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REPORT

ON

SOIL INVESTIGATION FOR CONSTRUCTION

OF

RESIDENTIAL APARTMENT BUILDING

AT

VISWAS NAGAR, TRICHY

Client

Mr. Sivasubramaniyan & Shyamala,

Mr. Subramaniyan & Nithya,

Mrs. Menmozhi,

Mrs. V. Senthamilselvi,

Mr. Vishnu Palanisamy & Saranya,

Mr. Balakrishnan,

Mr. Sridhar (Bangaru),

(Power Agent) Mr. D. Ramakrishnan,

Plot No:26,27&28, T.S.No : 18/76 (OLD S.F.NO:46/6C), Ward-Q, Block No:2,

Viswas Nagar, Tharanallur Village, Ariyamangalam Zone,

Trichy City Corporation.

By

Time

TIME INSTITUTE FOR MATERIALS TESTING

14/16 NEW RAJA COLONY,

BEEMA NAGAR,

CONTONMENT,

TIRUCHIRAPPALLI – 620 001.

Ph: 0431 2401326

NOVEMBER 2020

Dr.S. Senthamil Kumar, ME,Ph.D,FIGS, MIEI
Registered Geotechnical Engineer - No. 001/RGTE/2024
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Near New Bus Stand, Thanjavur - 613 005.
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SUMMARY

Project	:	Construction of Residential Apartment Building at Viswas Nagar, Trichy
Project Reference	:	Time / 32/ Soil Test / November 2020
Owner	:	Mr. Sivasubramaniyan & Shyamala, Mr. Subramaniyan & Nithya, Mrs. Menmozhi, Mrs. V. Senthamilselvi, Mr. Vishnu Palanisamy & Saranya, Mr. Balakrishnan, Mr. Sridhar (Bangaru), (Power Agent) Mr. D. Ramakrishnan, Plot No:26, 27 & 28, T. S. No : 18/76 (OLD S.F.NO:46/6C), Ward-Q, Block No:2, Viswas Nagar, Tharanallur Village, Ariyamangalam Zone, Trichy City Corporation.
Dwelling Units	:	50
Number of Floors	:	Stilt+5
Type of Structure	:	R.C.C. Framed construction
Location	:	Viswas Nagar, Trichy
Scope	:	Geotechnical investigation for the entire area of construction
Test conducted	:	Two explanatory bore holes up to 4.5m depth were made and conducted standard penetration tests at every 1.5m depth. The undisturbed soil samples were collected at every 1.5m depth for laboratory test.

FINDINGS

Soil at shallow depth	:	Clay exists up to 1.1m depth
Hard stratum	:	Disintegrated rock particles exists between 4m to 4.5m depth
Ground water table	:	Ground water table exist at 1.2m depth below ground level during investigation.

FOUNDATION : Based on the existing soil condition, it is recommended to provide foundation at 3m depth.

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SOIL INVESTIGATION

American Global Standards
AIAO-BAR
ACCREDITED
ISO - 17025:2017
B.N. Krishnaswami, M.E., (Struct Engg.,) Ph.D.,
Former Faculty, NIT, Trichy

Time Institute for Materials Testing
टाइम इन्स्टिट्यूट चीज़े शोध केलिए
Soil & Structural Consultant, Construction Materials Testing Lab
Quality is Excellence

November 12, 2020

To

Mr. Sivasubramaniyan & Shyamala,
Mr. Subramaniyan & Nithya,
Mrs. Menmozhi,
Mrs. V. Senthamilselvi,
Mr. Vishnu Palanisamy & Saranya,
Mr. Balakrishnan,
Mr. Sridhar (Bangaru),
(Power Agent) Mr. D. Ramakrishnan,
Plot No:26,27&28, T.S.No : 18/76 (OLD S.F.NO:46/6C), Ward-Q, Block No:2,
Viswas Nagar, Tharanallur Village, Ariyamangalam Zone,
Trichy City Corporation.

