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REPORT ON GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT SF NO 340/1A & 340/2A1, VEERAKERALAM VILLAGE, PERUR TALUK, COIMBATORE CORPORATION.

REPORT NO : AGIS/C-406/20 **Dt: 08.09.2020**

CLIENT : **M/s MOUNT HOUSING AND INFRASTRUCTURE LIMITED**
122 I, SILVER ROCK APARTMENT,
2ND FLOOR, VENKATASAMY ROAD WEST,
R. S. PURAM,
COIMBATORE-641002

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REPORT ON GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT SF NO 340/1A & 340/2A1, VEERAKERALAM VILLAGE, PERUR TALUK, COIMBATORE CORPORATION.

1.0 GENERAL

M/s. Mount Housing and Infrastructure Limited, 122 I, Silver Rock Apartment, 2nd Floor, Venkatasamy Road West, R. S. Puram, Coimbatore-641002 has requested for the consultancy services for suitable foundation based on soil investigation for the proposed construction of Apartment Building at SF NO 340/1A & 340/2A1, Veerakeralam Village, Perur Taluk, Coimbatore Corporation. Geotechnical investigation for the proposed construction was carried out by Alzum Geocivil Integrated Services, Coimbatore.

The scope of work includes identification of the depth of foundation, evaluation of safe bearing capacity to be the adopted in the design of foundation system, selection of suitable type of foundation and other detrimental effects of the soil by conducting Field investigation and Laboratory investigation.

Three trial pits were made up to the desired level at different locations covering the entire area of the proposed site. Standard Penetration Test was planned to assess the soil parameters to achieve the scope of investigation. The location of the trial pits were decided on mutual consultation. Topography of the site is plain.

2.0 FIELD INVESTIGATIONS

Three trial pits namely TP01, TP02 and TP03 were excavated and examined with simple tools over depths of 2.50m, 2.45m and 1.80m respectively. After examining the trial pits, Standard Penetration Test (SPT) were conducted in all the locations. Representative soil samples were collected and analyzed. Locations of the trial pits are shown in Annexure I.

2.1 STANDARD PENETRATION TEST

Standard Penetration Test (SPT) is the most extensively used in situ test all over the world. This test is carried in a bore hole/open pit using a spilt spoon sampler as per IS2131-1981. SPT has to be performed at the desired

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depth. The spilt spoon sampler is seated by 150mm by blows of a drop hammer of 63.5 kg falling vertically and freely from a height of 750mm. The split spoon sampler shall be further driven 300mm in two steps each of 150mm. The number of blows required for each 150mm penetration is observed. The total blows required for the second and third 150mm of penetration is termed the 'N' value. The N value thus observed will be subjected to two corrections namely i) Correction due to dilatancy and ii) Correction for overburden.

2.1.1 SPT 1 (Test Pit 1)

The pit had been excavated up to a depth of 2.50m from the existing ground level. SPT was conducted at 2.50m depth on Silty sand. The N Value obtained is 42.

2.1.2 SPT 2 (Test Pit 2)

The pit had been excavated up to a depth of 2.45m from the existing ground level. SPT was conducted at 2.45m depth on Silty sand. The N Value obtained is 51.

2.1.2 SPT 3 (Test Pit 3)

The pit had been excavated up to a depth of 1.80m from the existing ground level. SPT was conducted at 1.80m on Clayey sand. The N Value obtained is 25.

3.0 LABORATARY TESTS

The operations to be performed in the laboratory depend upon the type of the nature of data required for the problem at hand. In case of cohesion less material, like sand, the laboratory tests are usually minimum and the design parameters are worked out from field test data such as form SPT- N value, Core resistance and plate load test data. In the case of cohesive soils, the programme of laboratory testing can vary from carrying out simple tests such as unconfined tests to comprehensive study of soil behaviour using Triaxial shear tests and consolidation tests.

Following laboratory tests are conducted.

