	21.60 t-m			
Mu/ fck bdd	=	21.60 x	1.5	x	100000		=	0.175			
		250 x	23	x	56.70	x	56.70				
Ast required	=	0.2542 x	250	x	23.00	x	56.70 =	16.57 sqcm			
			5000								
Use 6/20 dia hsd rods,			**Ast**	=	**18.84 sqcm**		>	**16.57 sqcm**			
Design Positive B.M	=	2.65 x	9.900	x	9.900	x	0.08333 =	21.60 t-m			
Mu/ fck bdd	=	21.60 x	1.5	x	100000		=	0.175			
		250 x	23	x	56.70	x	56.70				
Ast required	=	0.2542 x	250	x	23.00	x	56.70 =	16.57 sqcm			
			5000								
Use 6/20 dia hsd rods,			**Ast**	=	**18.84 sqcm**		>	**16.57 sqcm**			
Shear force	=	2.65 x	9.900	/	2.00	=	13.09 t				
Shear stress	=	13.09 x	1.50	x	10000	/	230	x	567 =	1.51 N/sqmm	
100As/ bd	=	100 x	18.84	/	23.00	x	56.70	=	1.44		
Permissible shear stress	=	0.44 N/sqmm		<	1.51 N/sqmm						
Unbalanced shear stress	=	1.51 -	0.442	=	1.06 N/sqmm						
Unbalanced shear force	=	1.06 x	230	x	567	/	10000 =	13.88 t			
Using 8 dia TMT 2 legged stirrups at			125	crs							
Shear Capacity	=	0.87 x	415	x	100.48	x	567 /	125 x	10000		
	=	16.5 t	>	14	t						

RESULT :

Beam section	=	**230 X**	**600**	**mm**
Use 6/20 dia hsd rods,		**at top**		
Use 6/20 dia hsd rods,		**at bottom**		
Using 8 dia TMT 2 legged stirrups at		**125 crs**		

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DESIGN OF PLINTH BEAM

Span considered	=	18.650 m									
Assuming beam size to be		230 x	600	mm							
Load /m :											
Self wt. of beam	=	2.5 x	0.23	x	0.6	=	0.35 t				
Wall	=	2.0 x	0.23	x	2.000	=	0.92 t				
							1.27 t				
Effective Depth of beam	=	600 -	25	-	8	=	567 mm				
Design negative B.M	=	1.27 x	18.650	x	18.650	x	0.083	=	36.67 t-m		
Mu/ fck bdd	=	36.67 x	1.5	x	100000			=	0.298		
		250 x	23	x	56.70	x	56.70				
Ast required	=	0.4314 x	250	x	23.00	x	56.70	=	28.13 sqcm		
			5000								
Use 6/25 dia hsd rods,			**Ast**	=	**29.44 sqcm**			>	**28.13 sqcm**		
Design Positive B.M	=	1.27 x	18.650	x	18.650	x	0.08333	=	36.67 t-m		
Mu/ fck bdd	=	36.67 x	1.5	x	100000			=	0.298		
		250 x	23	x	56.70	x	56.70				
Ast required	=	0.4314 x	250	x	23.00	x	56.70	=	28.13 sqcm		
			5000								
Use 6/25 dia hsd rods,			**Ast**	=	**29.44 sqcm**			>	**28.13 sqcm**		
Shear force	=	1.27 x	18.650	/	2.00	=	11.80 t				
Shear stress	=	11.80 x	1.50	x	10000	/	230	x	567 =		1.36 N/sqmm
100As/ bd	=	100 x	29.44	/	23.00	x	56.70	=	2.26		
Permissible shear stress	=	0.44 N/sqmm		<	1.36 N/sqmm						
Unbalanced shear stress	=	1.36 -	0.442	=	0.91 N/sqmm						
Unbalanced shear force	=	0.91 x	230	x	567	/	10000 =		11.93 t		
Using 8 dia TMT 2 legged stirrups at			150	crs							
Shear Capacity	=	0.87 x	500	x	100.48	x	567 /		150 x		10000
	=	16.5 t	>	12	t						

RESULT :

Beam section	=	**230 X 600 mm**
Use 6/25 dia hsd rods,		**at top**
Use 6/25 dia hsd rods,		**at bottom**
Using 8 dia TMT 2 legged stirrups at		**150 crs**

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DESIGN OF ROOF BEAM

Span considered	=	7.696 m								
Assuming beam size to be		230 x	450	mm	3.8481					
Load /m :					14.8079					
Self wt. of beam	=	2.5 x	0.23	x	0.325	=	0.19 t			
Wall	=	0.115 x	0.23	x	26.000	=	0.69 t			
Slab	=	0.60 x	3.60	/	1.000	=	2.16 t			
							3.03 t			
Effective Depth of beam	=	450 -	25	-	10	=	415 mm			
Breadth of flange	=	5387 / 12 +	230	+	3	x	150			
	=	1129 mm								
Design negative B.M	=	3.03 x	7.696	x	7.696	x	0.100 =	17.97 t-m		
Mu/ fck bdd	=	17.97 x	1.5	x	100000		=	0.340		
		200 x	23	x	41.50	x	41.50			
Ast required	=	0.5105 x	200	x	23.00	x	41.50 =	17.72 sqcm		
			5500							
Use 8/20 dia hsd rods,			**Ast**	=	**18.84 sqcm**		>	**17.72 sqcm**		
Design Positive B.M	=	3.03 x	7.696	x	7.696	x	0.1 =	17.97 t-m		
Mu/ fck bdd	=	17.97 x	1.5	x	100000		=	0.069		
		200 x	112.8945	x	41.50	x	41.50			
Ast required	=	0.1005 x	200	x	112.89	x	41.50 =	18.84 sqcm		
			5000							
Use 8/20 dia hsd rods,			**Ast**	=	**18.84 sqcm**		>	**18.84 sqcm**		
Shear force	=	3.03 x	7.696	/	2.00	=	11.68 t			
Shear stress	=	11.68 x	1.50	x	10000	/	230 x	415 =	1.84 N/sqmm	
100As/ bd	=	100 x	18.84	/	23.00	x	41.50 =	1.97		
Permissible shear stress	=	0.53 N/sqmm		<	1.84 N/sqmm					
Unbalanced shear stress	=	1.84 -	0.525	=	1.31 N/sqmm					
Unbalanced shear force	=	1.31 x	230	x	415	/	10000 =	12.50 t		
Using 8 dia TMT 2 legged stirrups at			125	crs						
Shear Capacity	=	0.87 x	500	x	100.48	x	415 /	125 x	10000	
	=	14.5 t	>	13	t					

RESULT :

Beam section	=	**230 X 450 mm**
Use 8/20 dia hsd rods,		**at top**
Use 8/20 dia hsd rods,		**at bottom**
Using 8 dia TMT 2 legged stirrups at		**125 crs**

