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INTROCUCTION

The proposed structure is framed structure for S+5 Floors. The foundation is designed for Stilt + 5 Floors only. The least lateral dimensions of the column is 300 mm in stilt floor and 200 mm in all other floors. The minimum grade of concrete is M25 for RCC and M10 for PCC. The grade of steel considered for design is Fe 500. The structural design has been carried out with Limit state method.

LOADING AND DESIGN DETAILS

IS 875 Part 2 Loading details considered for design:

Dead load of Slab (0.15m thick)	=	3.5 kN/Sq.m
Floor Finish	=	1.000 kN/Sq.m
Live Load on floors (Residential	=	2.000 kN/Sq.m
Live Load on floors (Commercial)	=	5.000 kN/Sq.m
Live load on roof	=	1.500 kN/Sq.m
Live load on inaccessible roof	=	0.750 kN/Sq.m
Weathering coarse load	=	1.500 kN/Sq.m
Unit Weight of Concrete	=	25.000 kN/Cu.m
Unit Weight of block Work	=	20.000 kN/Cu.m

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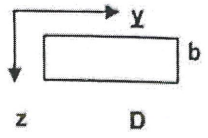
DESIGN OF COLUMN

COLUMN DESIGNATION :- C3

MEMBER NO :-

LOAD COMBINATION

Load Case	P (KN)	Muz (kN.m)	Muy (kN.m)
	1200	21	9



Description			Formula / Variable		Unit	Parameters
Column size :-						
Along z-axis			b		mm	200
Along y-axis			D		mm	750
Unsupported length			l_{ez}		m	3.00
Unsupported length			l_{ey}		m	3.00
Effective Length	Factor	1.2	Lez		m	3.60
Effective Length	Factor	1.2	Ley		m	3.60
(As per IS 456:2000, cl. E-3,table-28)						
Design Parameters :-						
Concrete mix			f_{ck}		N/mm²	25
Steel Grade			f_y		N/mm²	500
Effective cover for Muz			d^1		mm	52.5
Effective cover for Muy			d^1		mm	52.5
Slenderness ratio (about zz axis)			Lez/D	**short column**		4.80
Slenderness ratio (about yy axis)			Ley/b	**long column**		18.00
No addn. Mom. Reqd. due to slenderness						
Loadings :-						
Factored axial load			Pu		kN	1200.00
Factored moment around the z-axis			Mz(u)		kN.m	21.00
Factored moment around the y-axis			My(u)		kN.m	9.00
Minimum eccentricity z-axis			e=(Lez/500)*1000+D/30(or)20		mm	31.00
Minimum eccentricity y-axis			e=(Ley/500)*1000+b/30(or)20		mm	20.00
Moment around z-axis			Pu x e_z		kN.m	37.20
Moment around y-axis			Pu x e_y		kN.m	24.00
Additional eccentricity due to slenderness			eaz/D (SP16-Cl.3.4)			0.000
			e_{az} = 0.000 x 0.75			0.00
Additional eccentricity due to slenderness			eay/b (SP16-Cl.3.4)			0.162
			eay = 0.162 x 0.200			0.032
Additional moment due to slenderness			My(a) = Pu x e_{ay}		KN.m	38.880
Additional moment due to slenderness			Mz(a) = Pu x e_{az}		KN.m	0.00
DESIGN						
Provided reinforcement			***8 Nos. of 25 mm dia bar***			
			6 Nos. of 20 mm dia bar			
Reinforcement percentage provided			p		%	3.87
Calculation of Puz						
Refering to Chart 63			Puz / Ag		N/mm²	22.586
			Puz		KN	3387.925
			p / fck			0.15