REPORT ON SOIL INVESTIGATION

(Ref: TIME/32/ SOIL TEST/ NOVEMBER 2020)

Sub: Soil Investigation for construction of Residential Apartment Building at Viswas Nagar, Trichy - reg

Ref: Based on letter dated; 07.11.2020

1.1 Introduction:

It is proposed to construct a **Residential Apartment Building at Viswas Nagar, Trichy.** A detailed soil investigation was suggested for estimating the safe bearing capacity of the soil, so that suitable type of foundation can be decided for the proposed structure. Time Institute for Materials Testing (TIME) has been requested to undertake the soil investigation at the proposed site. The study carried out by TIME on **09.11.2020.** This report presents complete investigation data and discusses the results to recommend the bearing capacity values at different depths for the proposed structure.

Section 2 describes the soil profile and bore log details of two exploratory bore holes. Section 3 of this report gives the Grain size distribution curves and its analysis. Section 4 of this report presents the calculated safe Bearing capacity values of soil at various depths. Section 5 of this report reviews the test results and the recommended safe bearing capacity values for design of foundation.

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1.2 Investigation Programme, Procedure and Data:

Investigation was carried out in two locations at the site through exploratory bore holes. Soil samples were collected from each bore hole at different depths for conducting laboratory test. Standard Penetration test was conducted at regular intervals and 'N' values were recorded. Undisturbed soil samples were collected and were preserved and transported to the laboratory for detailed identification tests. Based on the field and laboratory tests on the samples collected, the results are furnished in this report.

An exploratory bore hole was advanced from the existing ground level using truck mounted rotary drilling techniques supplemented by Betonies mud circulation. This drilling procedure with mud circulation is found most suitable for making exploratory bore hole. The mud circulation was employed through the drill rods and letting it out though the side jets provided in the cutting tool thus preventing any disturbance at the bore hole bottom. Mud circulation was used to stabilize the sides and the bottom of the bore hole, and then to bring the soil cuts to the surface. It is important to note that the mud jet is not used to cut the soils as in the case of wash boring technique. Use of drilling mud will also help in preventing the disturbance to the soil at the bore hole bottom during drilling operations. Diameter of the bore hole is about 150mm.

Bore hole was always kept full with the drilling mud so that a positive head is maintained in the bore hole thus preventing any disturbance to the soil within the test zone.

The field tests included Standard penetration test, Disturbed and undisturbed soil sampling, identification of different soil layers, Ground water table observation, complete logging of the bore hole, etc.

Laboratory investigation consisted of classification tests like grain size distribution analysis, determination of specific gravity etc. Unconfined Compression test and Direct shear was conducted on preserved soil samples. All the field and laboratory tests were conducted according to the procedures stipulated in relevant IS codes.



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1.3 Review of Field and Laboratory Results:

The original ground level in the presently investigated area was plain terrain.

Sub soil Profile

Bore Hole 1:

The natural soil below the existing ground level is clay. This layer is extended up to 1.1m depth. Clay with gravels mixed exists from 1.1m to 2.2m depth. Gravels exist from 2.2m to 4m depth. Below which disintegrated rock particles exists from 4m to 4.5m depth from the existing ground level. The soil stratification and its variation under present investigation are available in bore log details. The 'N' value is progressively increasing towards depth, due to the soil stratification.

Bore Hole 2:

The natural soil below the existing ground level is clay. This layer is extended up to 0.9m depth. Clay with gravels mixed exists from 0.9m to 2.1m depth. Gravels exist from 2.1m to 4m depth. Below which disintegrated rock particles exists from 4m to 4.5m depth from the existing ground level. The soil stratification and its variation under present investigation are available in bore log details. The 'N' value is progressively increasing towards depth, due to the soil stratification.

Ground Water Table:

Ground water table exist at 1.2m depth below ground level during investigation.

Shear Strength

Standard Penetration test blow counts 'N' is measured at different levels in the bore hole. Mainly these 'N' values are used to assess the shear strength of different soil layers.

Cohesive layers: The classification of 'N' values in moderate to high plasticity medium clay deposits developed by Terzaghi is used for estimating the unconfined compression strength and untrained shear strength of clay and sandy clay layers. The consistency between the N values and the liquidity index values is reviewed while assigning undrained shear strength for soft to medium clay layers.