For cohesion less, soil

- Specific gravity

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- Sieve analysis
- Direct shear test
- For cohesive soil
- Specific gravity
- Natural moisture content
- Atterberg's limits
- Sieve Analysis
- Unconfined compression test
- Consolidation test
- Triaxial test

The field record is shown in Annexure II, results of the laboratory tests are shown in and III and the results of particle size distribution is shown in Annexure IV.

4.0 SOIL PROFILE

TP01

The top layer is of Black Clay soil for thickness of 1.15m. The second layer is of Reddish-brown Silty Sand with Kankar traces for thickness of 1.00m. Brownish Silty sand underlies the above layer. Brownish Silty sand appears to continue for considerable depth as observed from borewells drilled.

TP02

The top layer is of Black clay soil for thickness of 1.10m. The second layer is of Reddish-brown Silty Sand with Kankar traces for thickness of 1.10m. Brownish Silty sand underlies the above layer. Brownish Silty sand appears to continue for considerable depth as observed from borewells drilled.

TP03

The top layer is of Black Clay soil for thickness of 0.85m. Clayey sand with Kankar traces underlies the Clay layer.

5.0 FOUNDATION ANALYSIS

The Bearing Capacity of soil is calculated using the N value obtained from the standard penetration test. It is worked out based on the shear stress failure criterion and settlement criterion, by taking the N value as 46.

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5.1 BASED ON SHEAR STRENGTH FAILURE CRITERION

The safe bearing capacity based on shear failure is worked out empirically by the following equation,

Safe bearing capacity (SBC) = q =$0.314N^2B.Rw2 + 0.943 (100 + N2)$ Df. Rw1

N : SPT N value

Df : Depth of foundation in m

B : Breadth of the foundation (taken as 2.0m)

Rw1& Rw2 : Water table correction

5.2 BASED ON SETTLEMENT CRITERION

The safe bearing capacity based on settlement failure is worked out by the following equation.

The safe bearing capacity, by

$$q = 1.36 (N-3 \left(\frac{B + 0.3}{2 \times B}\right)^2 \times S.Rd. Rw1$$

Where,

B: Breadth of the footing (taken as 2.0m)

R: Water table correction factor (For the worst case, taken as 0.5)

Rd: Depth factor (taken as 1.2)

S: Permissible settlement in mm

The calculations of allowable net safe bearing capacity are shown under Annexure V.

6.0 DISCUSSIONS

The first layer is of Black clay for thickness varying from 0.85m to 1.15m. The second layer is of Silty sand/Clayey sand with Kankar traces and its thickness is about 1.10m. The N value obtained on this layer is 25. The third layer is of Silty sand and it appears to continue for a considerable depth. The N value obtained on this layer is averaged to 46. The foundation can be rested on the third layer (Silty sand). The Stiffness of soil increases with increase in depth.

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7.0 RECOMMENDATIONS

7.1 DEPTH OF FOUNDATION

a) The Depth of foundation shall be taken as 2.50m from the existing ground level so as to rest the foundation on Silty sand.

b) The Stiffness of soil increases with increase in depth.

7.2 ALLOWABLE NET SAFE BEARING CAPACITY

a) The engineering properties of soil and SPT N value are considered for the evaluation of safe bearing capacity.

b) The allowable bearing capacity of soil shall be taken as 300 kN/m^2 in the foundation design

7.3 TYPE OF FOUNDATION

a) The type of foundation depends upon the founding strata, loading intensity at the foundation level and configuration at loading points.

b) Framed structure with Isolated footing/Combined footing shall be provided for the proposed structure.

c) The size of the foundation shall be decided by the design engineer, based on the allowable bearing capacity recommended.