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Calculation of Pb(about yy-axis)	long column		
	Ratio of d' / b		**0.26**
From table 60			
Pb(about yy-axis)	Pb = (k1 + k2 p/fck)fckbD	KN	**706.27**
	From table 60-SP16		
	k1 = **0.184** ; k2 = **0.028**		
Calculation of Pb(about zz-axis)	short column		
	Ratio of d' / D		**0.07**
From table 60			
Pb(about zz-axis)	Pb = (k1 + k2 p/fck)fckbD	KN	**0.00**
	From table 60-SP16		
	k1 = **0.000** ; k2 = **0.000**		
Modification factor	ky = (Puz - Pu)/(Puz-Pby)		**0.816**
Modification factor	kz = (Puz - Pu)/(Puz-Pbz)		**0.646**
Modified Additional moment	My(a)' =My(a) x ky	KN.m	**31.722**
	Mz(a)' =Mz(a) x kz	KN.m	**0.000**
Net design moment	Mz = Mez + Mz(a)'	KN.m	**37.200**
	My = Mey + My(a)'	KN.m	**55.722**
Uniaxial Mom. capacity about zz-axis	(p / fck) = 0.15		
	Ratio of d' / D		**0.07**
Use chart no. 49	Pu / fck b D		**0.320**
	Mu / fck b D^2		**0.15**
	Muz1	kN.m	**421.88**
Uniaxial Mom. capacity about yy-axis			
0	Ratio of d' / b		**0.26**
Use chart no. 44	Pu / fck b D		**0.320**
	Mu / fck D b^2		**0.12**
	Muy1	kN.m	**90.00**
Allowable moment capacity			
	Pu / Puz		**0.354**
	Mz / Muz1		**0.088**
	My / Muy1		**0.619**
	α_n		**1**
Interaction Ratio			**0.595**
			Safe

Provide 8 mm ties @ 200 c/c

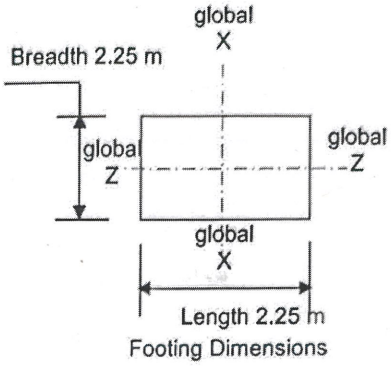
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DESIGN OF BI-AXIAL ISOLATED RCC FOOTING (IS 456, 2000)

Building Name
Footing Number:
Node number

COLUMN/PEDESTAL

Length (l, dim. || Z axis) =	230	mm
Breadth (b, dim. || X axis) =	450	mm
Height of pedestal =	1	m
Weight of pedestal/column =	**2.59**	KN



Footing Dimensions

FOOTING

Foot length (L, dim. || Z axis) =	2.25	m
Foot Breadth (B, dim. || X axis) =	2.25	m
Thickness of footing (t) =	450	mm
Clear cover to Reinforcement =	75	mm
Main bar dia of footing =	12	mm
Effective depth of footing =	**369**	mm
Selfweight of the footing =	**56.95**	KN
Area of Footing(A) =	**5.06**	m^2
Sect mod of foot about Z axis (Zz) =	**1.90**	m^3
Sec mod of foot about X axis (Zx) =	**1.90**	m^3

MATERIALS OF CONSTRUCTION

Grade of concrete f_{ck} =	25	N/mm^2
Grade of steel f_y =	500	N/mm^2

CHECK FOR GROSS BEARING PRESSURE

Safe **NET** bearing pressure =	250	KN/m^2
Safe gross bearing pr. =	**296.55**	KN/m^2
Unfactored load case number =	3	
Axial load from output (P1) =	1045	KN
Moment about Z axis (M_z) =	1	KN-m
Moment about X axis (M_x) =	3	KN-m
Depth of top of foot. from ground =	2	m
Unit wt of soil =	19	KN/m^3
Weight of soil retained above foot =	**188.44**	KN
P = (P1+soil+pedestal+foot selfwt) =	**1292.98**	KN
Maximum bearing pressure =	**257.51**	KN/m^2
Minimum bearing pressure =	**253.30**	KN/m^2

(net pr. + depth of foot * soil unit wt)

$$\frac{P}{A} \pm \frac{M_y}{Z_y} \pm \frac{M_x}{Z_x}$$

Hence footing is safe against max gross bearing pr.