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DESIGN OF ROOF BEAM

Span considered	=	9.600 m									
Assuming beam size to be		230 x	600	mm		4.8					
Load /m :						23.04					
Self wt. of beam	=	2.5 x	0.23	x	0.475	=	0.27 t				
Wall	=	2.0 x	0.23	x	3.150	=	1.45 t				
Slab	=	0.71 x	4.00	/	1.000	=	2.85 t				
							4.57 t				
Effective Depth of beam	=	600 -	25	-	8	=	567 mm				
Breadth of flange	=	6720 / 12 +	230	+	3	x	150				
	=	1240 mm									
Design negative B.M	=	4.57 x	9.600	x	9.600	x	0.083 =	35.11 t-m			
Mu/ fck bdd	=	35.11 / 250 x	1.5 / 30	x	100000 / 56.70	x	56.70 =	0.218			
Ast required	=	0.3277 x	250 / 5000	x	30.00	x	56.70 =	27.87 sqcm			
Use 6/25 dia hsd rods,			**Ast**	=	**29.44 sqcm**		>	**27.87 sqcm**			
Design Positive B.M	=	4.57 x	9.600	x	9.600	x	0.09091 =	38.31 t-m			
Mu/ fck bdd	=	38.31 / 250 x	1.5 / 124	x	100000 / 56.70	x	56.70 =	0.058			
Ast required	=	0.0836 x	250 / 5000	x	124.00	x	56.70 =	29.39 sqcm			
Use 6/25 dia hsd rods,			**Ast**	=	**29.44 sqcm**		>	**29.39 sqcm**			
Shear force	=	4.57 x	9.600	/	2.00	=	21.95 t				
Shear stress	=	21.95 x	1.50	x	10000	/	230 x	567 =	2.52 N/sqmm		
100As/ bd	=	100 x	29.44	/	23.00	x	56.70 =	2.26			
Permissible shear stress	=	0.53 N/sqmm		<	2.52 N/sqmm						
Unbalanced shear stress	=	2.52 -	0.525	=	2.00 N/sqmm						
Unbalanced shear force	=	2.00 x	230	x	567	/	10000 =	26.07 t			
Using 10 dia TMT 2 legged stirrups at			150	crs							
Shear Capacity	=	0.87 x	500	x	157.00	x	567 /	150 x	10000		
	=	26 t	>	26	t						

RESULT :

Beam section	=	**230 X 600 mm**
Use 6/25 dia hsd rods,		**at top**
Use 6/25 dia hsd rods,		**at bottom**
Using 10 dia TMT 2 legged stirrups at		**150 crs**

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DESIGN OF ROOF BEAM

Span considered	=	15.650 m									
Assuming beam size to be		450 x	900	mm	7.825						
Load /m :					61.2306						
Self wt. of beam	=	2.5 x	0.45	x	0.775	=	0.87 t				
Wall	=	2.0 x	0.23	x	3.150	=	1.45 t				
Slab	=	0.71 x	6.00	/	1.000	=	4.28 t				
							6.60 t				
Effective Depth of beam	=	900 -	25	-	16	=	859 mm				
Breadth of flange	=	10955 / 12 +	450	+	3	x	150				
	=	1813 mm									
Design negative B.M	=	6.60 x	15.650	x	15.650	x	0.083 =	134.62 t-m			
Mu/ fck bdd	=	134.62 x	1.5	x	100000		=	0.365			
		250 x	30	x	85.90	x	85.90				
Ast required	=	0.5473 x	250 / 5000	x	30.00	x	85.90 =	70.52 sqcm			
Use 12/32 dia hsd rods,			**Ast**	=	**96.46 sqcm**		>	**70.52 sqcm**			
Design Positive B.M	=	6.60 x	15.650	x	15.650	x	0.091 =	147.01 t-m			
Mu/ fck bdd	=	147.01 x	1.5	x	100000		=	0.066			
		250 x	181.2917	x	85.90	x	85.90				
Ast required	=	0.0956 x	250 / 4500	x	181.29	x	85.90 =	82.72 sqcm			
Use 12/32 dia hsd rods,			**Ast**	=	**96.46 sqcm**		>	**82.72 sqcm**			
Shear force	=	6.60 x	15.650	/	2.00	=	51.61 t				
Shear stress	=	51.61 x	1.50	x	10000	/	450 x	859 =	2.00 N/sqmm		
100As/ bd	=	100 x	96.46	/	45.00	x	85.90 =	2.50			
Permissible shear stress	=	0.53 N/sqmm		<	2.00 N/sqmm						
Unbalanced shear stress	=	2.00 -	0.525	=	1.48 N/sqmm						
Unbalanced shear force	=	1.48 x	450	x	859	/	10000 =	57.13 t			
Using 10 dia TMT 4 legged stirrups at			150	crs							
Shear Capacity	=	0.87 x	500	x	314.00	x	859 /	150 x	10000		
	=	78.2 t	>	57	t						

RESULT :

Beam section = **450 X 900 mm**
Use 12/32 dia hsd rods, at top
Use 12/32 dia hsd rods, at bottom
Using 10 dia TMT 4 legged stirrups at **150 crs**

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ROOF SLAB DESIGN

LOAD/ m2

Clear short span	=	4000	mm		4.00						
	=	6000	mm		6.00						
Ly / Lx	=	1.5		<	2.00	DESIGN AS TWO WAY SLAB					
Slab Thickness Assumed	=	150	mm								
Self weight of slab	=	2.4		x	0.150	=	0.36	t			
LIVE LOAD	=					=	0.30	t			
Floor finish	=	0.05		x	2.00	=	0.10	t			
					Total	=	0.760	or	**0.65**	t	
Effective Depth of slab	=	150		-	25	-	5	=	120	mm	
Effective short span	=	4000		+	120	=	4120	mm			
Effective long span	=	6000		+	120	=	6120	mm			

Three edges discontinuous, one short edge continuous

B.M. due to UDL

positive short span B.M	=	0.051	x	4.120	x	4.12	x	0.65	=	0.563	m.t		
positive short span B.M	=	0.037	x	4.120	x	4.12	x	0.65	=	0.408	m.t		
Short span													
Mu / bdd	=	0.56	x	1.50	x	100000			=	0.60			
		9.807	x	100	x	12.00	x	12.0	=				
Ast Required	=	0.3600	x	1.00	x	100.00	x	12.00	=	4.3	sqcm		
					100								
Use 10 dia hsd rods at	**150**	**crs**	**Ast**		=	**78.50**	**/**	**15.00**	=	**5.2**	**Sqcm**	**>**	**4.3 Sqcm**

Long span

Mu / bdd	=	0.41	x	1.50	x	100000			=	0.43			
		9.807	x	100.00	x	12.00	x	12.0	=				
Ast Required	=	0.3000	x	1.00	x	100.00	x	12.00	=	3.6	sqcm		
					100								
Use 10 dia hsd rods at	**150**	**crs**	**Ast**		=	**78.50**	**/**	**15.00**	=	**5.2**	**Sqcm**	**>**	**3.6 Sqcm**

RESULT :

Use Thickness of Slab	=	**150**	**mm**	
Main rods	=	**Use 10 dia hsd rods at**	**150**	**crs**
Distributors	=	**Use 10 dia hsd rods at**	**150**	**crs**

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ROOF SLAB DESIGN

LOAD/ m2

Clear short span	=	4000 mm		4.00						
	=	4800 mm		4.80						
Ly / Lx	=	1.2	<	2.00	DESIGN AS TWO WAY SLAB					
Slab Thickness Assumed	=	115 mm								
Self weight of slab	=	2.4	x	0.115	=	0.276	t			
LIVE LOAD	=				=	0.30	t			
Floor finish	=	0.05	x	2.00	=	0.10	t			
				Total	=	0.676	or	0.65	t	
Effective Depth of slab	=	115	-	25	-	4	=	86	mm	
Effective short span	=	4000	+	86	=	4086 mm				
Effective long span	=	4800	+	86	=	4886 mm				