8.0 PRECAUTIONS

a) Entire report should be studied before implementing the recommendations.

b) Loose pockets of soil/ weathered / fissured and fractured rock if encountered shall be removed and backfilled with concrete. A leveling course of concrete of M20 grade shall be laid and construction of foundations can be taken up subsequently.

c) The top soil available for a thickness of about 1.00m appears to be highly expansive and compressible. It is not advisable to rest the flooring, Utility pipelines, Sewage chambers, Compound walls and other structural members on this layer.

9.0 LIMITATIONS

a) The soil investigation have been carried out at locations in the site chosen by the clients so as to represent the entire site. The recommendations provided in this report are hence valid only for these

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test locations. However, if there is any change in sub soil conditions and properties at places between or beyond chosen test locations, Geocivil Integrated Services may be conducted for further advice.

b) With passage of time, the recommendations may vary due to manmade and naturals environment changes

For Alzum Geocivil Integrated Services

Dr.P.D. Arumairaj, M.E., Ph.D.,
FIGS., FIE, C.Eng., MIEEE., MISTE
Principal Consultant
Professor of Civil Engg (Retd)
Govt College of Technology, Coimbatore.

REFERENCES

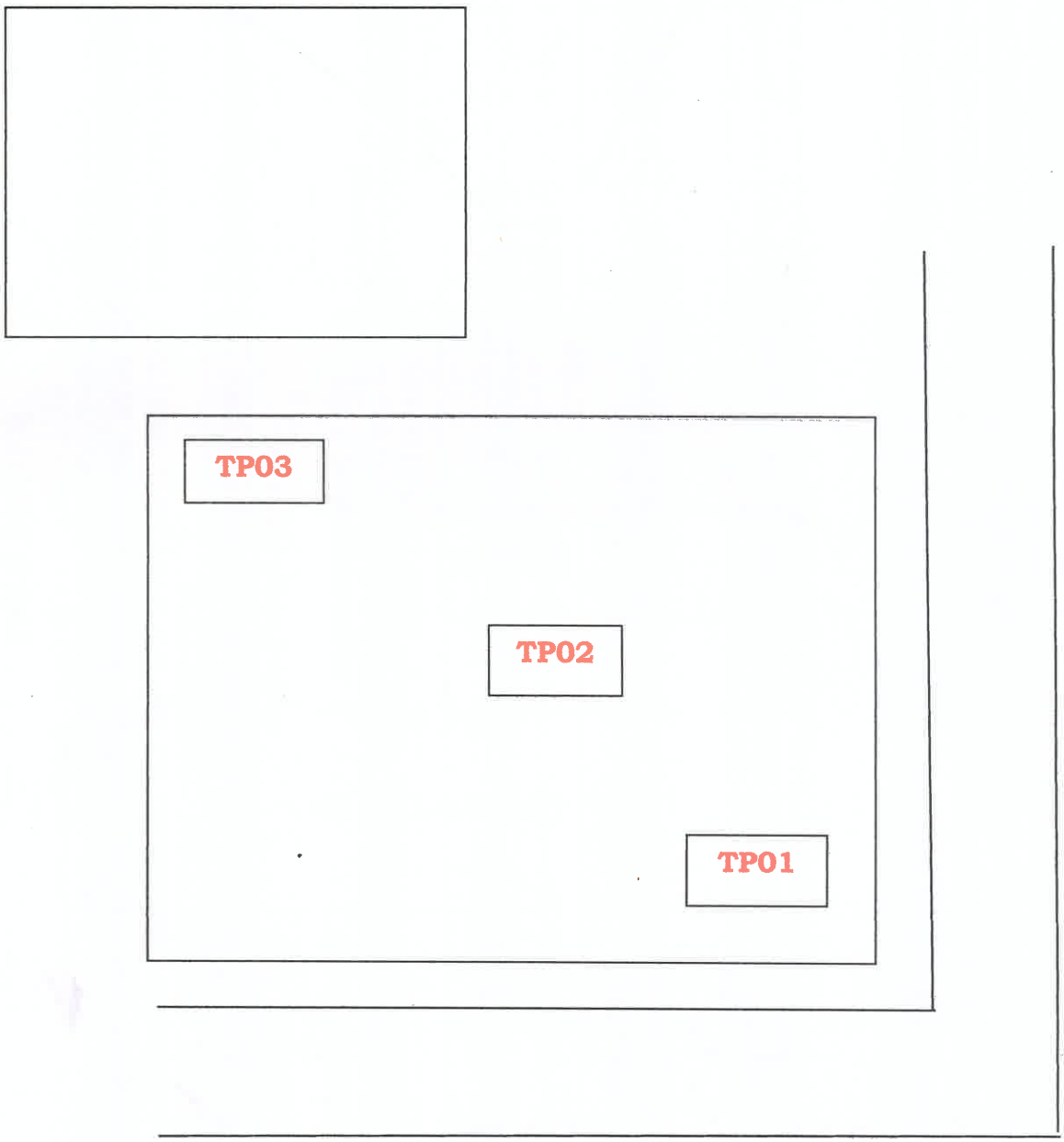
1. IS 2720 (Part 1) – 1983, Indian Standard criteria for Determination of Preparations of soil samples.
2. IS 2720 (Part 2) – 1983, Indian Standard criteria for Determination of Water Content.
3. IS 2720 (Part 3) – 1983, Indian Standard criteria for Determination of Grain Size Analysis.
4. IS 2720 (Part 4) – 1983, Indian Standard criteria for Determination of Atterberg's Limits.
5. IS 2720 (Part 5) – 1983, Indian Standard criteria for Determination of shear strength parameters using Triaxial Shear apparatus.