DESIGN FORCES

Factored load comb. no.	18	
Axial load:(P_u) =	755	KN
Moment about Z axis (M_{uz}) =	1.5	KN-m
Moment about X axis (M_{ux}) =	4.5	KN-m
Maximum effective soil pressure $p_{e\,max}$		
(Pu/Area+ Muz/Zz + Mux/Zx) =	**152.30**	KN/m^2
Minimum effective soil pressure $p_{e\,min}$		
(Pu/Area - Muz/Zz - Mux/Zx) =	**145.98**	KN/m^2

Design of footing is done using above maximum effective soil pressure

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CALCULATION FOR BOTTOM STEEL

Mu about X1 X1 = (pe max x length^2/2)= **77.68** KN-m per meter

Mulimit = **452.82** KN-m per meter

The section is singly reinforced

$$A_{st} = \frac{0.5 f_{ck}}{f_y}\left[1 - \sqrt{1 - \frac{4.6 M_u}{f_{ck} bd^2}}\right] bd$$

Hence, Ast = **497.597** mm^2

Min Ast = **442.800** mm^2 (0.12 % for slab, cl 26.5.2.1)

Spacing = **227.29** mm (considering max of above two calculated values of Ast)

pt provided = **0.13** %

Hence provide 12 mm dia bar @ 227 mm c/c parellel to length of footing (|| to Z)

Mu about N1 N1 = (pe max x length^2/2)= **61.68** KN-m per meter

Calc. Ast = **392.819** mm^2 The section is singly reinforced

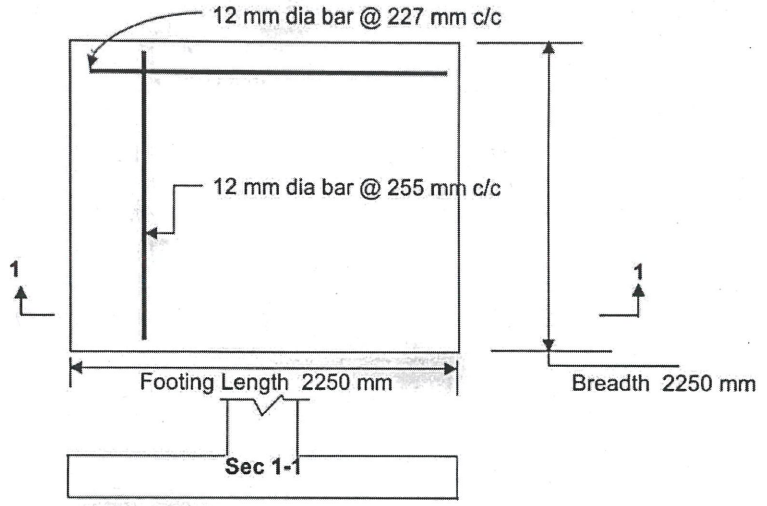
Min Ast = **442.8** mm^2 (0.12 % for slab, cl 26.5.2.1)

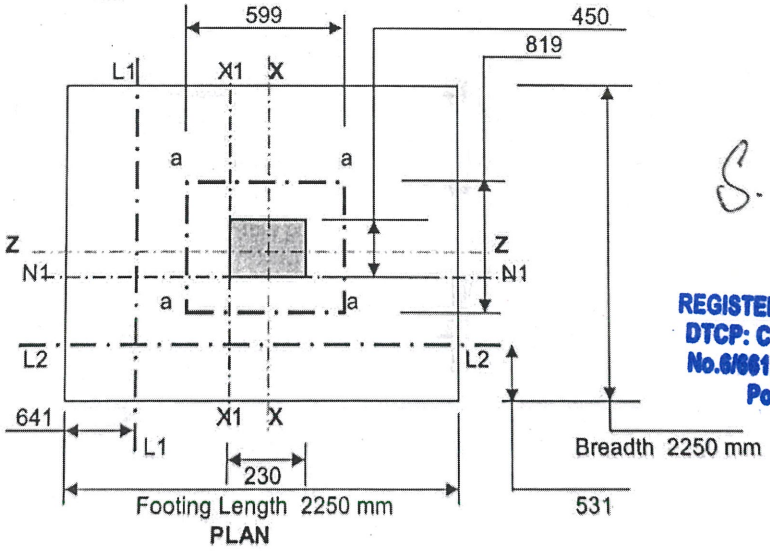
Spacing = **255.41** mm (considering max of above two calculated values of Ast)

pt provided = **0.12** %

Hence provide 12 mm dia bar @ 255 mm c/c parellel to breadth of footing (|| to X)

Arrangement of bottom reinforcement as per above design is shown below





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CHECK FOR ONE WAY SHEAR :

One way shear at critical section L1- L1

Distance of critical sec. from edge of footing = **0.641** m

Shear force Vs =pe max x 0.641 x 1m width of footing = **97.622** KN

Shear stress τ_v = Vs/bd = **0.265** N/mm^2

τ_c = **0.28** N/mm^2

tv < tc hence O.K.