Three edges discontinuous, one short edge continuous

B.M. due to UDL

positive short span B.M	=	0.051 x		4.086	x	4.086	x	0.65	=	0.553 m.t
positive short span B.M	=	0.037 x		4.086	x	4.086	x	0.65	=	0.402 m.t
Short span										
Mu / bdd	=	0.55 x		1.50	x	100000			=	1.14
		9.807 x		100	x	8.60	x	8.6	=	
Ast Required	=	0.3850 x		1.00	x	100.00	x	8.60	=	3.3 sqcm
					100					
Use 8 dia hsd rods at	**150**	**crs**	**Ast**		=	**50.24**	**/**	**15.00**	=	**3.3 Sqcm > 3.3 Sqcm**

Long span

Mu / bdd	=	0.40 x		1.50	x	100000			=	0.83
		9.807 x		100.00	x	8.60	x	8.6	=	
Ast Required	=	0.3100 x		1.00	x	100.00	x	8.60	=	2.7 sqcm
					100					
Use 8 dia hsd rods at	**150**	**crs**	**Ast**		=	**50.24**	**/**	**15.00**	=	**3.3 Sqcm > 2.7 Sqcm**

RESULT :

Use Thickness of Slab	=	**115 mm**		
Main rods	=	**Use 8 dia hsd rods at**	**150**	**crs**
Distributors	=	**Use 8 dia hsd rods at**	**150**	**crs**

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52/141, Flat C2, PJM Apartments, East Vanniar Street,
West K.K. Nagar, Chennai-600 078.
Mobile: 9884400273, Email: mpunithan@yahoo.com

ROOF SLAB DESIGN

LOAD/ m2

Clear short span	=	3300 mm		3.30						
	=	4800 mm		4.80						
Ly / Lx	=	1.45455	<	2.00	DESIGN AS TWO WAY SLAB					
Slab Thickness Assumed	=	115 mm								
Self weight of slab	=	2.4	x	0.115	=	0.276	t			
LIVE LOAD	=				=	0.30	t			
Floor finish	=	0.05	x	2.00	=	0.10	t			
				Total	=	0.676	or	0.65	t	
Effective Depth of slab	=	115	-	25	-	4	=	86	mm	
Effective short span	=	3300	+	86	=	3386 mm				
Effective long span	=	4800	+	86	=	4886 mm				

Three edges discontinuous, one short edge continuous

B.M. due to UDL

positive short span B.M	=	0.051 x		3.386	x	3.386	x	0.65	=	0.38 m.t
positive short span B.M	=	0.037 x		3.386	x	3.386	x	0.65	=	0.276 m.t

Short span

Mu / bdd	=	0.38 x		1.50	x	100000			=	0.79
		9.807 x		100	x	8.60	x	8.6	=	
Ast Required	=	0.3450 x		1.00	x	100.00	x	8.60	=	3.0 sqcm
					100					
Use 8 dia hsd rods at	**150**	**crs**	**Ast**		=	**50.24**	**/**	**15.00**	=	**3.3 Sqcm > 3.0 Sqcm**

Long span

Mu / bdd	=	0.28 x		1.50	x	100000			=	0.57
		9.807 x		100.00	x	8.60	x	8.6	=	
Ast Required	=	0.3110 x		1.00	x	100.00	x	8.60	=	2.7 sqcm
					100					
Use 8 dia hsd rods at	**150**	**crs**	**Ast**		=	**50.24**	**/**	**15.00**	=	**3.3 Sqcm > 2.7 Sqcm**

RESULT :

Use Thickness of Slab	=	115 mm		
Main rods	=	**Use 8 dia hsd rods at**	**150**	**crs**
Distributors	=	**Use 8 dia hsd rods at**	**150**	**crs**

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RESIDENTIAL FLATS

Design of foundation using M25 grade concrete,

load	=	70	t											
FOUNDATION SIZE	=	**9.0**	**X**	**8.0**	Ft									
Column size	=	450	x	300	mm									
Soil pressure	=	11.00	t /sq.m											
Foundation area required	=	6.36	sqm											
Assuming size of fodn. to be		2.74	x	2.44	=	6.69	sqm							
Depth of footing at face	=	500	mm											
Depth of footing at end	=	0	mm			2443.20		1221.60						
Effective depth	=	445	mm			1988.40		994.20						
Cantilever B.M	=	11.00	x	1.222	x	1.222	x	0.50						
	=	8.21	t - m											
Mu / (bdd fck)	=	8.21	x	1.5		100000	/(	250.00	x	44.50	x	44.5	x	100)
	=	0.025												
Area of steel required			=	0.03606	x	44.50	x	250.00	x	100.00	/	5500		
			=	7.29	sq-cm.									
use 12 tor rods at	**150**	**crs**	**Ast**	**=**	**113.0**	**/**	**15.00**	**=**	**7.54**	**sq-cm.**	**>**	**7.29**	**sq-cm.**	

Shear force on footing

Shear to be checked at distance effective depth from column face

Shear force	=	11	x	0.7766	=	8.54	t			
Shear stress	=	8.54	x	1.50	x	10000	/(	1000.00	x	445)
	=	0.288	N/sq-mm.							
100 As / bd	=	7.54	/	44.50						
	=	0.16935								
Permissible shear stress	=	**0.360**	**N/sq-mm.**	**=**	**0.288**	**N/sq-mm.**				

RESULT :

Foundation size	=	**2743**	**x**	**2438**	**m**	**9.00**	**X**	**8.00** Ft
Overall depth	=	**0**	**+**	**500**	**=**	**500**	**mm**	
Reinforcement	=	**use 12 tor rods at**				**150**	**crs**	

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load	=	**90**	**t**														
FOUNDATION SIZE	=	**10.0**	**X**	**9.0**	Ft												
Column size	=	450	x	300	mm												
Soil pressure	=	11.00	t /sq.m														
Foundation area required	=	8.18	sqm														
Assuming size of fodn. to be		3.05	x	2.74	=	8.36	sqm										
Depth of footing at face	=	600	mm														
Depth of footing at end	=	0	mm			2748.00		1374.00									
Effective depth	=	545	mm			2293.20		1146.60									
Cantilever B.M	=	11.00	x	1.374	x	1.374	x	0.50									
	=	10.38	t - m														
Mu / (bdd fck)	=	10.38	x	1.5		100000	/(	250.00	x	54.50	x	54.5	x	100	)		
	=	0.021															
Area of steel required			=	0.03041	x	54.50	x	250.00	x	100.00	/	5500					
			=	7.53	sq-cm.												
use 12 tor rods at	**150**	**crs**		**Ast**		**=**	**113.0**	**/**	**15.00**	**=**	**7.54**	**sq-cm.**	**>**	**7.53**	**sq-cm.**		

Shear force on footing

Shear to be checked at distance effective depth from column face

Shear force	=	11	x	0.829	=	9.12	t				
Shear stress	=	9.12	x	1.50	x	10000	/(	1000.00	x	545)	
	=	0.251	N/sq-mm.								
100 As / bd	=	7.54	/	54.50							
	=	0.13828									
Permissible shear stress	=	**0.360**	**N/sq-mm.**		**=**	**0.251**	**N/sq-mm.**				

RESULT :

Foundation size	=	**3048**	**x**	**2743**	**m**		**10.00**	**X**	**9.00**	**Ft**
Overall depth	=	**0**	**+**	**600**	**=**	**600**	**mm**			
Reinforcement	=	**use 12 tor rods at**				**150**	**crs**			