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Non – Cohesive Layers: The sand layers towards depth are of residual type and suitable modifications are made to take care of the presence of plastic fines in the residual soil. Conventional method of estimating the angle of shearing resistance ϕ corresponding to the relative density and the grain size distribution as per the classification suggested by Peck and suitably modified by the R.K. Hough to account for the influence of grain size distribution is adopted here.

1.4 Conclusions & Recommendations:

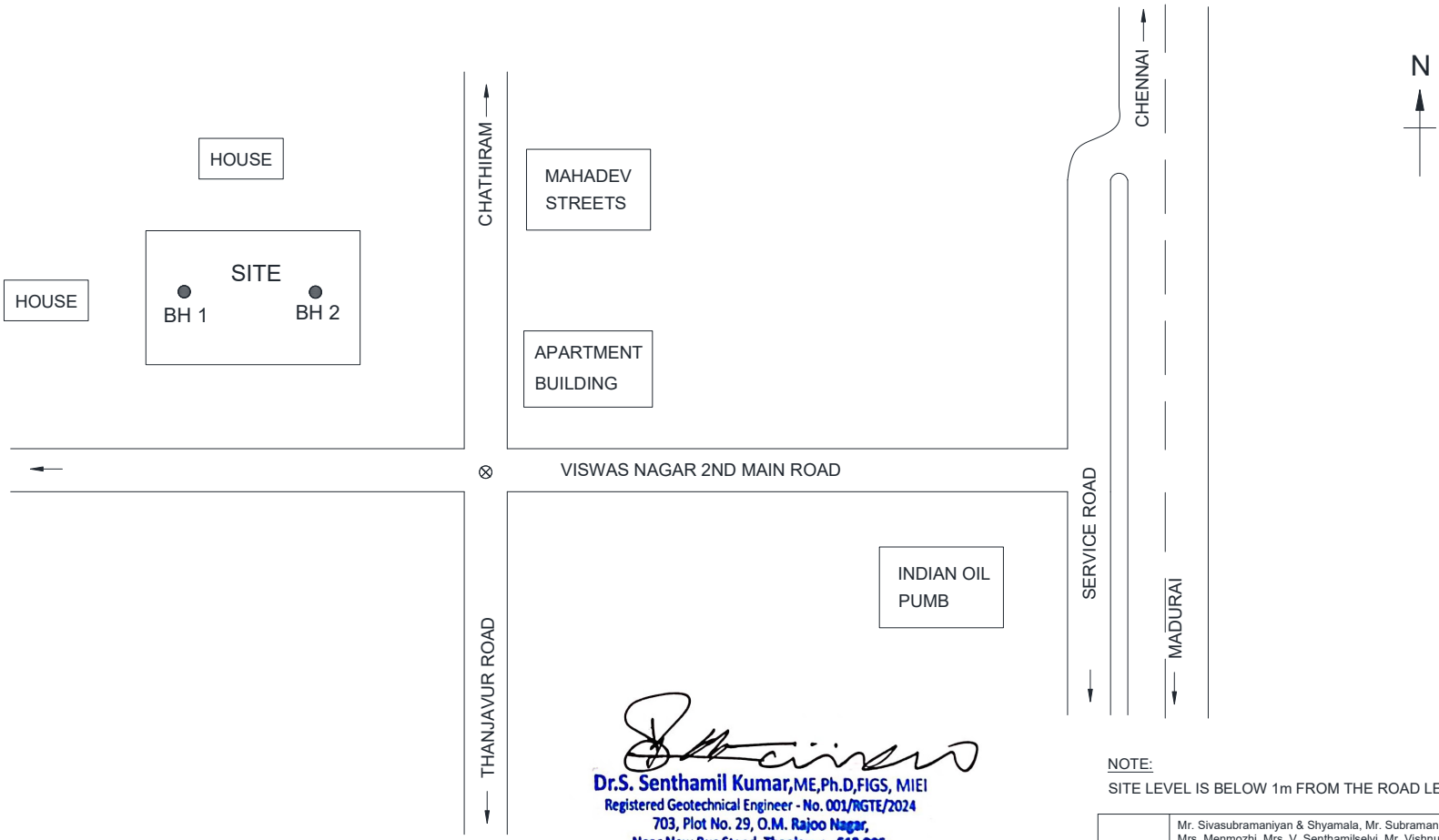
Shear strength parameters and compressibility parameters of different soil layers are established from the field and laboratory tests.