One way shear at critical section L2- L2

Distance of critical sec. from edge of footing = **0.531** m

Shear force Vs =pe max x 0.531 x 1m width of footing = **80.869** KN

Shear stress τ_v = Vs/bd = **0.219** N/mm^2

τ_c = **0.26** N/mm^2

tv < tc hence O.K.

CHECK FOR TWO WAY SHEAR

Ref. cl 34.2.4 and cl.31.6.3 of IS 456 : 2000

Allowable shear stress $\tau_{v\,allowable}$ = $k_s \tau_c$

k_s = (0.5 + bc) = **1.5** >1

Hence, k_s= **1**

$\tau_c = 0.25 (fck)^{0.5}$ = **1.25** N/mm^2

$\tau_{v\,allowable} = k_s \times \tau_c$ = **1.25** N/mm^2

Shear force Vs = 152.296 (2.25 x 2.25 - 0.599 x 0.819) = **696.29** KN

Length of critical section = 2 x (599 + 819) = **2836** mm

Area of the critical section (length of critical sec x eff. d) = **1046484** mm^2

Hence shear stress τ_v = **0.665** N/mm^2

tv < allowable hence O.K.

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Design for area of steel and shear for singly reinforced beam by limit state design method

GRADE BEAM

Calculation of Ast req for beams

Ref IS 456-2000 Cl G-1.1b & G-1.1c For sections without compression reinforcement

f_y N/mm^2	f_{ck} N/mm^2	b mm	D mm	Cc mm	Cg of bar mm	d mm	$M_{u\ lim}$ kN.m	$p_{t\ lim}$ %
500	25	200	300	32	12.5	255.5	43.41	0.94

$M_{u\ support}$ kNm	Ast req. spt mm^2	p_t req.spt %	$M_{u\ span}$ kNm	Ast span mm^2	p_t req.span %	check for depth		
						d req mm	d prov mm	Result
15	143.04	0.28	20	194.91	0.38	150.19	255.5	okay

Reinforcement details provided at support and span of beam

Reinf. details at support					Reinf. details at span				
Nos.	dia mm	Ast support mm^2	pt support %	Result	Nos.	dia mm	Ast span mm^2	pt span %	Result
3	12	339.29	0.66	okay	3	12	339.29	0.66	okay
0	0				0	0			

Check for shear in beams (limit state design method)

Ref IS 456-2000 Cl 40.1, Cl 40.2.3, Table 19, Table 20 & Cl 40.2.1

f_{ck} N/mm^2	V_u kN	pt prov. %	τ_v Cl 40.1 N/mm^2	τ_c Table 19 N/mm^2	$\tau_{c\ max}$ Table 20 N/mm^2	Result
						Provide min shear reinf, Cl 26.5.1.6
25	20	0.66	0.39	0.55	3.1	tau_v <tau_cmax, Ok

Design for shear reinforcement (vertical stirrups)

Ref IS 456-2000 Cl 40.4a

V_u kN	τ_c b d kN	V_{us} req kN	V_{us}/d req kN/cm	f_y N/mm^2	assuming stirrup dia mm	no. of stirrup legs	stirrup sp assumed mm	V_{us}/d prov. kN/cm Cl 40.4 a	Result Cl 40.4a
20	28.11	-8.11	-0.32	415	8	2	200	1.815	Hence ok

Check for minimum and maximum spacing of stirrup

Min stirrup spacing mm Cl 26.5.1.6	Max stirrup spacing mm Cl 26.5.1.5	stirrup sp prov. mm	Result
546.64	191.625	200	Not ok

Side face reinforcement

Ref IS 456-2000 Cl 26.5.1.3

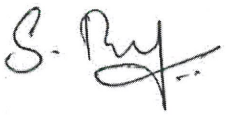
b mm	D of web mm	side face reinf. req. / face Cl 26.5.1.3	side face reinf. mm^2 /face prov.			spc b/w bars not to exceed Cl 26.5.1.3
			no. per face	dia of bar	Ast prov. mm^2	
200	300	not req	2	10	157.08	200 mm

