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REPORT

ON

SOIL INVESTIGATION FOR CONSTRUCTION OF RESIDENTIAL APARTMENT BUILDING AT

Door No 112,Plot No:31,T.S.No:27,Ward No:AK,
Block No:4,Crawford Colony Main Road,
Crawford,K.Abhishekapuram Village,
Zone-IV(Ponmalai Zone),Trichy-620012

Client

Mr. Fahd Habibullah,
No. 112, Crawford Colony Main Road,
Trichy – 620 012.

By

Time

Quality is Excellence

TIME INSTITUTE FOR MATERIALS TESTING
NO.25, SANJEEVI NAGAR,
TRICHY – CHENNAI BYEPASS ROAD,
TIRUCHIRAPPALLI – 620 002.
Ph: 0431 2401326

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

AUGUST 2022

SUMMARY

Project	:	Construction of Residential Apartment Building at Crawford, Trichy
Project Reference	:	Time /17/ Soil Test / August 2022
Owner	:	Mr. Fahd Habibullah, No. 112, Crawford Colony Main Road, Trichy – 620 012.
Type of Structure	:	R.C.C. Framed construction
Location	:	Crawford, Trichy
Scope	:	Geotechnical investigation for the entire area of construction
Test conducted	:	Two explanatory bore holes up to 4.5m depth were made and conducted standard penetration tests at every 1.5m depth. The undisturbed soil samples were collected at every 1.5m depth for laboratory test.
FINDINGS		
Soil at shallow depth	:	Gravels exist up to 1.8m depth
Hard stratum	:	Soft rock exists between 3.5m to 4.5m depth in Bore hole 1 & 2m to 4.5m depth in Bore Hole 2
Ground water table	:	Ground water table was not encountered during the investigation up to 4.5m depth
FOUNDATION	:	Based on the existing soil condition, it is recommended to provide foundation at 3m depth.

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

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Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

SOIL INVESTIGATION

Krishnaswami, M.E., (Struct. Engg.), Ph.D.,
Former Faculty, NIT, Trichy

Time Institute for Materials Testing

टाइम इन्स्टिट्यूट चीज़े शोध केलिए

Soil & Structural Consultant, Construction Materials Testing Lab

Quality is Excellence

August 09, 2022

To

Mr. Fahd Habibullah,
No. 112, Crawford Colony Main Road,
Trichy – 620 012.

REPORT ON SOIL INVESTIGATION

(Ref: TIME/17/ SOIL TEST/ AUGUST 2022)

Sub: Soil Investigation for Construction of Residential Apartment Building at Crawford, Trichy – reg.,

Ref: Based on letter dated: 06.08.2022

1.1 Introduction:

It is proposed to construct a **Residential Apartment Building at Crawford, Trichy.** A detailed soil investigation was suggested for estimating the safe bearing capacity of the soil, so that suitable type of foundation can be decided for the proposed structure. Time Institute for Materials Testing (TIME) has been requested to undertake the soil investigation at the proposed site. The study was carried out by TIME between **07.08.2022 & 08.08.2022.** This report presents complete investigation data and discusses the results to recommend the bearing capacity values at different depths for the proposed structure.

Section 2 describes the soil profile and bore log details of two explanatory bore holes. Section 3 of this report gives the Grain size distribution curves and its analysis. Section 4 of this report presents the calculated safe Bearing capacity values of soil at various depths. Section 5 of this report reviews the test results and the recommended safe bearing capacity values for design of foundation. Section 6 of this report presents the Soil & Rock sample photos. Section 7 of this report presents the Site Photos.

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

1.2 Investigation Programme, Procedure and Data:

Investigation was carried out in two locations at the site through exploratory bore holes. Soil samples were collected from each bore hole at different depths for conducting laboratory test. Standard Penetration test was conducted at regular intervals and 'N' values were recorded. Undisturbed soil samples were collected and were preserved and transported to the laboratory for detailed identification tests. Based on the field and laboratory tests on the samples collected, the results are furnished in this report.

An exploratory bore hole was advanced from the existing ground level using truck mounted rotary drilling techniques supplemented by Betonies mud circulation. This drilling procedure with mud circulation is found most suitable for making exploratory bore hole. The mud circulation was employed through the drill rods and letting it out though the side jets provided in the cutting tool thus preventing any disturbance at the bore hole bottom. Mud circulation was used to stabilize the sides and the bottom of the bore hole, and then to bring the soil cuts to the surface. It is important to note that the mud jet is not used to cut the soils as in the case of wash boring technique. Use of drilling mud will also help in preventing the disturbance to the soil at the bore hole bottom during drilling operations. Diameter of the bore hole is about 150mm.

Bore hole was always kept full with the drilling mud so that a positive head is maintained in the bore hole thus preventing any disturbance to the soil within the test zone.

The field tests included Standard penetration test, Disturbed and undisturbed soil sampling, identification of different soil layers, Ground water table observation, complete logging of the bore hole, etc.

Laboratory investigation consisted of classification tests like grain size distribution analysis, determination of specific gravity etc. Unconfined Compression test and Direct shear was conducted on preserved soil samples. All the field and laboratory tests were conducted according to the procedures stipulated in relevant IS codes.

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

1.3 Review of Field and Laboratory Results:

Sub soil Profile

The original ground level in the presently investigated area was plain terrain.