Field work was executed generally in accordance with IS specification listed below:

1. IS-1892 : Code of practice for Subsurface Investigation of Foundations.
2. IS-1498 : Classification and Identification of soils for General Engineering Purposes.
3. IS-2131 : Method for Standard Penetration Test for Soil
4. IS-2132 : Code for practice for Thin Walled Tube Sampling of Soil.

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ANNEXURE – I
LOCATION



ANNEXURE – II
FIELD RECORD

Ref: AGIS/CBE/2020-21/C – 406/DOC 001 REV 0														
Name of Work:		**REPORT ON GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT SF NO 340/1A & 340/2A1, VEERAKERALAM VILLAGE, PERUR TALUK, COIMBATORE CORPORATION.**											
Soil Investigation done by:		ALZUM GEOCIVIL INTEGRATED SERVICES, CBE	**Type of Exploration:** Open exploration							Date of Commencement				02.09.2020
			Ground Water Level: NIL							Date of Completion				02.09.2020
Trial Pit No: 1		TP 01	Thickness of layer (m)	Type of testing	`Standard penetration test data					Bed Level				
Location:		Mentioned on site plan								Chainage				
Depth below G.L.(R.L)	Soil Profile	Description of Soil			N – Value- Depth of penetration					Relative density / Consistency	Graphical representation of penetration resistance			
					1st 15cm	2nd 15cm	3rd 15cm	For last 30cm/ observed N' value			10	20	30	40	50
1.15		Black Clay	1.15	-	-	-	-	-							
2.15		Reddish brown Silty sand with Kankar traces	1.0	-	-	-	-	-							
2.50		Brownish Silty sand	0.35	SPT	9	20	22	42							

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Ref: AGIS/CBE/2020-21/C – 406/DOC 001 REV 0																
Name of Work:	**REPORT ON GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT SF NO 340/1A & 340/2A1, VEERAKERALAM VILLAGE, PERUR TALUK, COIMBATORE CORPORATION.**															
Soil Investigation done by:	ALZUM GEOCIVIL INTEGRATED SERVICES, CBE		**Type of Exploration:** Open exploration							Date of Commencement					02.09.2020	
			Ground Water Level: NIL							Date of Completion					02.09.2020	
Trial Pit No: 1	TP 02		Thickness of layer (m)	Type of testing	Standard penetration test data					Bed Level						
Location:	Mentioned on site plan									Chainage						
Depth below G.L.(R.L)	Soil Profile	Description of Soil			N – Value- Depth of penetration					Relative density / Consistency	Graphical representation of penetration resistance					
					1st 15cm	2nd 15cm	3rd 15cm	For last 30cm/ observed N' value			10	20	30	40	50	
1.10		Black Clay	1.10	-	-	-	-	-								
2.20		Reddish brown Silty sand with Kankar traces	1.10	-	-	-	-	-								
2.45		Brownish Silty sand	0.25	SPT	5	15	36	51								