Check for span to depth ratio

Ref IS 456-2000 Cl 23.2.1

Type of beam	f_y N/mm^2	span mm	d mm	$p_{t\ req.}$ %	$p_{t\ prov.}$ %	p_c %	MFt	MFc
Cont.Beam	500	7000	255.5	0.38	0.66	0	1.538	1

l/d prov	l/d Cl 23.2.1	Result Cl 23.2.1
27.40	39.99	Okay



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Design for area of steel and shear for singly reinforced beam by limit state design method

Calculation of Ast req for beams

Ref IS 456-2000 Cl G-1.1b & G-1.1c For sections without compression reinforcement

f_y N/mm^2	f_{ck} N/mm^2	b mm	D mm	Cc mm	Cg of bar mm	d mm	$M_{u\,lim}$ kN.m	$p_{t\,lim}$ %
500	25	230	450	35	12.5	402.5	123.89	0.94

$M_{u\,support}$ kNm	Ast req. spt mm^2	p_t req.spt %	$M_{u\,span}$ kNm	Ast span mm^2	p_t req.span %	check for depth		
						d req mm	d prov mm	Result
100	667.76	0.72	100	667.76	0.72	361.61	402.5	okay

Reinforcement details provided at support and span of beam

Reinf. details at support					Reinf. details at span				
Nos.	dia mm	Ast support mm^2	pt support %	Result	Nos.	dia mm	Ast span mm^2	pt span %	Result
2	16	804.25	0.87	okay	2	16	804.25	0.87	okay
2	16				2	16			

Check for shear in beams (limit state design method)

Ref IS 456-2000 Cl 40.1, Cl 40.2.3, Table 19, Table 20 & Cl 40.2.1

f_{ck} N/mm^2	V_u kN	pt prov. %	τ_v Cl 40.1 N/mm^2	τ_c Table 19 N/mm^2	$\tau_{c\,max}$ Table 20 N/mm^2	Result
						tau_v > tau_c,design for shear
25	65	0.87	0.70	0.61	3.1	tau_v <tau_cmax, Ok

Design for shear reinforcement (vertical stirrups)

Ref IS 456-2000 Cl 40.4a

V_u kN	τ_c b d kN	V_{us} req kN	V_{us}/d req kN/cm	f_y N/mm^2	assuming stirrup dia mm	no. of stirrup legs	stirrup sp assumed mm	V_{us}/d prov. kN/cm Cl 40.4 a	Result Cl 40.4a
65	56.47	8.53	0.21	415	8	2	200	1.815	Hence ok

Check for minimum and maximum spacing of stirrup

Min stirrup spacing mm Cl 26.5.1.6	Max stirrup spacing mm Cl 26.5.1.5	stirrup sp prov. mm	Result
475.34	300	200	Hence ok

Side face reinforcement

Ref IS 456-2000 Cl 26.5.1.3

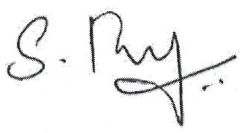
b mm	D of web mm	side face reinf. req. / face Cl 26.5.1.3	side face reinf. mm^2 /face prov.			spc b/w bars not to exceed Cl 26.5.1.3
			no. per face	dia of bar	Ast prov. mm^2	
230	450	not req	2	10	157.08	230 mm

Check for span to depth ratio

Ref IS 456-2000 Cl 23.2.1

Type of beam	f_y N/mm^2	span mm	d mm	$p_{t\,req.}$ %	$p_{t\,prov.}$ %	p_c %	MFt	MFc
Cont.Beam	500	6620	402.5	0.72	0.87	0	1.039	1

l/d prov	l/d Cl 23.2.1	Result Cl 23.2.1
16.45	27.01	Okay



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Design for area of steel and shear for two way slab by limit state design method

Slab Geometry

Lx m	Ly m	Ly/Lx	Result
4.22	4.7	1.114	<2, Hence two way slab

Grade of concrete, steel, & overall depth of slab

f_y N/mm^2	f_{ck} N/mm^2	b mm	D mm
500	25	1000	150

Lx-shorter span					
Cc bot mm	Cg of bot bar mm	d bot mm	Cc top mm	Cg of top bar mm	d top mm
25	5	120	25	5	120