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load	=	**133**	**t**														
FOUNDATION SIZE	=	**12.0**	**X**	**11.0**	Ft												
Column size	=	450	x	300	mm												
Soil pressure	=	11.00	t /sq.m														
Foundation area required	=	12.09	sqm														
Assuming size of fodn. to be		3.66	x	3.35	=	12.26	sqm										
Depth of footing at face	=	650	mm														
Depth of footing at end	=	0	mm			3357.60		1678.80									
Effective depth	=	595	mm			2902.80		1451.40									
Cantilever B.M	=	11.00	x	1.679	x	1.679	x	0.50									
	=	15.50	t - m														
Mu / (bdd fck)	=	15.50	x	1.5		100000	/(	250.00	x	59.50	x	59.5	x	100	)		
	=	0.026															
Area of steel required			=	0.03809	x	59.50	x	250.00	x	100.00	/	5500					
			=	10.30	sq-cm.												
use 16 tor rods at		**150**	**crs**		**Ast**	=	**201.0**	/	**15.00**	=	**13.40**	**sq-cm.**	>	**10.30**	**sq-cm.**		

Shear force on footing

Shear to be checked at distance effective depth from column face

Shear force	=	11	x	1.0838	=	11.92	t				
Shear stress	=	11.92	x	1.50	x	10000	/(	1000.00	x	595	)
	=	0.301	N/sq-mm.								
100 As / bd	=	13.40	/	59.50							
	=	0.22517									
Permissible shear stress	=	**0.360**	**N/sq-mm.**	=	**0.301**	**N/sq-mm.**					

RESULT :

Foundation size	=	**3658**	**x**	**3353**	**m**		**12.00**	**X**	**11.00** Ft
Overall depth	=	**0**	**+**	**650**	**=**	**650**	**mm**		
Reinforcement	=	**use 16 tor rods at**				**150**	**crs**		

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RESIDENTIAL FLATS

COLUMN A3

No. of floors	=	1	+	5	=	6						
Foundation depth	=	1.8	m									
Ground to floor level	=	1	m									
GFloor to floor height	=	3.05	m									
FFloor to floor height	=	3.05	m									
Thickness of slab assumed	=	0.115	m									
Slab loading t/ sqm	=	0.6	t									
Breadth of beams	=	0.23	m									
Depth of beams assumed	=	0.41	m									
Parapet wall height	=	1	m									
Plinth beams assumed	=	0.2	m									
Total height of wall	=	17.2	m									
Total height of column	=	22.1	m									
Size of column	=	0.45	x	0.3	m							

Total load:

Due to slab	=	6	x	1.73	x	2.34	x	0.6			=	14.6 t
Wall	=	2	x	0.23	x	4.08	x	17.2			=	32.2 t
Roof beams	=	2.5	x	0.23	x	0.29	x	0.2	x	6	=	0.21 t
Plinth beam	=	2.5	x	0.23	x	0.2	x	0.2	x	1	=	0.02 t
Self wt	=	2.5	x	0.45	x	0.3	x	22.1			=	7.46 t
												54.4 t
						Total					=	**54.4 t**

Using size of column to be	=	**450**	**X**	**300**	**mm**						
With 8/16 + 0/12 Rods, Ast	=	8	x	113	+	3	x	113	=	1243	sqmm
Capacity of column	=	450	x	300	x	6	+	1243	x	190	
	=	**105 t**		**>**		**68 t**					
Minimum Reinforcement	=	0.008	x	45	x	30	=	10.8	sqcm	<	12.4 sqcm
% of Reinforcement	=	1243	/	45	x	30	=	0.92	<	4	

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COLUMN N4

No. of floors	=	1	+	5	=	6						
Foundation depth	=	3	m									
Ground to floor level	=	1	m									
GFloor to floor height	=	3.05	m									
FFloor to floor height	=	3.05	m									
Thickness of slab assumed	=	0.115	m									
Slab loading t/ sqm	=	0.6	t									
Breadth of beams	=	0.23	m									
Depth of beams assumed	=	0.25	m									
Parapet wall height	=	1	m									
Plinth beams assumed	=	0.21	m									
Total height of wall	=	18.1	m									
Total height of column	=	23.3	m									
Size of column	=	0.45	x	0.3	m							

Total load:

Due to slab	=	6	x	3.37	x	1.64	x	0.6			=	19.8	t
Wall	=	2	x	0.23	x	4.19	x	18.1			=	34.9	t
Roof beams	=	2.5	x	0.23	x	0.14	x	0.21	x	6	=	0.1	t
Plinth beam	=	2.5	x	0.23	x	0.21	x	0.21	x	1	=	0.03	t
Self wt	=	2.5	x	0.45	x	0.3	x	23.3			=	7.86	t
												62.7	t
						Total					=	**62.7**	**t**

Using size of column to be	=	**450**	**X**	**300**	**mm**						
With 8/16 + 4/12 Rods, Ast	=	8	x	201	+	4	x	113	=	2060	sqmm
Capacity of column	=	450	x	300	x	6	+	2060	x	190	
	=	**120**	**t**	>		**94**	**t**				
Minimum Reinforcement	=	0.008	x	45	x	30	=	10.8	sqcm	<	20.6 sqcm
% of Reinforcement	=	2060	/	45	x	30	=	1.53	<	4	

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COLUMN N4

No. of floors	=	1	+	5	=	6						
Foundation depth	=	1.8	m									
Ground to floor level	=	1	m									
GFloor to floor height	=	3.05	m									
FFloor to floor height	=	3.05	m									
Thickness of slab assumed	=	0.115	m									
Slab loading t/ sqm	=	0.6	t									
Breadth of beams	=	0.23	m									
Depth of beams assumed	=	0.38	m									
Parapet wall height	=	1	m									
Plinth beams assumed	=	0.29	m									
Total height of wall	=	17.3	m									
Total height of column	=	22.1	m									
Size of column	=	0.45	x	0.3	m							

Total load:

Due to slab	=	6	x	3.73	x	3.96	x	0.6			=	53	t
Wall	=	2	x	0.23	x	5.72	x	17.3			=	45.5	t
Roof beams	=	2.5	x	0.23	x	0.27	x	0.29	x	6	=	0.27	t
Plinth beam	=	2.5	x	0.23	x	0.29	x	0.29	x	1	=	0.05	t
Self wt	=	2.5	x	0.45	x	0.3	x	22.1			=	7.46	t
												106	t
						Total					=	**106**	**t**

Using size of column to be	**=**	**450**	**X**	**300**	**mm**							
With 8/25 Rods, Ast	=	8	x	314	+	4	x	113	=	2964	sqmm	
Capacity of column	=	450	x	300	x	6	+	2964	x	190		
	=	**137**	**t**	**>**		**133**	**t**					
Minimum Reinforcement	=	0.008	x	45	x	30	=	10.8	sqcm	<	29.6	sqcm
% of Reinforcement	=	2964	/	45	x	30	=	2.2	<	4		

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MURUGU CONSULTANT

9A/10, Ist Floor, Umapathy Street Ext.
West Mambalam, Chennai – 600 033.

PH: 91 – 9884400273

To
The Principal Cheif Engineer,
Greater Chennai Corporation,
Rippen Building, Chennai.

Dated : 08 - 04 - 2026

Dear Sir

Sub : Construction of Residential Building stilt + 5 floors, 20 dwelling unit in single block in Ashok Nagar – Stability certificate

The Residential buildings in the address Block No. H20 & H21, Monthope Colony, 46th Street/8th Avenue, Ashok Nagar, Chennai- 600 083, comprised in Old S.No. 146 Part, 148/1A Part, 149/1 Part, 149/2 Part & 150 Part, T.S No. 3/14, Block-48, of Kodambakkam Part-2 Village, Mambalam Taluk Within the limits of Greater Chennai Corporation, Old Division No – 132, New Division No. N135, Zone 10, is designed as per IS : 456, 2000, IS 1893 (part 1) : 2002 for seismic zone 3 and loading has been taken from IS 875 part 1 & 2 for dead load, live load. & seismic load and the SBC of the soil is 11 t /sqm found structurally sound & safe.