Bore Hole 1:

The soil below the existing ground level is filled up soil. This layer is extended up to 0.4m depth. Red soil with gravels mixed exists from 0.4m to 1.1m depth. Gravels exist from 1.1m to 1.8m depth. Disintegrated rock particles exist from 1.8m to 3.5m depth. Below which soft rock exists from 3.5m to 4.5m depth from the existing ground level. The soil stratification and its variation under present investigation are available in bore log details. The 'N' value is progressively increasing towards depth due to the soil stratification.

Bore Hole 2:

The soil below the existing ground level is filled up soil. This layer is extended up to 0.3m depth. Red soil with gravels mixed exists from 0.3m to 0.9m depth. Gravels exist from 0.9m to 2m depth. Below which Disintegrated rock particles exist from 2m to 4.5m depth from the existing ground level. The soil stratification and its variation under present investigation are available in bore log details. The 'N' value is progressively increasing towards depth due to the soil stratification.

Ground Water Table

Ground water table was not encountered during the investigation up to 4.5m depth from the ground level.

Shear Strength

Standard Penetration test blow counts 'N' is measured at different levels in the bore hole. Mainly these 'N' values are used to assess the shear strength of different soil layers.

Cohesive layers: The classification of 'N' values in moderate to high plasticity medium clay deposits developed by Terzaghi is used for estimating the unconfined compression strength and untrained shear strength of clay and sandy clay layers. The consistency between the N values and the liquidity index values is reviewed while assigning undrained shear strength for soft to medium clay layers.

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

Non - Cohesive Layers: The sand layers towards depth are of residual type and suitable modifications are made to take care of the presence of plastic fines in the residual soil. Conventional method of estimating the angle of shearing resistance ϕ corresponding to the relative density and the grain size distribution as per the classification suggested by Peck and suitably modified by the R.K. Hough to account for the influence of grain size distribution is adopted here.

7.0 Conclusions & Recommendation:

Shear strength parameters and compressibility parameters of different soil layers are established from the field and laboratory tests.

Based on the existing soil condition, it is recommended to provide foundation at 3m depth.

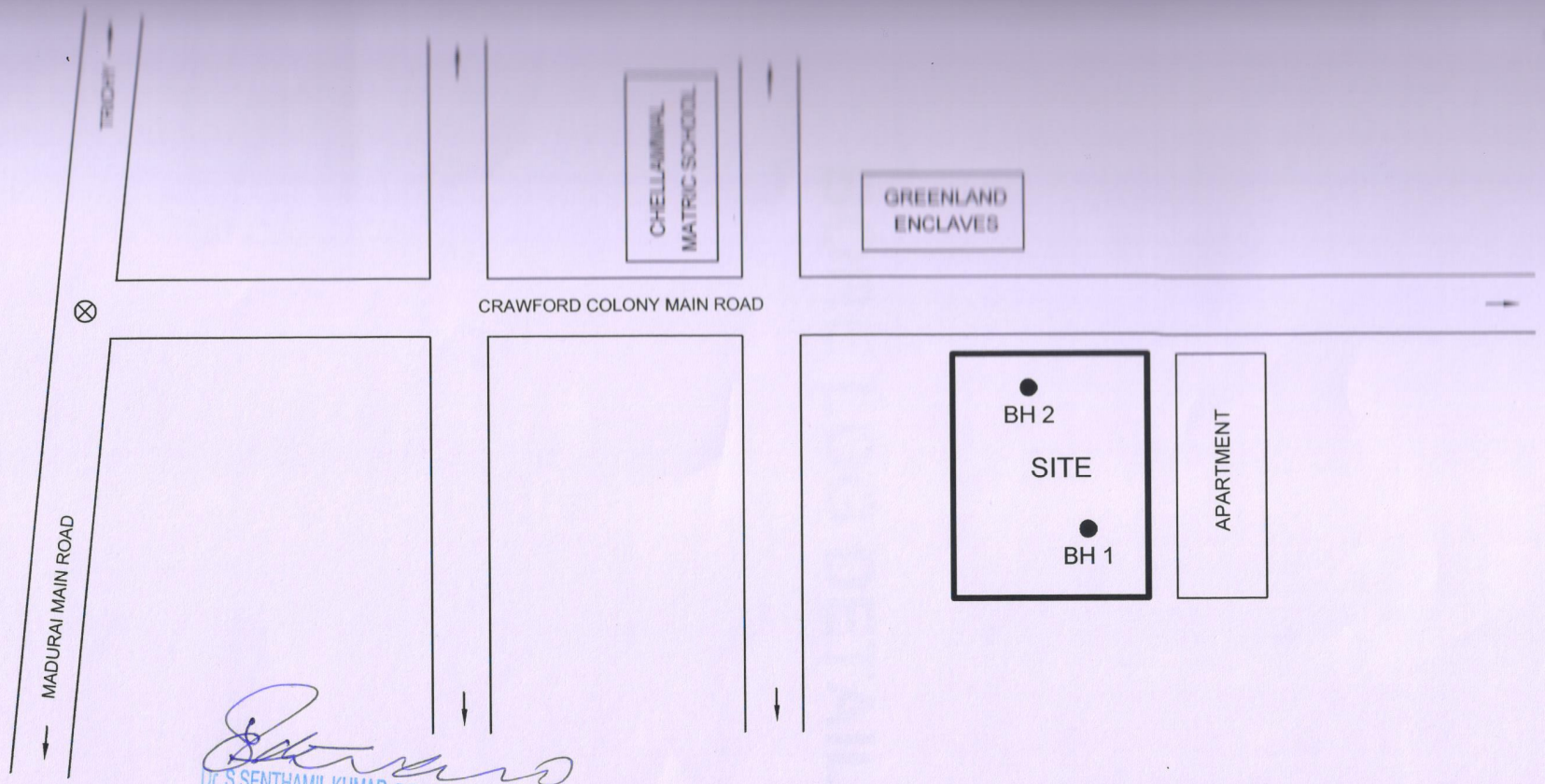
For Time Institute for Materials Testing

Dr. B.N. Krishnaswami.