Based on the existing soil condition, it is recommended to provide foundation at 3m depth.

12th November 2020

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SITE LOCATION PLAN



LEGEND:

- BORE LOG LOCATION
- SITE LOCATION

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NOTE:

SITE LEVEL IS BELOW 1m FROM THE ROAD LEVEL

SITE PLAN

CLIENT	Mr. Sivasubramaniyan & Shyamala, Mr. Subramaniyan & Nithya, Mrs. Menmozhi, Mrs. V. Senthamilselvi, Mr. Vishnu Palanisamy & Saranya, Mr. Balakrishnan, Mr. Sridhar (Bangaru), (Power Agent) Mr. D. Ramakrishnan, Viswas Nagar, Old Palpannai, Trichy.		
SOIL CONSULTANT	TIME Institute for Materials Testing 4th Cross, 14/16. New Raja Colony, Beemanagar, Trichy - 620001. PHONE : 0431 - 2401326(OFF)		
DATE OF SURVEY	09.11.2020	TITLE PLAN OF BORE HOLE AND SITE DETAILS AT VISWAS NAGAR, TRICHY	
SCALE	NTS	DRG.NO DRAWING - 01	REV. 0

BORE LOG DETAILS

2.2 BORE LOG DETAILS FOR BORE HOLE 1

Project : Report on soil investigation for construction of Residential Apartment Building

Location : Viswas Nagar, Trichy

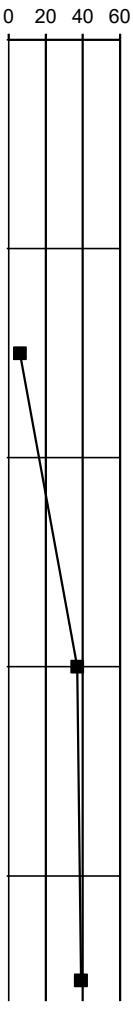
Started on : 09.11.2020 **Ended on** : 09.11.2020

Depth of G.W.T : 1.2m

Type of Boring : Calyx

Site Level : Below 1m from the road level

Lithology of Bore Hole			SPT Data						Graphical representation of penetration Resistance('N' Corrected)
Depth below G.L. (m)	Profile	Soil Description	Depth of SPT (m)	0-15cm	15-30cm	30-45cm	N' Observed	N' Corrected	
0 - 1.1m		Clay							
1.1m - 2.2m		Clay with gravels mixed	**1.5**	4	4	5	**9**	**6**	
2.2m - 4m		Gravels	**3.0**	Rebound			**50**	**37**	
4m - 4.5m		Disintegrated rock particles	**4.5**	Rebound			**52**	**39**	



Bore terminated at 4.5m depth from the existing ground level

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2.3 BORE LOG DETAILS FOR BORE HOLE 2

Project : Report on soil investigation for construction of Residential Apartment Building

Location : Viswas Nagar, Trichy

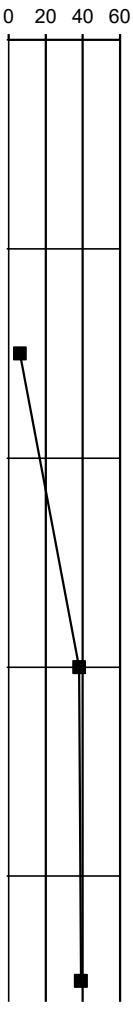
Started on : 09.11.2020 **Ended on** : 09.11.2020

Depth of G.W.T : 1.2m

Type of Boring : Calyx

Site Level : Below 1m from the road level

Lithology of Bore Hole			SPT Data						Graphical representation of penetration Resistance('N' Corrected)
Depth below G.L. (m)	Profile	Soil Description	Depth of SPT (m)	0-15cm	15-30cm	30-45cm	N' Observed	N' Corrected	
0 - 0.9m		Clay							
0.9m - 2.1m		Clay with gravels mixed	**1.5**	4	4	4	**8**	**6**	
2.1m - 4m		Gravels	**3.0**	Rebound			**51**	**38**	
4m - 4.5m		Disintegrated rock particles	**4.5**	Rebound			**53**	**39**	