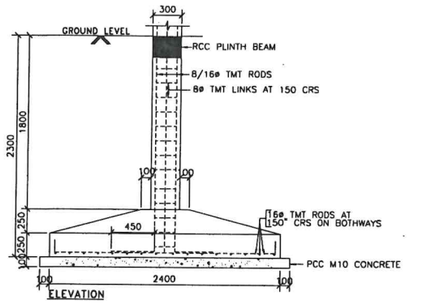
Thanking you

Yours truly,

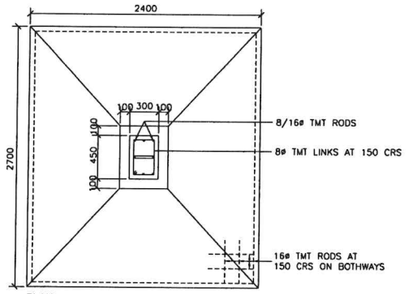
For MURUGU CONSULTANT

M.PUNITHAN, B.Sc., M.I.E
M-1371961

M. PUNITHAN B.Sc.,M.I.E.
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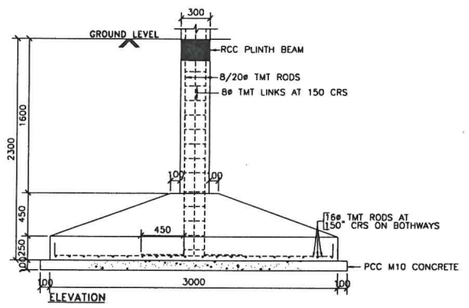


ELEVATION

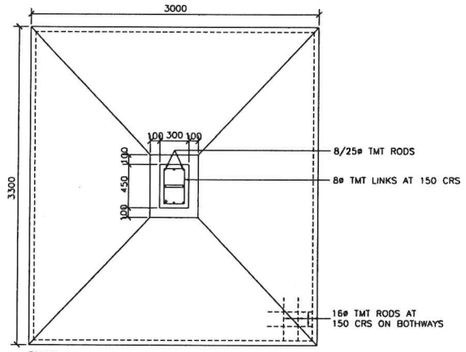


PLAN

DETAIL OF FOUNDATION FOR COLUMNS C1

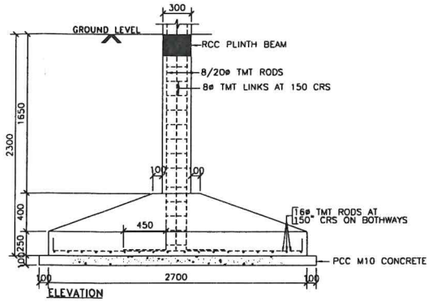


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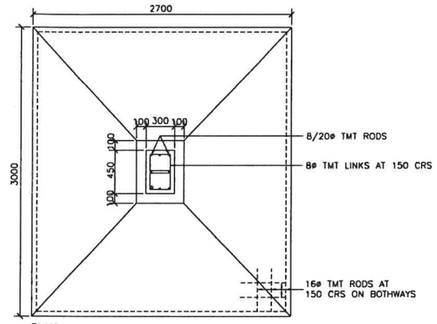


PLAN

DETAIL OF FOUNDATION FOR COLUMNS C3

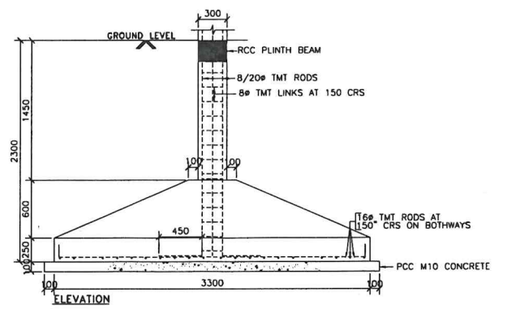


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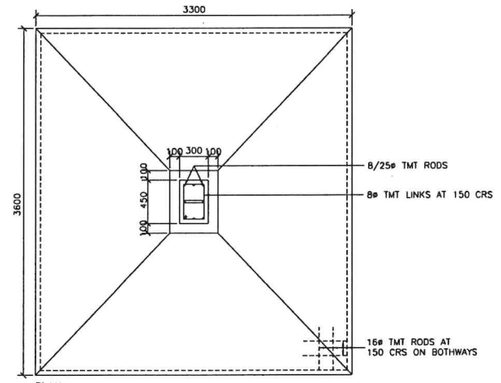


PLAN

DETAIL OF FOUNDATION FOR COLUMNS C2

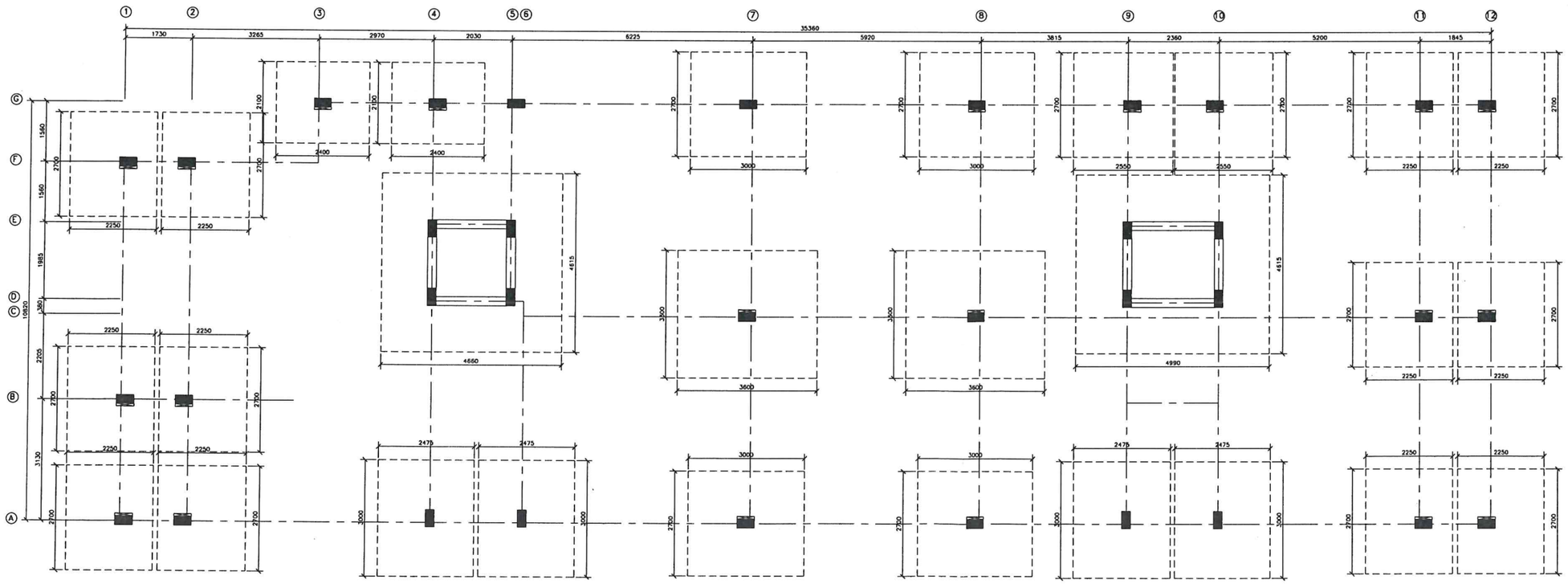


ELEVATION



PLAN

DETAIL OF FOUNDATION FOR COLUMNS C3



COLUMN & FOUNDATION LAYOUT

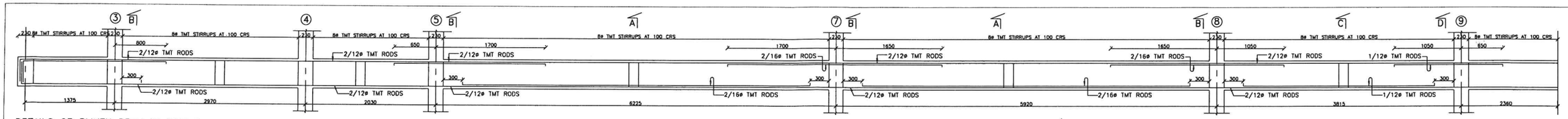
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NOTES:

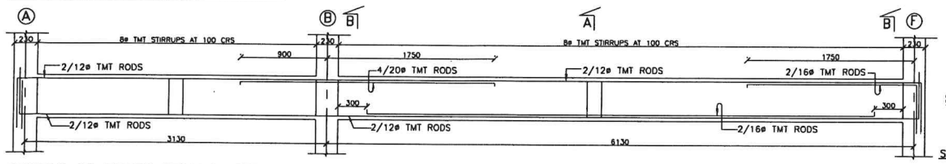
1. ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE STATED.
2. ALL RCC WORKS TO BE WITH M30 CONCRETE AND TO BE EXECUTED AS PER IS : 456 – 2000, USING 20mm GRADED GRANITE METAL, WATER CEMENT RATIO NOT EXCEEDING 0.45, HIGH YIELD STRENGTH DEFORMED BARS (TOR SHALL CONFORM TO IS: 1786 – 2008 (Fe 500).
3. LAP LENGTH FOR ALL BARS TO BE 45 TIMES THE DIAMETER.
4. CONCRETE COVER FOR ALL MAIN RODS IN FOUNDATION AND PLINTH BEAMS TO BE 40 UNLESS OTHERWISE STATED.

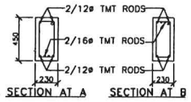
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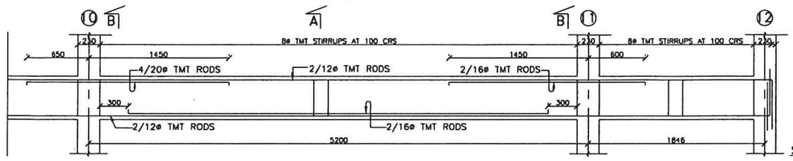
CONSULTANTS	MURUGU CONSULTANT 9A/10, UMAPATHY STREET EXT. W.MAMBALAM, CHENNAI – 33. Ph No.9884400273.
	M/S HORMONY HOMES 12TH AVENUE, ASHOK NAGAR CHENNAI – 600 083.
	PROPOSED CONSTRUCTION OF RESIDENTIAL BUILDING IN SANTHOPE COLONY.
	LAYOUT AND RCC DETIALS OF PLINTH BEAM
ARCHITECT	diastyle NO. 63/55, 12TH AVENUE, ASHOK NAGAR CHENNAI – 600 083. PHONE : 65853376, 65359407.
	SCALE : 1 : 50, 1 : 25
	DRG.No: MC-134/2025 SHEET 1