05th August 2022

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

SITE LOCATION PLAN



Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

NOTE:

SITE LEVEL IS EQUAL TO THE ROAD LEVEL

LEGEND:

● BORE LOG LOCATION

⊗ SITE LOCATION

SITE PLAN

CLIENT	Mr. Fahd Habibullah, No. 112, Crawford Colony Main Road, Trichy - 620 012.		
SOIL CONSULTANT	TIME Institute for Materials Testing No 25, Sanjeevi Nagar, Trichy - Chennai Byepass road , Trichy - 620002. PHONE : 0431 - 2401326(OFF)		
DATE OF SURVEY	07.08.2022 & 08.08.2022	TITLE PLAN OF BORE HOLE AND SITE DETAILS AT CRAWFORD, TRICHY	
SCALE	NTS	DRG.NO DRAWING - 01	REV. 0

BORE LOG DETAILS

2.2 BORE LOG DETAILS FOR BORE HOLE 1

Project : Report on soil investigation for construction of Residential Apartment Building

Location : Crawford, Trichy

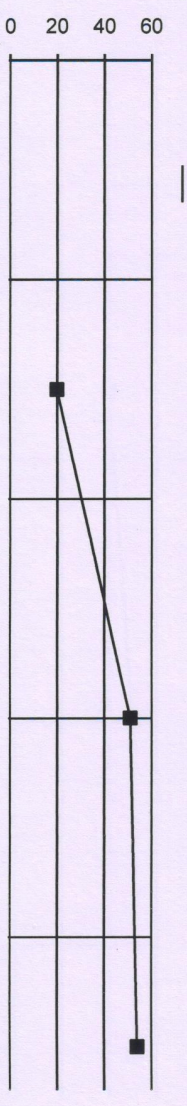
Started on : 07.08.2022 **Ended on** : 07.08.2022

Depth of G.W.T : Nil

Type of Boring : Calyx drilling

Site Level : Equal to the road level

Lithology of Bore Hole			SPT Data					Graphical representation of penetration Resistance ('N' Value)
Depth below G.L. (m)	Profile	Soil Description	Depth of SPT (m)	0-15cm	15-30cm	30-45cm	N value	
0 - 0.4m		Filled up soil						
0.4m -1.1m		Red soil with gravels mixed						
1.1m - 1.8m		Gravels	**1.5**	6	8	12	**20**	
1.8m - 3.5m		Disintegrated rock particles	**3.0**	Rebound			**51**	
3.5m - 4.5m		Soft rock	**4.5**	Rebound			**54**	



Bore terminated at 4.5m depth from the existing ground level

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

2.3 BORE LOG DETAILS FOR BORE HOLE 2

Project : Report on soil investigation for construction of Residential Apartment Building

Location : Crawford, Trichy

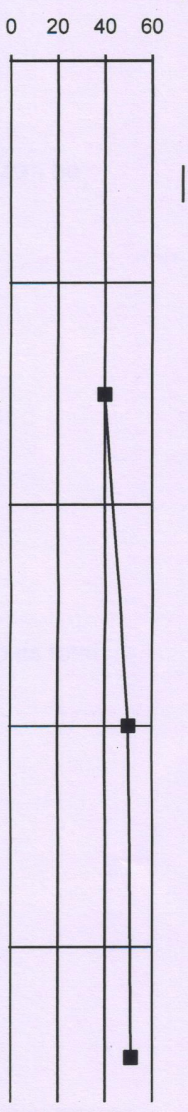
Started on : 08.08.2022 **Ended on** : 08.08.2022

Depth of G.W.T : Nil

Type of Boring : Calyx drilling

Site Level : Equal to the road level

Lithology of Bore Hole			SPT Data					Graphical representation of penetration Resistance ('N' Value)
Depth below G.L. (m)	Profile	Soil Description	Depth of SPT (m)	0-15cm	15-30cm	30-45cm	N value	
0 - 0.3m		Filled up soil						
0.3m - 0.9m		Red soil with gravels mixed						
0.9m - 2m		Gravels	1.5	8	16	24	40	
2m - 4.5m		Disintegrated rock particles	3.0	Rebound			50	
			4.5	Rebound			51	



Bore terminated at 4.5m depth from the existing ground level

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

2.4 Summary of Engineering Geo Properties

Natural Moisture Content:

The natural water content also called the natural moisture content is the ratio of the weight of water to the weight of the solids in a given mass of soil. This ratio is usually expressed as percentage.

$$W = [(W2-W3)/(W3-W1)] \times 100$$

Where,

W_1 = Weight of container gm

W_2 = Weight of container +wet soil gm

W_3 = Weight of container +dry soil

Bulk Density:

The total, or wet, density of soil, $_t$, is the ratio of the total mass of soil to its total volume and can be defined as follows:

$$\gamma_{bulk} = M / V$$

Where

M_t = Total Mass of soil

V_t = Total Volume

Dry Density:

The soil dry density, $_b$, is the ratio of the mass of the solid phase of the soil (i.e., dried soil) to its total volume (solid and pore volumes together) and is defined as follows:

$$\gamma_{dry} = M_b / V_b$$

Where

M_b = Total Mass of dried soil

V_b = Total Volume

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

Specific gravity:

Specific gravity is the ratio of the weight of a given volumeof a material at a standard temperature to the weight of an equal volume of distilled water at the same stated temperature.