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Ref: AGIS/CBE/2020-21/C – 406/DOC 001 REV 0

Name of Work:	**REPORT ON GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT SF NO 340/1A & 340/2A1, VEERAKERALAM VILLAGE, PERUR TALUK, COIMBATORE CORPORATION.**														
Soil Investigation done by:	ALZUM GEOCIVIL INTEGRATED SERVICES, CBE	**Type of Exploration:** Open exploration									Date of Commencement				02.09.2020
		Ground Water Level: NIL									Date of Completion				02.09.2020
Trial Pit No: 1	TP 03	Thickness of layer (m)	Type of testing	Standard penetration test data							Bed Level				
Location:	Mentioned on site plan										Chainage				
Depth below G.L.(R.L)	Soil Profile	Description of Soil			N – Value- Depth of penetration						Relative density / Consistency	Graphical representation of penetration resistance				
					1st15cm	2nd 15cm	3rd 15cm	For last 30cm/ observed N' value				10	20	30	40	50
0.85		Black Clay	0.85	-	-	-	-	-							
1.80		Clayey sand with Kankar traces	0.95	SPT	4	13	12	25							

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ANNEXURE III
LAB TEST RESULTS

Ref: AGIS/CBE/2020-21/C-406/DOC 001 REV 0

Name of work	**REPORT ON GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT SF NO 340/1A & 340/2A1, VEERAKERALAM VILLAGE, PERUR TALUK, COIMBATORE CORPORATION.**

Soil Investigation done by: ALZUM GEOCIVIL INTEGRATED SERVICES, CBE	**Bore hole id** TP-01	**Location:** Mentioned on Site Plan	**Ground Water Level: NOT MET**	**Work Commenced On**	02.09.2020
				Work Completed On	02.09.2020

SOIL PROFILE AND DEPTH IN METRES	DEPTH OF SAMPLING	IS SOIL CLASSIFICATION	NATURAL WET DENSITY (kN/m³)	NATURAL MOISTURE CONTENT %	INDEX TEST			SIEVE ANALYSIS			SPECIFIC GRAVITY	SHEAR TEST		STANDARD PENETRATION TEST		Rock Parameters		
					LIQUID LIMIT %	PLASTIC LIMIT %	PLASTICITY INDEX %	GRAVEL %	SAND %	SILT & CLAY %		COHESION C kN/m²	ANGLE OF INTERNAL FRICTION °	DEPTH OF TESTING m	OBSERVED N VALUE	Core Recovery	RQD	Free Swell %
	2.50	SM	16.87	10.1	37.0	25.0	12.0	1.60	83.5	14.7	2.70	-	-	2.50	42	-	-	10

Name of work	REPORT ON GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT SF NO 340/1A & 340/2A1, VEERAKERALAM VILLAGE, PERUR TALUK, COIMBATORE CORPORATION.