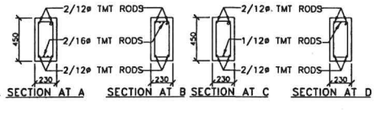


DETAILS OF PLINTH BEAM IN ROW G

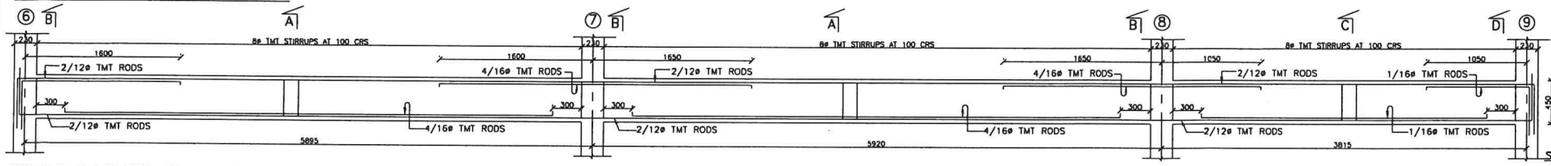


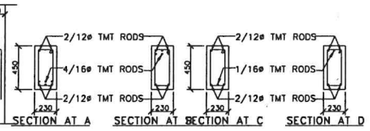




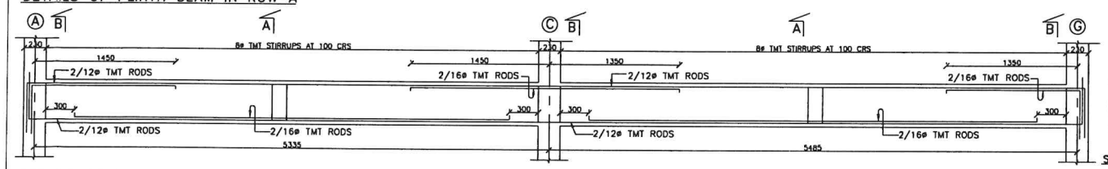


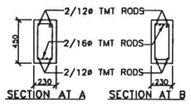
DETAILS OF PLINTH BEAM IN ROW 1



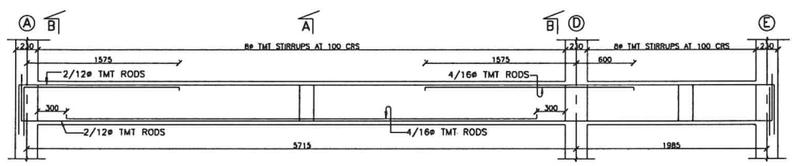


DETAILS OF PLINTH BEAM IN ROW A

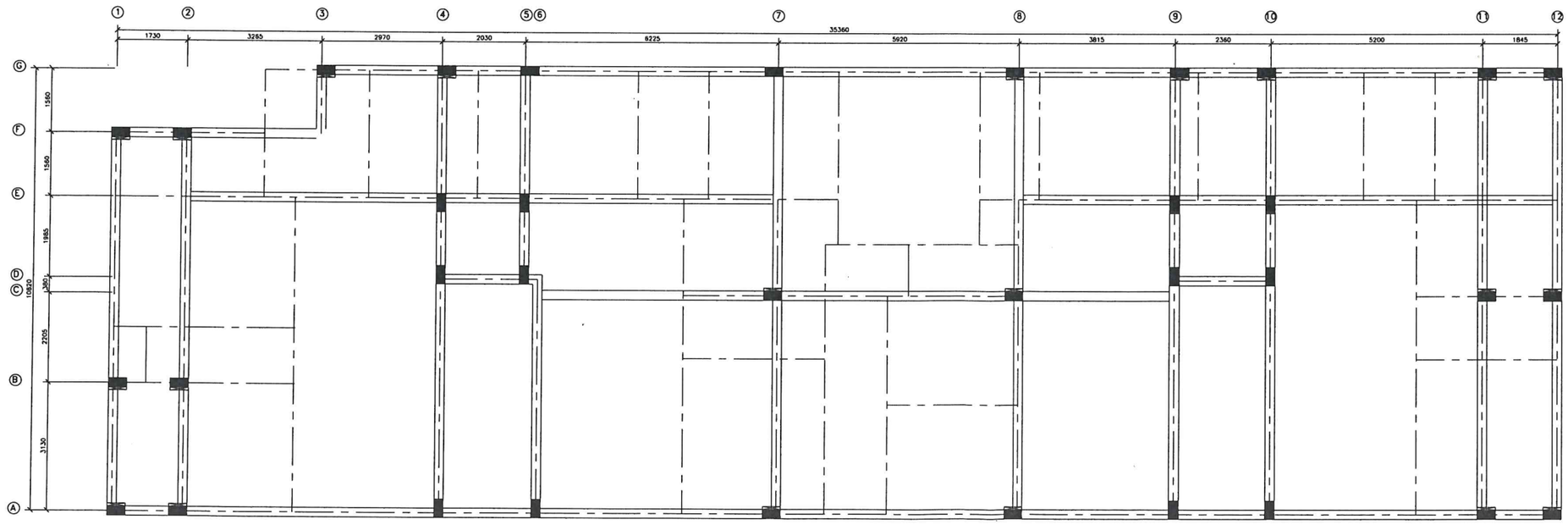




DETAILS OF PLINTH BEAM IN ROW 12



DETAILS OF ROOF BEAM IN ROW 5



PLINTH BEAM LAYOUT

M. PUNITHAN B.Sc.,M.I.E.
Registered Structural Engineer Grade-II
CMDA Reg. No. SE/GR-II/19/04/044
Renewed 10-04-2024. Valid upto 09-04-2029
52/141, Flat C2, PJM Apartments, East Vanniar Street,
West K.K. Nagar, Chennai-600 078.
Mobile: 9884400273, Email: mpunithan@yahoo.com

NOTES:

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ALT	DATE	DESCRIPTION	DRAWN	CHECKED	APPROVED
0	23/04/25	AS DRAWN			

CONSULTANTS: MURUGU CONSULTANT
8A/10, UMAPATHY STREET EXT. W.MAMBALAM, CHENNAI – 33. Ph No.9884400273.

M/S HORMONY HOMES
12TH AVENUE, ASHOK NAGAR
CHENNAI – 600 083.

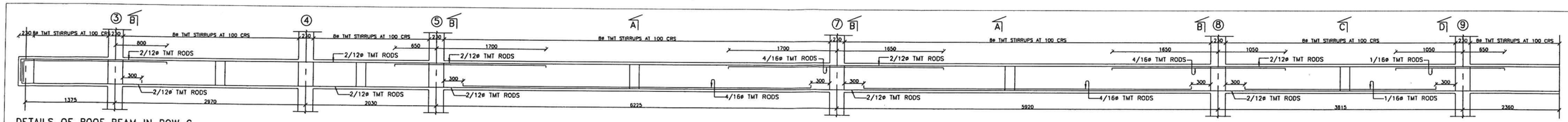
PROPOSED CONSTRUCTION OF RESIDENTIAL BUILDING IN MANTHOPE COLONY.

LAYOUT AND RCC DETIALS OF PLINTH BEAM

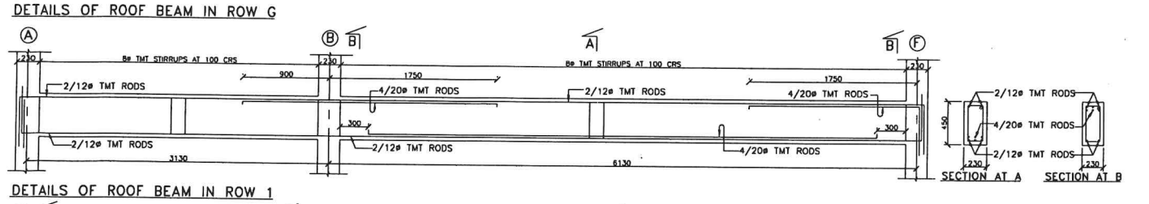
ARCHITECT: diastyle
NO. 65/55, 12TH AVENUE, ASHOR NAGAR
CHENNAI – 600 083.
PHONE : 60833326, 65359407.

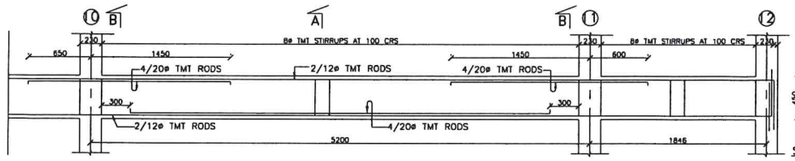
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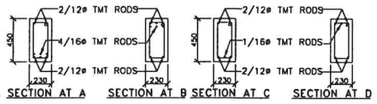
DRG.No: MC-134/2025 SHEET 2