The specific gravity G of the soil = $(W_2 - W_1) / [(W_4 - W_1)-(W_3-W_2)]$

Where

W_1 = Weight of density bottle

W_2 = Weight of density bottle + dry soil

W_3 = Weight of bottle + dry soil + water at temperature

W_4 = Weight of density bottle + Water

Void Ratio:

Soil void ratio (e) is the ratio of the volume of voids to the volume of solids:

$$e = (Vv) / (Vs)$$

Where

Vv is the volume of the voids (empty or filled with fluid),

Vs is the volume of solids.

Porosity:

Soil porosity (η) , which is defined as the ratio of the volume of voids to the total volume of the soil.

The porosity and the void ratio are inter-related as follows:

$$\eta = (1 - (\gamma_d/G_s))*100$$

$$e = \eta/(1-\eta) \;\&\; \eta = e / (1+e)$$

Where

γ_d = Dry Density

G_s = Specific gravity

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

Summary of Engineering Geo Properties for Bore Hole 1 & 2

Bore hole No.	Depth (m)	Soil Description	Natural Moisture Content (%)	Bulk Density (gm/cc)	Dry Density (gm/cc)	Specific gravity	Porosity (%)	Void Ratio
Bore Hole 1	1.5	Gravels	8.75	2.04	1.88	2.66	29	0.42
	3.0	Disintegrated rock particles	7.10	2.07	1.93	2.66	27	0.38
	4.5	Soft rock	0.42	2.15	2.14	2.68	20	0.25
Bore Hole 2	1.5	Gravels	8.89	2.05	1.88	2.66	29	0.41
	3.0	Disintegrated rock particles	7.16	2.06	1.92	2.66	28	0.38
	4.5		7.11	2.07	1.93	2.66	27	0.38

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

2.5 Summary of Laboratory Test Results for Bore Hole 1 & 2

Bore hole No.	Depth (m)	Soil Description	Gravel (%)	Coarse sand (%)	Medium Sand (%)	Fine sand (%)	Silt (%)	Clay (%)
Bore Hole 1	1.5	Gravels	45.3	28.4	19.8	6.5	0.0	0.0
Bore Hole 2	1.5	Gravels	42.9	28.4	18.4	10.3	0.0	0.0

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

2.6 Shear Strength Parameters for Different layers of Bore Hole 1 & 2

Bore Hole No.	Depth below G.L. (m)	Soil Description	N	Shear Strength 'Ø' (in degrees)
Bore Hole 1	1.5	Gravels	20	31.36
	3.0	Disintegrated rock particles	51	36.57
	4.5	Soft rock	54	37.20
Bore Hole 2	1.5	Gravels	40	36.14
	3.0	Disintegrated rock particles	50	36.78
	4.5		51	36.99

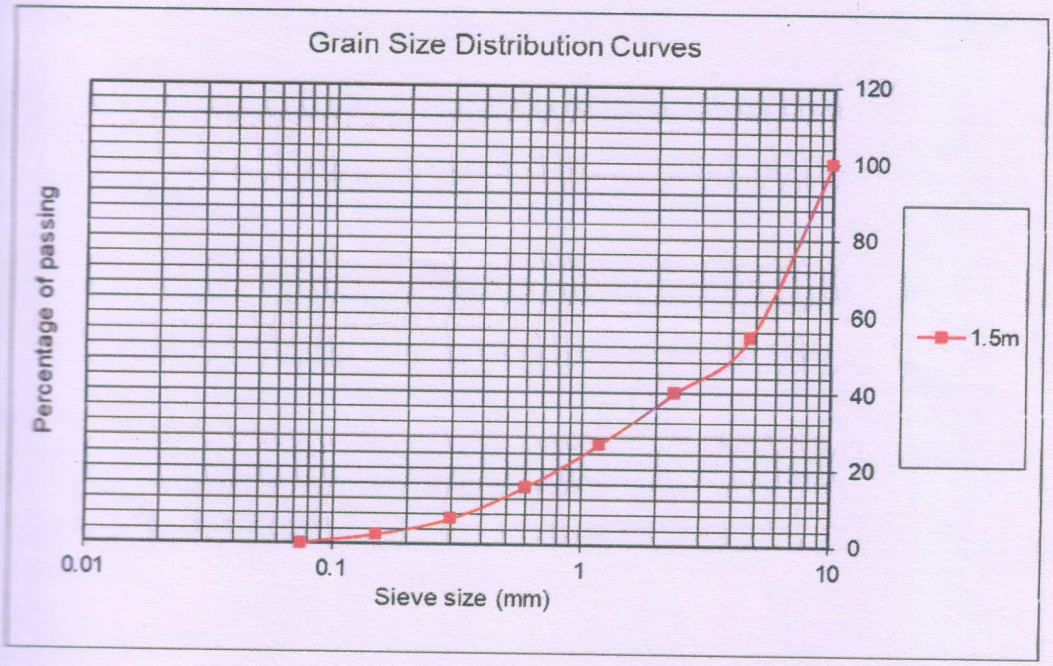
Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