Ly-longer span					
Cc bot mm	Cg of bot bar mm	d bot mm	Cc top mm	Cg of top bar mm	d top mm
25	15	110	25	15	110

Load calculation of the slab

Dead Load of the slab DL kN/m^2	Floor finish of the slab FF kN/m^2	Live load of the slab LL kN/m^2	Misc. load of the slab ML kN/m^2	Total unfactored load of the slab TL kN/m^2	Partial safety factor γ_f Table 18 IS 456-2000	Design load of the slab w kN/m^2
3.75	1	2	0	6.75	1.5	10.125

Moment & Shear calculation

Moment calculation for '1m' strip of the slab spanning Lx

w kN/m^2	Lx m	w Lx^2 kNm	- $M_{ux\ cont.\ edge}$ 'kNm'		+ $M_{ux\ mid\text{-}span}$ 'kNm'		V_u 'kN' Table 13 IS 456	
			- α_x	- α_x w L_x^2	+ α_x	+ α_x w L_x^2	Coef-shear	C w Lx
10.125	4.22	180.31	0.085	15.33	0.063	11.36	0.400	17.09

Calculation of Ast req for slab spanning Lx

Ref IS 456-2000 Cl G-1.1b & G-1.1c

- $M_{ux\ cont.}$ kNm	Ast req.cont. mm^2	pt req.cont. %	+ $M_{ux\ span}$ kNm	Ast span mm^2	pt req.span %
15.32635	309.75	0.26	11.36	226.26	0.19

Reinforcement details provided at support and span of slab spanning Lx

Reinf. details at support				Result	Reinf. details at span				Result
dia prov. mm	spacing mm	Ast cont. mm^2	pt cont. %		dia prov. mm	spacing mm	Ast span mm^2	pt span %	
10	150	523.60	0.44	okay	10	150	523.60	0.44	okay
0	250				0	150			

Moment calculation for '1m' strip of the slab spanning Ly

w kN/m^2	Lx m	w Lx^2 kNm	- $M_{uy\,cont.\ edge}$ 'kNm'		+ $M_{uy\ mid\text{-}span}$ 'kNm'	
			- α_y	- α_y w L_x^2	+ α_y	+ α_y w L_x^2
10.125	4.22	180.31	0.047	8.47	0.035	6.31

Calculation of Ast req for slab spanning Ly

Ref IS 456-2000 Cl G-1.1b & G-1.1c

- $M_{uy\ cont.}$ kNm	Ast req.cont. mm^2	pt req.cont. %	+ $M_{uy\ span}$ kNm	Ast min mm^2	pt req.span %
8.47	183.30	0.17	6.31	180.00	0.16

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Reinforcement details provided at support and span of slab spanning Ly

Reinf. details at support				Result	Reinf. details at span				Result
dia prov. mm	spacing mm	Ast cont. mm^2	pt cont. %		dia prov. mm	spacing mm	Ast span mm^2	pt span %	
10	150	523.60	0.48	okay	10	150	523.60	0.48	okay
0	250				0	250			

Check for shear in solid slabs for limit state design method
Ref IS 456-2000 Cl 40.1, Cl 40.2.3, Table 19, Table 20 & Cl 40.2.1.1

f_{ck} N/mm^2	V_u kN	b mm	D of slab mm	clear cover mm	c_g of bar mm	d mm
25	17.091	1000	150	25	5	120

p_t %	τ_v Cl 40.1 N/mm^2	$k\tau_c$ Cl 40.2.1.1 N/mm^2	$\tau_{c\,max}$ Table 20 N/mm^2	Result
				tau_v < k tau_c, Ok
0.44	0.14	0.60	3.1	tau_v <1/2 tau_c max,Ok

Check for span to depth ratio
Ref IS 456-2000 Cl 23.2.1

Type of beam	f_y N/mm^2	span mm	d mm	$p_{t\,req.}$ %	$p_{t\,prov.}$ %	p_c %	MFt	MFc
Cont.slab	500	4220	120	0.19	0.44	0	2.479	1

l/d prov	l/d Cl 23.2.1	Result Cl 23.2.1
35.17	64.45	Okay

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