Bore terminated at 4.5m depth from the existing ground level

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2.4 Summary of Engineering Geo Properties for Bore Hole 1 & 2

Bore hole No.	Depth (m)	Soil Description	Natural Moisture Content (%)	Bulk Density (gm/cc)	Dry Density (gm/cc)	Specific gravity
Bore Hole 1	1.5	Clay with gravels mixed	17.46	1.88	1.60	2.62
	3.0	Gravels	9.18	2.04	1.87	2.66
	4.5	Disintegrated rock particles	7.20	2.06	1.92	2.67
Bore Hole 2	1.5	Clay with gravels mixed	17.31	1.90	1.62	2.62
	3.0	Gravels	9.05	2.05	1.88	2.66
	4.5	Disintegrated rock particles	7.12	2.07	1.93	2.67

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2.5 Summary of Laboratory Test Results for Bore Hole 1 & 2

Bore hole No.	Depth (m)	Soil Description	Atterberg limits			Gravel (%)	Coarse sand (%)	Medium Sand (%)	Fine sand (%)	Silt (%)	Clay (%)
			Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)						
Bore Hole 1	1.5	Clay with gravels mixed	34.5	22.6	11.9	10.5	12.7	14.6	22.1	3.7	36.4
	3.0	Gravels	-	-	-	42.1	30.7	16.7	10.5	0.0	0.0
Bore Hole 2	1.5	Clay with gravels mixed	34.8	22.2	12.6	11.9	12.7	15.7	20.2	3.9	35.6
	3.0	Gravels	-	-	-	43.8	30.5	19.9	5.8	0.0	0.0

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2.6 Shear Strength Parameters for Different layers of Bore Hole 1 & 2

Bore Hole No.	Depth below G.L. (m)	Soil Description	N	Cohesion 'c' (t/m^2)	Shear Strength 'Ø' (in degrees)
Bore Hole 1	1.5	Clay with gravels mixed	6	2.80	27.88
	3.0	Gravels	37	-	32.78
	4.5	Disintegrated rock particles	39	-	33.24
Bore Hole 2	1.5	Clay with gravels mixed	6	2.10	27.62
	3.0	Gravels	38	-	33.01
	4.5	Disintegrated rock particles	39	-	33.24

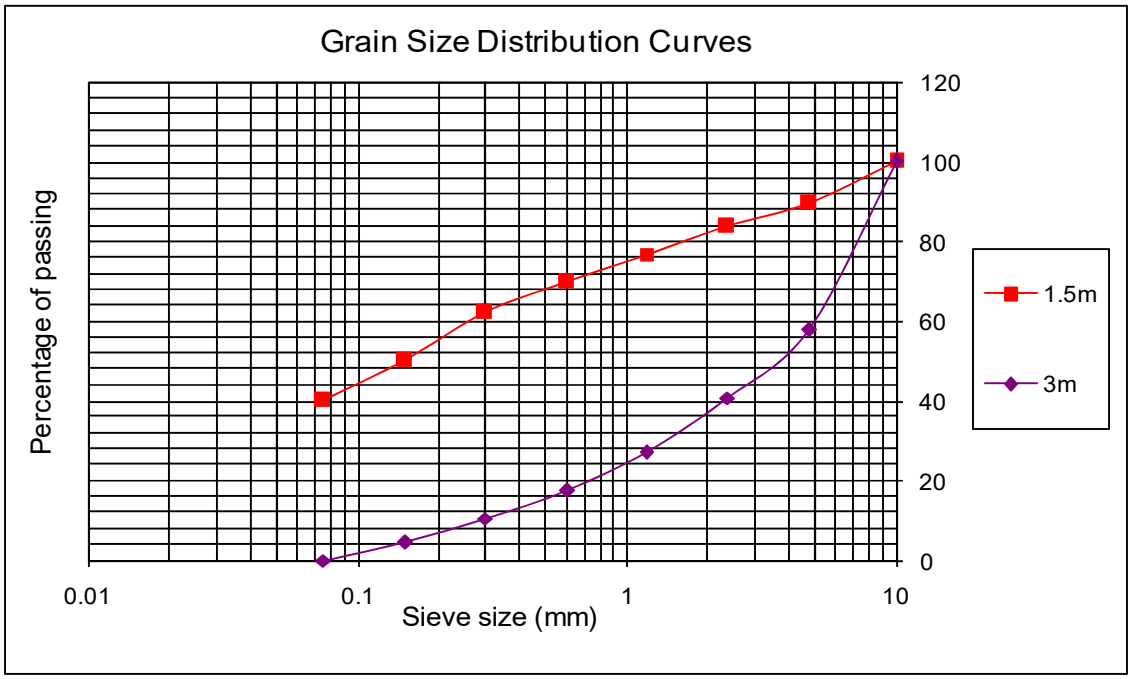
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GRAIN SIZE ANALYSIS