GRAIN SIZE ANALYSIS

3.1 TEST REPORT ON SIEVE ANALYSIS OF SAMPLE

Bore Hole – 1 @ 1.5m

SIEVE SIZE	PERCENTAGE OF PASSING
	1.5m Depth
10 mm	100
4.75 mm	54.7
2.36 mm	40.1
1.18 mm	26.3
600 micron	15
300 micron	6.5
150 micron	2.3
75 micron	0



Description	1.5m Depth
Fineness modulus	4.551

Observation:

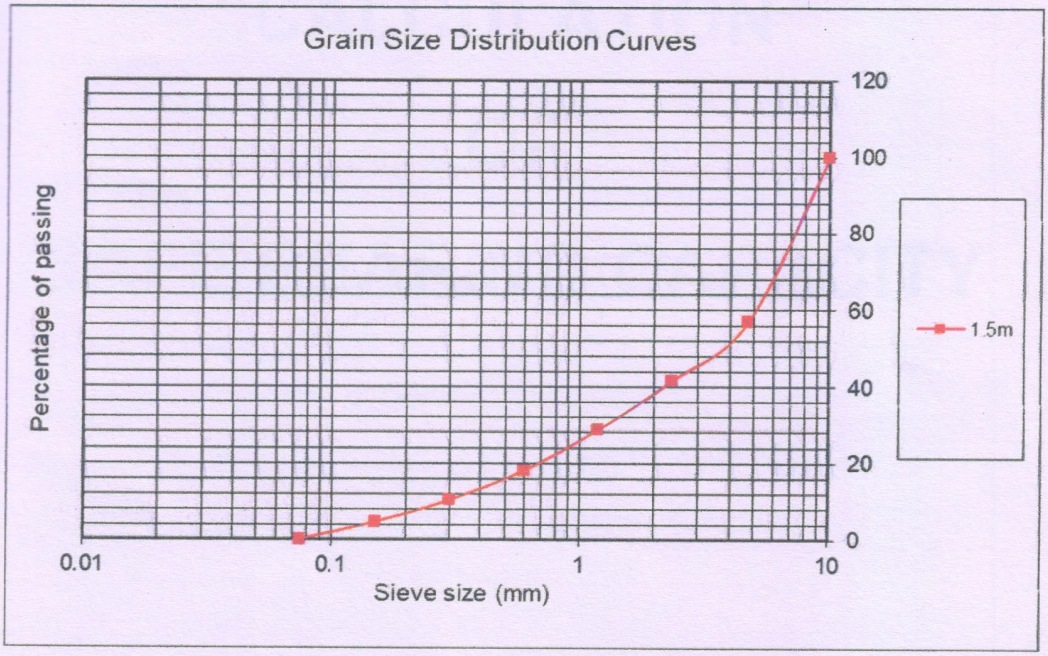
The inference from the graph shows that the gravels exists at 1.5m depth.

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

3.2 TEST REPORT ON SIEVE ANALYSIS OF SAMPLE

Bore Hole – 2 @ 1.5m

SIEVE SIZE	PERCENTAGE OF PASSING
	1.5m Depth
10 mm	100
4.75 mm	57.1
2.36 mm	41.8
1.18 mm	28.7
600 micron	18.1
300 micron	10.3
150 micron	4.5
75 micron	0



Description	1.5m Depth
Fineness modulus	4.395

Observation:

The inference from the graph shows that the gravels exists at 1.5m depth.

Dr. S.SENTHAMIL KUMAR, B.E, M.E, Ph.D, FIGS, MIEI,
Registered Geotechnical Engineer,
(TJR / RGE / 2019 / 086), Chartered Engineer,
703, Plot No.29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR-613 005.

CALCULATION

OF

SAFE BEARING CAPACITY

4. CALCULATION OF SAFE BEARING CAPACITY

4.1 Safe bearing capacity calculation based on settlement and Shear

Safe Bearing Capacity calculations based on settlement consideration (SPT value):

Safe Bearing Capacity calculations for shallow foundation will vary based on the width of foundation.

For example, in the case of Individual footing such as isolated, combined etc., we are supposed to use the formula as mentioned below. (Using Teng's Modified formula)

$$q_{na} = 51.4 \times (N-3) \times \left(\frac{B+0.3}{2B}\right)^2 \times R_w \times C_d$$

Where,

q_{na}	=	Net safe bearing capacity of soil (kN/m^2) (for 2.5cm permissible settlement)
N	=	Corrected SPT values
B	=	Width of foundation (m) (let us assume 2m)
R_w	=	Correction factor for water table level
C_d	=	Correction factor for depth of embedment

For Bore Hole 1 @ 3m depth SPT 'N' Value = 51

$\Rightarrow$ **q_{na} = 51.4 x (51 - 3) x ((2 + 0.3)/(2 x 2))^2 x 0.5 x 1 = 407.85 kN/m^2**

$\therefore$ **q_{na} = 40.78 t/m^2**

Dr.S.SENTHAMIL KUMAR, BE.,ME.,Ph.D.,FGS,MIG
Registered Geotechnical Engineer,
(TJR/RGE/2019/096), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

Safe Bearing Capacity calculations based on shear consideration (Direct Shear):

As Per IS 6403:1981

At 3m depth (Bore Hole 1)

Proving Ring capacity 200kg = 966.2 division

Area of the shear box = 6cm x 6cm = $36cm^2$

Normal Stress (kg/cm^2)	No. of Division	Stress
0.5	24	0.138
1.0	87	0.506
1.5	151	0.880