Soil Investigation done by: ALZUM GEOCIVIL INTEGRATED SERVICES, CBE	Bore hole id TP-02	Location: Mentioned on Site Plan	Ground Water Level: NOT MET	Work Commenced On	02.09.2020
				Work Completed On	02.09.2020

SOIL PROFILE AND DEPTH IN METRES	DEPTH OF SAMPLING	IS SOIL CLASSIFICATION	NATURAL WET DENSITY (kN/m³)	NATURAL MOISTURE CONTENT %	INDEX TEST			SIEVE ANALYSIS			SPECIFIC GRAVITY	SHEAR TEST		STANDARD PENETRATION TEST		Rock Parameters		
					LIQUID LIMIT %	PLASTIC LIMIT %	PLASTICITY INDEX %	GRAVEL %	SAND %	SILT & CLAY %		COHESION C kN/m²	ANGLE OF INTERNAL FRICTION °	DEPTH OF TESTING m	OBSERVED N VALUE	Core Recovery	RQD	Free Swell %
	2.45	SM	16.91	14.0	39.0	22.0	17.0	1.8	86.4	11.8	2.67	-	-	2.45	51	-	-	10

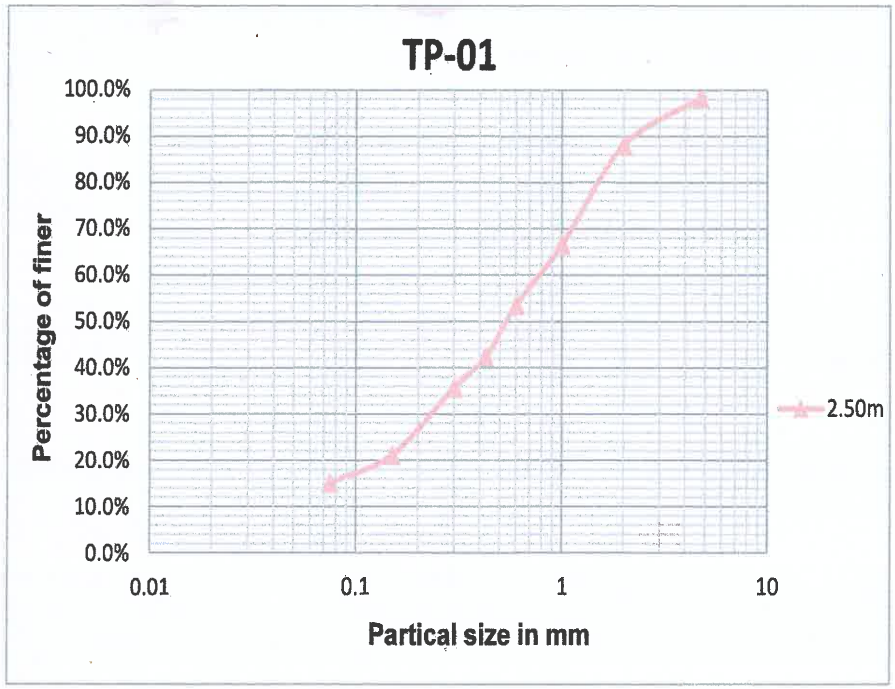
Name of work	REPORT ON GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT SF NO 340/1A & 340/2A1, VEERAKERALAM VILLAGE, PERUR TALUK, COIMBATORE CORPORATION.