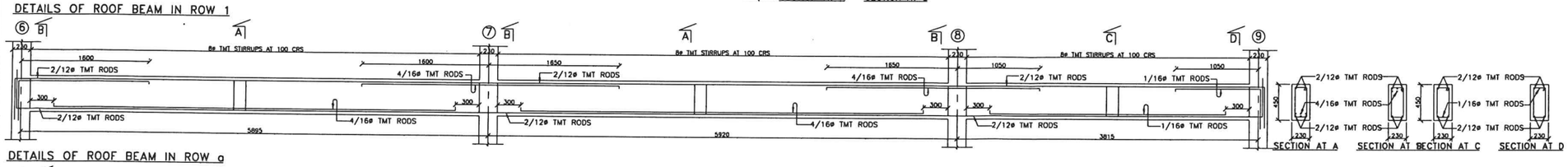
DETAILS OF ROOF BEAM IN ROW G



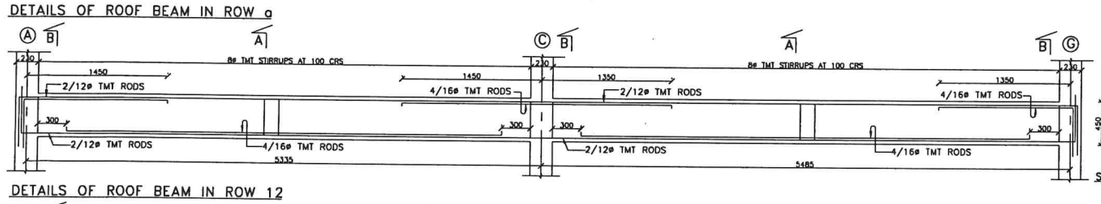


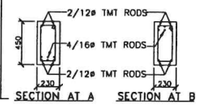


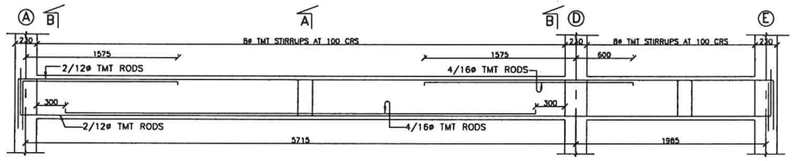
DETAILS OF ROOF BEAM IN ROW 1

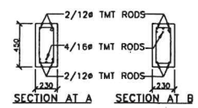


DETAILS OF ROOF BEAM IN ROW a

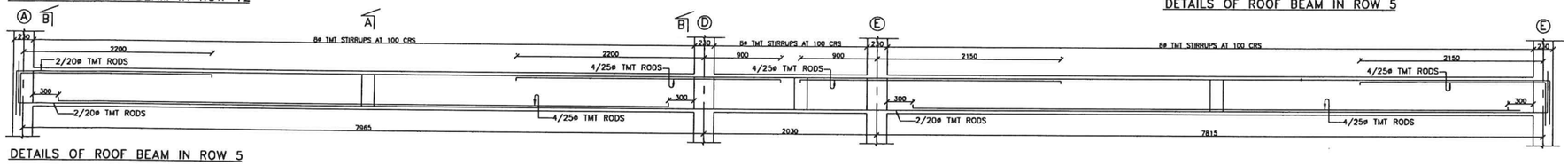




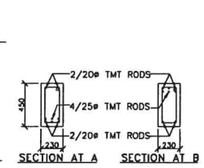




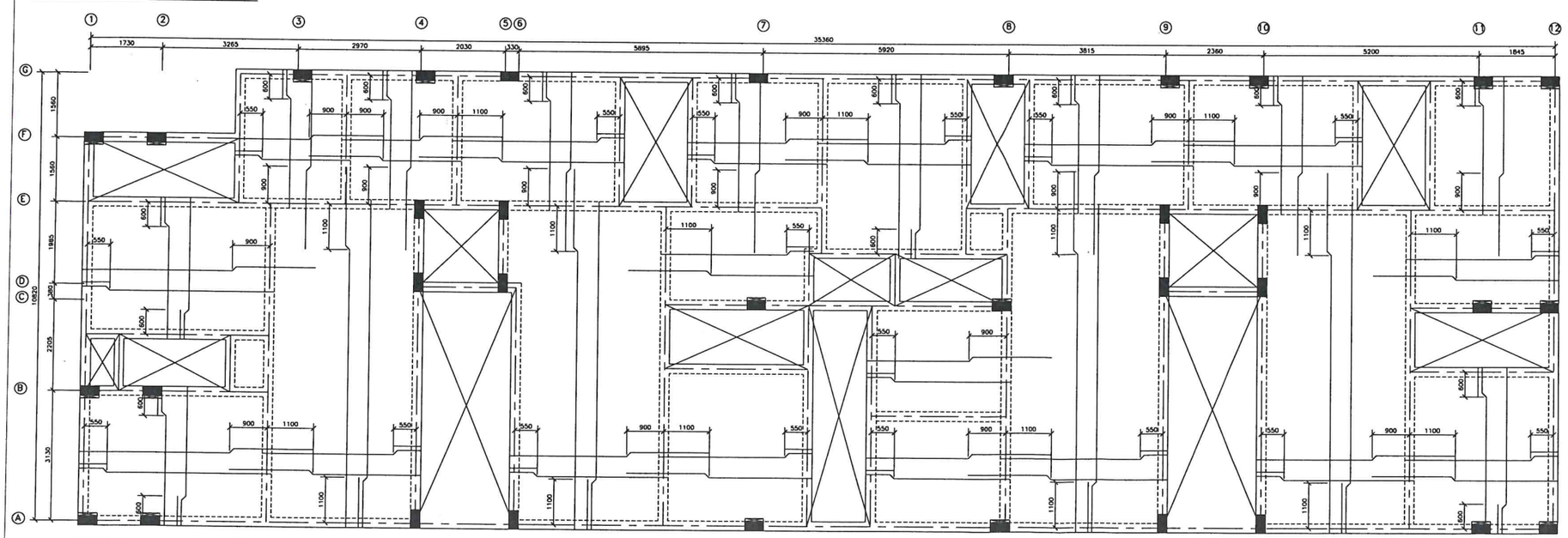
DETAILS OF ROOF BEAM IN ROW 12



DETAILS OF ROOF BEAM IN ROW 5



DETAILS OF ROOF BEAM IN ROW 5



ROOF SLAB DETAILS 4.5" THICK
(8ø TMT RODS AT 6" CRS BOTHWAYS)

M. PUNITHAN B.Sc.,M.I.E.
Registered Structural Engineer Grade-II
CMDA Reg. No. SE/GR-II/19/04/044
Renewed 10-04-2024. Valid upto 09-04-2029
52/141, Flat C2, PJM Apartments, East Vanniar Street,
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Mobile: 9884400273, Email: mpunithan@yahoo.com

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0	23/04/25	AS Drawn			

CONSULTANTS	MURUGU CONSULTANT 9A/10, UMAPATHY STREET EXT. W.MAMBALAM, CHENNAI – 33. Ph No.8884400273.
CLIENT	M/S HORMONY HOMES 12TH AVEHUE, ASHOK NAGAR CHENNAI – 600 083.
ARCHITECT	diastyle NO. 83/35, 12TH AVEHUE, ASHOK NAGAR CHENNAI – 600 083. PHONE : 65853328, 65359407.
	PROPOSED CONSTRUCTION OF RESIDENTIAL BUILDING IN WEST MAMBALAM.
	LAYOUT AND RCC DETIALS ROOF BEAM & SLAB
	SCALE : 1 : 50, 1 : 25
	DRG.No: MC-134/2025 SHEET 3

THE LANDLENS SUITE

Eight ways to know a piece of land before you pay for it.

This report covers one part of the picture. These cover the rest, drawn from the same government records, for any property in Tamil Nadu.

LandLens One
Ask anything about a property and get an answer grounded in live government records, with the sources shown.
/landlens/one

Verify My Land
Every record check run on one property and returned as a single report.
/landlens/verify-my-land

Legal Opinion
An advocate's written opinion on the title, built on the records we pull for it.
/landlens

One Map
Every layer on one map: the parcel, its boundary, zoning, water bodies and what sits around it.
/landlens/one/map

PriceMap
What land actually changed hands for around this plot, taken from the registered deeds.
/landlens/pricemap

Build Scope
What you are allowed to build here: floor space, height, coverage and setbacks.
/landlens/build-scope

Property Tracker
Watch a property and hear about it when something new is registered against it.
/landlens/property-tracker

Deal Room
Run the transaction in one place: documents, stamping, signatures and the parties.
/landlens/deal-room

FREE TOOLS · NO ACCOUNT NEEDED

Twenty-four checks you can run yourself, at no cost.

Records and verification

The registers themselves, read for you.

- Encumbrance Certificate
- Patta and Chitta
- Field Measurement Sketch
- EC Analyzer
- Bank charge check
- Court case search
- Seller background check
- Approval check by survey

Value and duty

What it is worth and what it costs to register.

- Guideline value by street
- Guideline value by survey
- Stamp duty and registration fees
- Capital gains
- Floor space index
- Area unit converter

Zoning and land use

What the plan says can happen on this land.

- Land use zone finder
- CMDA land use check
- Coastal regulation zone
- Master plan check
- Water bodies finder
- Airport height clearance

Registers and directories

Who holds it, and which office keeps the record.

- Find your registering office
- Temple property check
- Waqf property check
- RERA register

Every free tool
/tools

Services we run for you
/services

The LandLens suite
/landlens/one

Verified RealEstate™

Every claim in this document traces back to a government record.

- Legal opinions, due diligence and title verification
- Building plan, layout and sub-division approvals
- Completion and occupancy certificates, regularisation
- Escrow, negotiation and NRI property management
- Land survey, soil testing, architecture and construction

Verified RealEstate
3/350/2 Mettukuppam,
OMR Thoraipakkam,
Chennai 600 097, India
hello@verified.realestate
+91 9996 12 9996

Prepared from the registers named on the page each finding sits on. Where a register could not be read, this document says so in its place rather than leaving the gap to be read as a clean result.