Stress = Load / Area

= (200/966.2) * 24* (1/36) = 0.138

Ø = $\tan^{-1}$ (0.880-0.138) = 36.57°

Net safe bearing capacity of the soil (for general shear failure as per IS 6403:1981)

$$q_{ult} = C N_c S_c d_c i_c + q (N_q - 1) S_q d_q i_q + 0.5 B \gamma N_\gamma S_\gamma d_\gamma i_\gamma W'$$

Where

C	=	Cohesion in t/m^2
N_c, Nq, N_γ	=	Bearing Capacity Factors (From Table 1 of IS 6403 – 1981)
S_c, S_q, S_γ	=	Shape Factors (From Table 2 of IS 6403 – 1981)
d_c, d_q, d_γ	=	Depth Factors
i_c, i_q, i_γ	=	Inclination Factors
q	=	Effective surcharge at the base level of foundation in t/m^2
B	=	Width of footing, diameter of circular footing in m
γ	=	Bulk unit weight of soil in t/m^3
W'	=	Correction factor for location of water table

At 3m depth (Bore hole 1), the Soil profile is Disntegrated rock particles

If	C	=	0 (Since no Cohesion)
	Φ	=	36.57 (based on laboratory test)
Therefore	N_c	=	55.261;
	N_q	=	42.976;
	N_γ	=	67.251

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,MISTE
Registered Geotechnical Engineer,
(TJR/RGE/2019/088), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

Applying these parameters in the above equation

$$q_{ult} = C N_c S_c d_c i_c + q (N_q - 1) S_q d_q i_q + 0.5 B \gamma N_\gamma S_\gamma d_\gamma i_\gamma W'$$

Where $C = 0$ **(Since no Cohesion)**

Therefore, $q_{ult} = q (N_q - 1) S_q d_q i_q + 0.5 B \gamma N_\gamma S_\gamma d_\gamma i_\gamma W'$

Bearing Capacity calculation through shear density of soil 'γ'

There are two relationships of weight of soil to Volume of soil as

i) Total density or Bulk density

ii) Dry density

Therefore, Total Bulk density, $\gamma_t = \frac{\text{Total Weight of soil}}{\text{Total Volume of soil}} = \frac{W}{V}$

Dry Density $\gamma_d = W_s/V$; Whereas W_s = Total weight of solids

In a particular case the **total density** will be known as **saturated density**, when the soil is saturated.

Another density of soil will be known as submerged or buoyant density, γ_b when the soil is submerged in water.

$$\gamma_b = \gamma_t - \gamma_w$$

Where γ_w = Density of water = 1 g/cc or 1 t/m^3

Mostly soils submerged in water are saturated.

Hence $\gamma_{buoyant} = \gamma_{sat} - \gamma_w$

Based on the above equations,

$\gamma = \gamma_{sat} = 1.8$ t/m^3

$d = 3$m

Here, $\gamma_b = \gamma_{sat} - 1 = 1.8 - 1 = 0.8$ t/m^3

Therefore, $q = \gamma_b \times d = 0.8 \times 3 = 2.4$ t/m^2

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MIE
Registered Geotechnical Engineer,
(TJR/RGE/2019/088), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

$S_q = S_\gamma$ = Shape factor which is equal to 1

$d_q = d_\gamma$ = $1 + 0.1 \frac{df}{B} \sqrt{N\emptyset}$

= 1 (Minimum value of 1 can be taken)

i_q = inclination factor $(1 - a/90)^2$

Where α = inclination of the load to the vertical in degree which is equal to zero

i_q = 1

B = Width of footing (2m)

W' = 0.5

Applying in the above equation

q_{ult} = $q\ (Nq - 1)\ S_q\ d_q\ i_q + 0.5\ B\ \gamma\ N_\gamma\ S_\gamma\ d_\gamma\ i_\gamma\ W'$

= $2.4 \times (42.976 - 1) \times 1 \times 1 \times 1 + 0.5 \times 2 \times 1.8 \times 67.251 \times 1 \times 1 \times 1 \times 0.5$

= 161.26 t/m^2

Applying factor of safety as 3

Then allowable bearing capacity

q_{na} = 161.26 / 3

= 53.76 t/m^2

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,(Ph.D.),MIE
Registered Govt. Approved Chartered Engineer,
[TJR/RGE/2013/0004, Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

SAFE BEARING CAPACITY RESULTS

4. Calculation of Safe bearing capacity of the soil:

4.2 Safe Bearing capacity of soil based on SPT values for Bore Holes 1 & 2

Bore Hole No.	Depth (m)	SPT value	Safe Bearing capacity of soil	
			KN/m^2	t/m^2
BH – 1	1.50	20	144.45	14.44
	3.00	51	407.85	40.78
	4.50	54	433.35	43.33
BH – 2	1.50	40	314.39	31.43
	3.00	50	399.36	39.93
	4.50	51	407.85	40.78

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MIEI
Registered Geotechnical Engineer,
(TJR/RGE/2019/086), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

4.3 Safe Bearing capacity of soil based on Shear Failure Criteria:

Through Direct Shear test

Bore Hole No.	Depth (m)	Normal Stress (kg/cm^2)	Division	Ø (in degrees)	Safe Bearing Capacity of Soil (t/m^2)
BH - 1	1.50	0.5 1.0 1.5	23 76 129	31.36°	17.40
	3.00	0.5 1.0 1.5	24 88 153	36.57°	53.76
	4.50	0.5 1.0 1.5	24 88 156	37.20°	77.56
BH - 2	1.50	0.5 1.0 1.5	24 87 151	36.14°	34.34
	3.00	0.5 1.0 1.5	24 89 154	36.78°	55.58
	4.50	0.5 1.0 1.5	24 90 155	36.99°	75.24