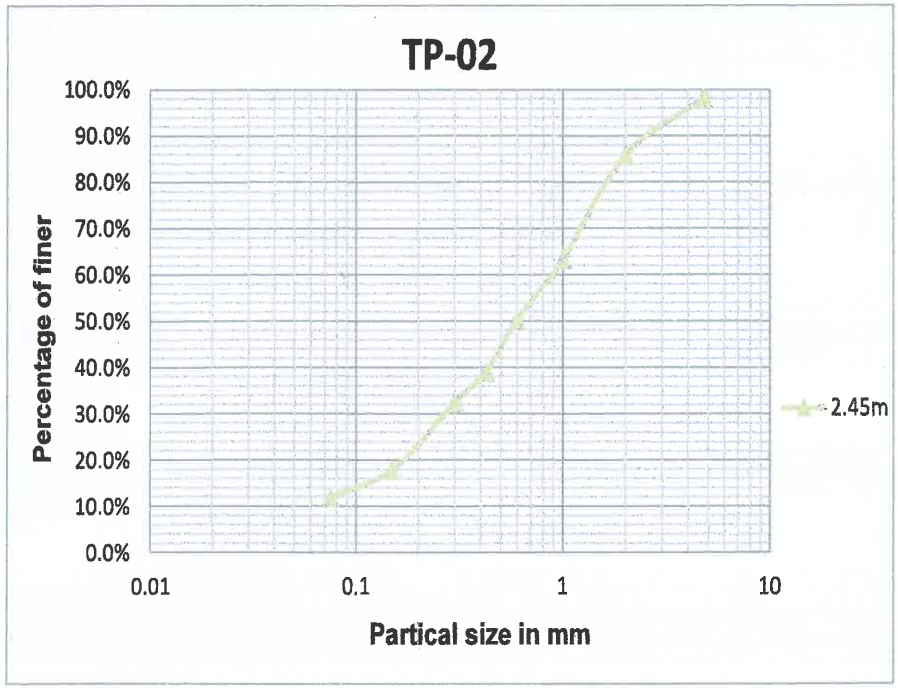
Soil Investigation done by: ALZUM GEOCIVIL INTEGRATED SERVICES, CBE	Bore hole id TP-03	Location: Mentioned on Site Plan	Ground Water Level: NOT MET	Work Commenced On	02.09.2020
				Work Completed On	02.09.2020

SOIL PROFILE AND DEPTH IN METRES	DEPTH OF SAMPLING	IS SOIL CLASSIFICATION	NATURAL WET DENSITY (kN/m³)	NATURAL MOISTURE CONTENT %	INDEX TEST			SIEVE ANALYSIS			SPECIFIC GRAVITY	SHEAR TEST		STANDARD PENETRATION TEST		Rock Parameters		
					LIQUID LIMIT %	PLASTIC LIMIT %	PLASTICITY INDEX %	GRAVEL %	SAND %	SILT & CLAY %		COHESION C kN/m²	ANGLE OF INTERNAL FRICTION °	DEPTH OF TESTING m	OBSERVED N VALUE	Core Recovery	RQD	Free Swell %
	1.80	SC	15.47	12.3	45.0	16.0	29.0	14.8	61.3	23.1	2.67	-	-	1.80	25	-	-	30

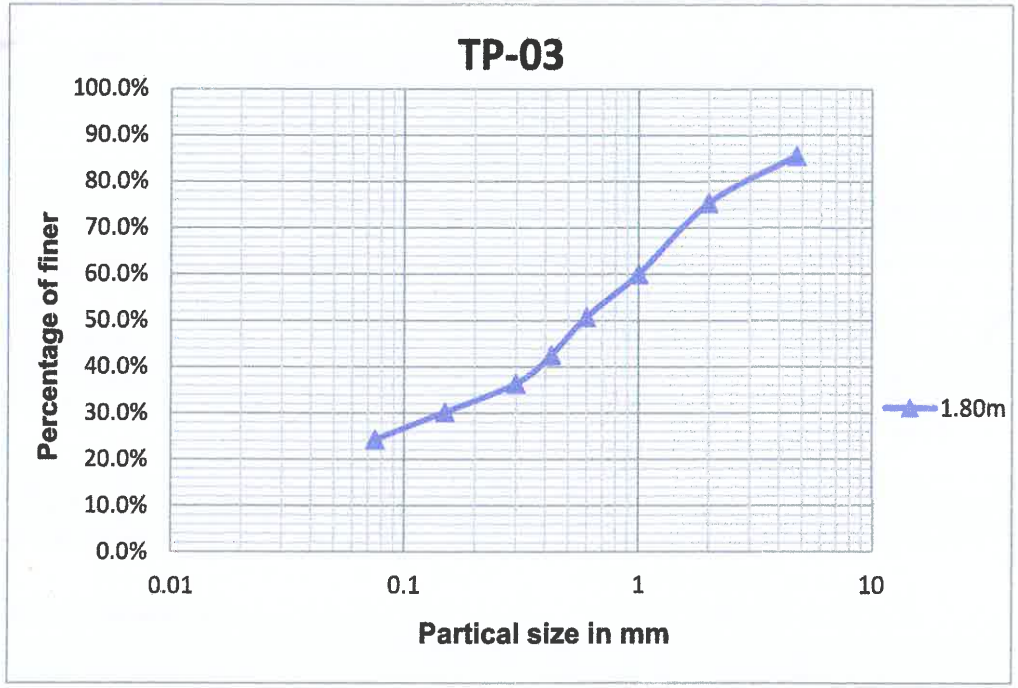
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ANNEXURE – IV
SIEVE ANALYSIS RESULTS