3.1 TEST REPORT ON SIEVE ANALYSIS OF SAMPLE

Bore Hole – 1 @ 1.5m & 3m

SIEVE SIZE	PERCENTAGE OF PASSING	
	1.5m Depth	3m Depth
10 mm	100	100
4.75 mm	89.5	57.9
2.36 mm	83.7	40.7
1.18 mm	76.8	27.2
600 micron	69.7	17.4
300 micron	62.2	10.5
150 micron	50.1	4.6
75 micron	40.1	0



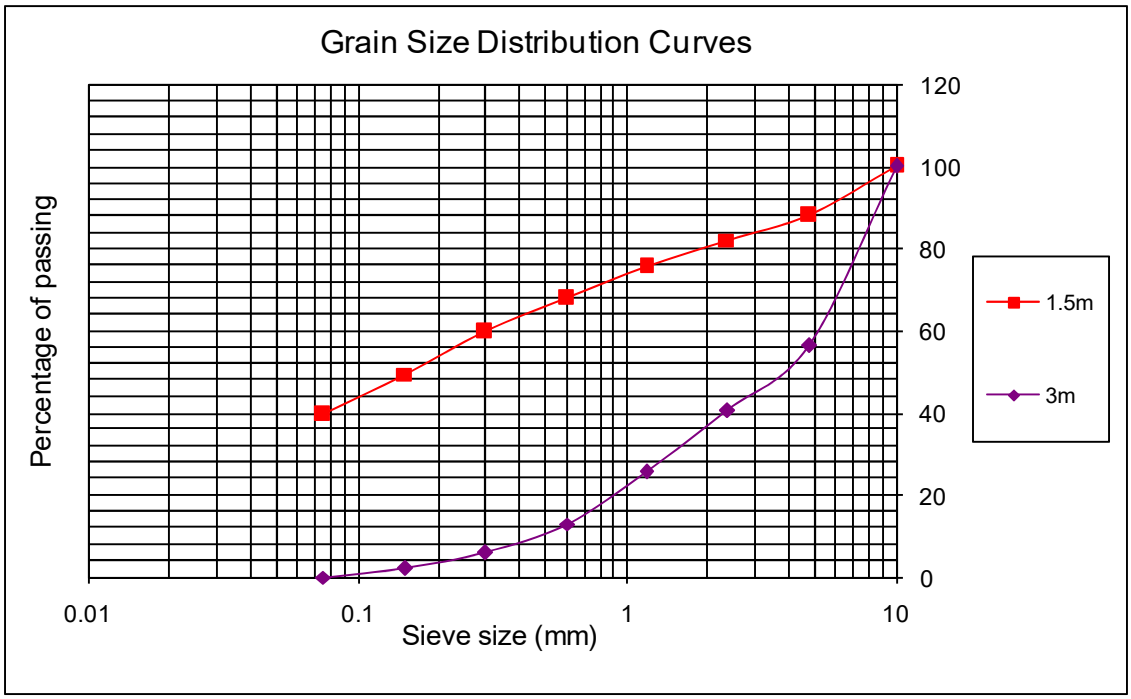
Description	1.5m Depth	3m Depth
Fineness modulus	1.279	4.417
Uniformity Coefficient (C_u)	---	16.667
Coefficient of Curvature (C_c)	---	1.500
Observation	---	C_u > 4 & C_c lies between 1 & 3, hence well graded gravels

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3.2 TEST REPORT ON SIEVE ANALYSIS OF SAMPLE