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MIE
Registered Geotechnical Engineer,
(TJR/RGE/2019/086), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 009.
Mobile : 9443976090

4.4 Comparison of Safe bearing capacity of soil through Shear and Settlement consideration:

Bore Hole no.	Depth (m)	Safe Bearing Capacity (t/m^2)	
		Shear consideration (Direct Shear)	Settlement consideration
BH – 1	1.50	17.40	14.44
	3.00	53.76	40.78
	4.50	77.56	43.33
BH – 2	1.50	34.34	31.43
	3.00	55.58	39.93
	4.50	75.24	40.78

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MIEI
Registered Geotechnical Engineer,
(TJR/RGE/2019/086), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

5. Result:

5.1 Recommended Safe bearing capacity values at different depths

Considering the lesser value of the above, the Safe Bearing Capacity, values at different depths are recommended below:

Bore Hole	Depth (m)	Safe Bearing Capacity (t/m^2)
BH – 1	1.50	14.44
	3.00	40.78
	4.50	43.33
BH – 2	1.50	31.43
	3.00	39.93
	4.50	40.78

The above test data and findings are relevant and applicable only for the soil conditions existing at the test site. Any change in the soil strata observed during excavation may be reported to the laboratory, which will redefine the safe bearing capacity of the soil.

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MIEI
Registered Geotechnical Engineer,
(TJR/RGE/2019/086), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

SOIL & ROCK SAMPLE PHOTOS

0



4.5m

Bore Hole No: 1

Depth : 0 to 4.5m

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MISTE
Registered Geotechnical Engineer,
(TJR/RGE/2019/086), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

0



4.5m

Bore Hole No: 2

Depth : 0 to 4.5m

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MIE
Registered Geotechnical Engineer,
[TJR/RGE/2019/086], Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

SITE PHOTO



SITE: CRAWFORD, TRICHY

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FICS,MIE
Registered Geotechnical Engineer,
[TJR/RGE/2019/086], Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005
Mobile : 9443976090

ANNEXURE

Time Institute for Materials Testing
Soil Division,
#25, Sanjeevi Nagar,
Trichy- Chennai Byepass road, Trichy - 620 002.
Phone: 0431 2401326
Fax: 0431 2401596
Email: tmaterialstesting@yahoo.co.in

Annexure - A

NATURAL MOISTURE CONTENT

(As per IS 2720 (Part - 2) - 1973)

Project Name : **Soil investigation for construction of Residential Apartment Building at Crawford, Trichy**

Bore Hole No. : **1**

Depth : **3m**

Trial	Wt of container (W) gm	Wt of container +wet soil (W_1) gm	Wt of container + Dry soil (W_2) gm	Wt of Water ($W_3 = W_1-W_2$) gm	Wt of Dry Soil ($W_4 = W_2-W$) gm	Water Content (%) = (W_3-W_4)*100
1	24.00	49.70	48.00	1.7	24.0	7.08
2	24.50	52.10	50.25	1.9	25.8	7.18
3	22.80	53.30	51.30	2.0	28.5	7.02

Natural Moisture Content = 7.10

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MIE
Registered Geotechnical Engineer,
(TJR/RGE/2019/086), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

Time Institute for Materials Testing
Soil Division,
#25, Sanjeevi Nagar,
Trichy- Chennai Byepass road, Trichy - 620 002.
Phone: 0431 2401326
Fax: 0431 2401596
Email: tmaterialstesting@yahoo.co.in

Annexure - B

SPECIFIC GRAVITY TEST
(As per IS 2720 (Part - 3) - 1980)

Project Name : **Soil investigation for construction of Residential Apartment Building at Crawford, Trichy**

Bore Hole No. : **1**

Depth : **3m**

Trial	Bottle Wt (W1) gm	Bottle + Sample (W2) gm	Bottle + Sample + Water (W3) gm	Bottle + Water (W4) gm	Specific Gravity
1	32.6	93.3	118.7	80.8	2.662
2	32.6	92.5	118.2	80.8	2.662
3	32.6	93.6	118.9	80.8	2.664

Specific gravity of Fine Aggregate = 2.66

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIOS,MIE
Registered Geotechnical Engineer,
(TJR/RGE/2019/086), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005.
Mobile : 9443976090

Time Institute for Materials Testing
Soil Division,
#25, Sanjeevi Nagar,
Trichy- Chennai Byepass road, Trichy - 620 002.
Phone: 0431 2401326
Fax: 0431 2401596
Email: tmaterialstesting@yahoo.co.in

Annexure - C

DENSITY CALCULATION

Project Name : **Soil investigation for construction of Residential Apartment Building at Crawford, Trichy**

Bore Hole No. : **1**

Depth : **3m**

Sl. No.	Description	Reading
1	Weight of Empty Box (gm)	630.00
2	Weight of Box + Soil (gm)	816.00
3	Weight of Soil (gm)	186.00
4	Volume of Box (cm^3)	90.00
5	**Bulk Density (gm/cm^3)**	**2.07**
6	Weight of Container (gm)	24.00
7	Weight of Container + Wet Soil (gm)	49.70
8	Weight of Container + Dry Soil (gm)	48.00
9	Moisture Content (%)	7.08
10	**Dry Density (gm/cm^3)**	**1.93**