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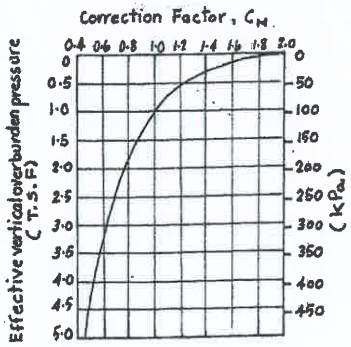


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ANNEXURE – V
SAFE BEARING CAPACITY

Allowable bearing capacity calculation

Width of the footing, B	=	2	m
Depth of foundation, Df	=	2.5	m
Unit weight of soil	=	17	kN/m^3
Submergible unt wt	=	7	kN/m^3
Avg. observed N value	=	46	
Eff. overburden pressure at foundation level	=	17.5	kN/m^3
Overburden correction factor	=	1.3	
N value after overburden corretion, N'	=	59.8	
Depth of GWT	=	10	m
Dialtancy corretion, N"	=	59.8	
Factor of Safety	=	3	
Rw1	=	0.5	
Rw2	=	0.5	
Ultimate Bearing capacity based on shear	=	$0.314xN"^2xBxRw1+0.943x(100+N"^2)xDfxRw2$	
	=	5456.01	KN/m^2
Safe bearing capacity based on shear	=	1818.67	KN/m^2
Safe bearing capacity based on settlement	=	$1.36x(N"-3)x((B+0.3)/2B)^2xSxRdxRw)$	
Settlement for Sand	=	25	mm
Depth factor, Rd	=	1+Df/B <2	
Rd	=	1.2	Say
Water table factor, Rw	=	0.5+0.5(Dw/B)	0.5<Rw<1
Rw	=	0.5	Since GWT is below 10m from EGL
Qs	=	383.102 kN/m^2	For 25mm controlled Settlement
Allowable Bearing Pressure at Df depth is	=	**300** kN/m^2	



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COIMBATORE LOCAL PLANNING AUTHORITY

Corporation Complex, Sivanandha Colony, Coimbatore-641012

Phone : 0422-2492700, E-mail: clpams@gmail.com

PROFESSIONAL REGISTRATION CERTIFICATE

This is to Certify that the Thiru. **P.D.ARUMAIRAJ** has enrolled as **Geo-Technical Engineer (GTE)** with Coimbatore Local Planning Authority (CLPA) who has the qualification of **Ph.D (Civil Engineering)** and experience in the relevant field for **14 Years**.

He has been given the Registration No. **Coimbatore LPA/GTE/19/07/008** on **18.07.2019**. He shall be required to comply with the duties and responsibilities prescribed by the Government of Tamil Nadu in Part V, "Registration of Professionals", Tamil Nadu Combined Development and Building Rules, 2019 approved vide G.O.(Ms) No.18 MAWS (MA1) Dept. Dated. 04.02.2019.

This Registration is valid upto **17.07.2024**.



18 JUL 2019

Member Secretary, (I/C).,
Coimbatore Local Planning Authority
Coimbatore - 641012.

18/7/19



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