Bore Hole – 2 @ 1.5m & 3m

SIEVE SIZE	PERCENTAGE OF PASSING	
	1.5m Depth	3m Depth
10 mm	100	100
4.75 mm	88.1	56.2
2.36 mm	82	40.6
1.18 mm	75.4	25.7
600 micron	67.9	12.9
300 micron	59.7	5.8
150 micron	49.3	2.1
75 micron	39.5	0



Description	1.5m Depth	3m Depth
Fineness modulus	1.381	4.567
Uniformity Coefficient (C_u)	---	17.000
Coefficient of Curvature (C_c)	---	1.673
Observation	---	C_u > 4 & C_c lies between 1 & 3, hence well graded gravels

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SAFE BEARING CAPACITY RESULTS

4. Calculation of Safe bearing capacity of the soil:

4.1 Safe Bearing capacity of soil based on SPT values for Bore Hole 1 & 2

Bore Hole No.	Depth (m)	SPT value	Safe Bearing capacity of soil	
			KN/m^2	t/m^2
BH – 1	1.50	6	25.49	2.54
	3.00	37	288.90	28.89
	4.50	39	305.89	30.58
BH – 2	1.50	6	25.49	2.54
	3.00	38	297.39	29.73
	4.50	39	305.89	30.58

4.2 Safe Bearing capacity of soil based on Shear Failure Criteria:

Through Unconfined Compression Test (UCC)

Bore Hole No.	Depth (m)	Max. Load (Div)	Max. Deformation (Div)	Cohesion (t/m^2)	Safe Bearing capacity of soil (t/m^2)
BH – 1	1.50	20	70	2.80	4.79
BH – 2	1.50	15	60	2.10	3.60

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Through Direct Shear test

Bore Hole No.	Depth (m)	Load	Division	Ø (in degrees)	Safe Bearing Capacity of Soil (t/m^2)*
BH -1	1.50	0.5 1.0 1.5	23 69 115	27.88°	10.90
	3.00	0.5 1.0 1.5	24 80 136	32.78°	31.55
	4.50	0.5 1.0 1.5	24 81 138	33.24°	44.19
BH -2	1.50	0.5 1.0 1.5	23 69 114	27.62°	10.56
	3.00	0.5 1.0 1.5	25 81 138	33.01°	32.46
	4.50	0.5 1.0 1.5	25 82 139	33.47°	44.19

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4.3 Comparison of Safe bearing capacity of soil through Shear and Settlement consideration:

Bore Hole no.	Depth (m)	Safe Bearing Capacity (t/m^2)		
		Shear consideration		Settlement consideration
		(UCC)	(Direct Shear)	
BH – 1	1.50	4.79	10.90	2.54
	3.00	---	31.55	28.89
	4.50	---	44.19	30.58
BH – 2	1.50	3.60	10.56	2.54
	3.00	---	32.46	29.73
	4.50	---	44.19	30.58

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5. Result:

5.1 Recommended Safe bearing capacity values at different depths

Considering the lesser value of the above, the Safe Bearing Capacity, values at different depths are recommended below:

Bore Hole	Depth (m)	Safe Bearing Capacity (t/m^2)
BH – 1	1.50	2.54
	3.00	28.89
	4.50	30.58

BH - 2	1.50	2.54
	3.00	29.73
	4.50	30.58

The above test data and findings are relevant and applicable only for the soil conditions existing at the test site. Any change in the soil strata observed during excavation may be reported to the laboratory, which will redefine the safe bearing capacity of the soil.

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Certified EC, Patta, legal opinions, property verification, due diligence, litigation solutions, and legal heir certificates.

Property Development & Approvals

Building plan approvals, construction, layout and sub-division approvals, zone conversion, and occupancy certification.

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Escrow Services, Rentals, Litigation Solutions, Property Valuation and Property Management

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Encumbrance Certificate (EC) Search, Find Your SRO, Temple Property Check, WAQF Property Check, View Patta/Chitta, View FMB, Patta Analyzer, EC Analyzer

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Land Use Zone Finder, CRZ Zone Check, Aquifer Zone Check, CBA Check, EWS Check, Road Width Check, Proposed Road Widening Check, FSI Calculator

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