Result

Bulk Density of Soil = 2.07

Dry Density of Soil = 1.93

Dr.S.SENTHAMIL KUMAR, B.E.,M.E.,Ph.D.,FIGS,MIE
Registered Geotechnical Engineer,
(TJR/RGE/2019/086), Chartered Engineer,
703, Plot No. 29, OM Rajoo Nagar,
Near New Bus Stand, THANJAVUR - 613 005
Mobile : 9443976090

Time Institute for Materials Testing
Soil Division,
#25, Sanjeevi Nagar,
Trichy- Chennai Byepass road, Trichy - 620 002.
Phone: 0431 2401326
Fax: 0431 2401596
Email: tmaterialstesting@yahoo.co.in

Annexure - F

Direct Shear Test (IS 2720 - Part 13 -1986)

Bore Hole	1
Depth (m)	3m
Date of Test	09.08.2022

L.C. Of Dial Gauge	0.002
Proving ring Constant	0.206996
Area of the specimen (cm^2)	36

Trial 1

Normal Stress: 0.5 kg/cm^2

Horizontal Gauge Reading	Proving Ring Reading	Shear Deformation	Shear Stress (kg/cm^2)
0	0	0.00	0.000
50	10	0.50	0.057
100	12	1.00	0.069
150	14	1.50	0.080
200	16	2.00	0.092
250	18	2.50	0.103
300	20	3.00	0.115
350	22	3.50	0.126
400	24	4.00	0.138

Trial 2

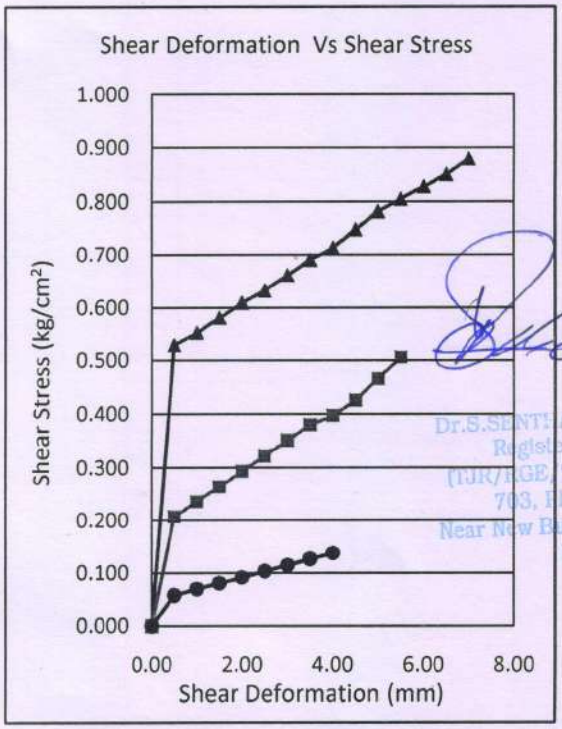
Normal Stress: 1 kg/cm^2

Horizontal Gauge Reading	Proving Ring Reading	Shear Deformation	Shear Stress (kg/cm^2)
0	0	0.00	0.000
50	36	0.50	0.207
100	41	1.00	0.236
150	46	1.50	0.264
200	51	2.00	0.293
250	56	2.50	0.322
300	61	3.00	0.351
350	66	3.50	0.379
400	69	4.00	0.397
450	74	4.50	0.425
500	81	5.00	0.466
550	88	5.50	0.506

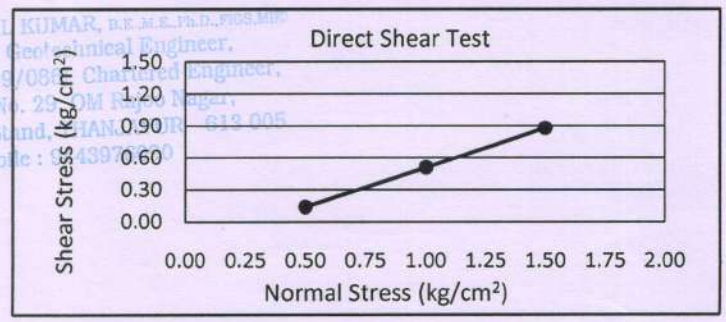
Trial 3

Normal Stress: 1.5 kg/cm^2

Horizontal Gauge Reading	Proving Ring Reading	Shear Deformation	Shear Stress (kg/cm^2)
0	0	0.00	0.000
50	92	0.50	0.529
100	96	1.00	0.552
150	101	1.50	0.581
200	106	2.00	0.609
250	110	2.50	0.632
300	115	3.00	0.661
350	120	3.50	0.690
400	124	4.00	0.713
450	130	4.50	0.747
500	136	5.00	0.782
550	140	5.50	0.805
600	144	6.00	0.828
650	148	6.50	0.851
700	153	7.00	0.880



Normal Stress (Kg/cm^2)	Proving Ring Reading at failure	Shear Stress (kg/cm^2)
0.50	24	0.138
1.00	88	0.506
1.50	153	0.880



Ø = $\tan^{-1}$ (0.880 - 0.138)

Angle of Shearing Resistance (Ø) = 36.57 °



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3/350/2 Mettukuppam,
OMR Thoraipakkam,
Chennai, India

hello@verified.realestate
+91 9996 12 9996
www.verified.realestate