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Report on
The Geotechnical Investigation for the proposed Construction of Residential Apartment Building in S.F. No: 121/3B, T.S. No: 2, Block No: 33, Ward No: D (49) at Koundampalayam, Coimbatore North, Coimbatore Corporation.

CLIENT:

M/s. Dhiya Foundation Private Limited,
Coimbatore.

Project Code: 102411
14 October 2024

GEOCARE ENGINEERING CONSULTANCY
(An ISO 9001:2015 Certified Company)
#17/25B, Annur Road,
Karumathampatti, Coimbatore - 641 659.
Phone: 0422 359 2569
Mobile: 9597262815
email: geocareconsultancy@gmail.com

GEOTECHNICAL INVESTIGATION BY MACHINE BORING METHOD

Client Name	M/s. Dhiya Foundation Private Limited,
Client Address	Coimbatore.
Project	Proposed construction of Residential Apartment Building,**Stilt+5 Floors (25Dwellings)**
Site Address	**Ward No:D, Block No:33, T.S.No:2, (S.F.No:121/3B pt), Koundampalayam Village, Coimbatore North Taluk, Coimbatore Corporation**
Date of field test	09th to 10th October 2024.

Field test conducted by	B. Naveenkumar, B.E
Report Prepared by	R. Suresh, B.E
Report Verified by	R. Sathishkumar, M.E (Geotech) / S. Prabhavathi, M.E (Geotech)
Report Approved by	R. Sathishkumar, M.E (Geotech)

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GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENT BUILDING IN S.F. NO: 121/3B, T.S. NO: 2, BLOCK NO: 33, WARD NO: D (49) AT KOUNDAMPALAYAM, COIMBATORE NORTH, COIMBATORE CORPORATION.

1. Introduction

The Management, M/s. Dhiya Foundation Private Limited, proposes to construct a Residential Apartment Building in S.F. No: 121/3B, T.S. No: 2, Block No: 33, Ward No: D (49) at Koundampalayam, Coimbatore North, Coimbatore Corporation. In order to enable an adequate and economic design, the Geotechnical Investigation was assigned to M/s. GEOCARE Engineering Consultancy, Coimbatore in view to furnish the detailed technical information of the nature of subsoil strata for detail analysis and foundation designs.

2. Objectives and Scope of Work

2.1. Objectives

The object of the Geotechnical Investigation is to evaluate the following:

- To ascertain the subsoil strata.
- To record the ground water level if any.
- To determine the physical, and engineering properties of soil/rock samples.
- To evaluate the allowable bearing capacity of soil/rock.
- To recommend improvements to the weak soil strata, if any

2.2. Scope of Work

The scope of work includes the following field (In-situ) and laboratory tests.

2.2.1. Field (In-Situ) Investigations

i) Boring 150mm dia/Nx size boreholes in all kinds of strata using rotary drilling by wash boring method as per IS –1892 –1979.

ii) Conducting field tests such as Standard Penetration Tests as per IS 2131- 1981 at various depths in the boreholes.

iii) Collection of undisturbed and disturbed soil samples at specified depths for determination of cohesion 'c' friction factor 'ϕ' and others index & engineering properties as per Indian Standards.

2.2.2. Laboratory Testing

The scope of laboratory testing is as follows:

i) Determination of Natural Moisture Content and Density as per IS 2720 - part 2.

ii) Grain size analysis as per IS 2720 - part 4.

iii) Determination of shear strength parameters as per IS 2720 – part 11 / 13.

2.3. Report

This report comprises of soil profile, recommendations regarding allowable bearing capacity, improvement in bearing capacity of soil.

3. Project Details

3.1. The Site

The site for the proposed construction of Residential Apartment Building is located at S.F. No: 121/3B, T.S. No: 2, Block No: 33, Ward No: D (49), Koundampalayam, Coimbatore North, Coimbatore Corporation.

3.2. Weather Condition

The weather was clear and sunny at the time of investigation.

3.3. Seismic Zone

The site for the proposed Residential Apartment Building at S.F. No: 121/3B, T.S. No: 2, Block No: 33, Ward No: D (49), Koundampalayam, Coimbatore North, Coimbatore Corporation which comes under seismic zone-III as per IS 1893 (Part-I)-2002.

3.4. Geology of Coimbatore District

0About 60 per cent of the district is covered by red soils, of which red calcareous soil is predominant. They occupy most parts of Palladam, Coimbatore. Medium to deep red calcareous soils are found mainly in Pollachi and Udumalpet taluks. Parts of Palladam, Avinashi and Udumalpet taluks are occupied by red non-calcareous soils. The highlands in Coimbatore, Palladam and Avinashi taluks are mostly occupied by the black soils, which are dark gray to greyish brown in color. The Alluvial soils are found in small patches along the Noyyal River mainly in the upper reaches.

* Reference CWC Coimbatore District

4. Schedule of Investigation

4.1. Field Investigations

To study the subsoil strata at the proposed location 02 nos. of 150mm dia/Nx size boreholes were bored by wash boring method using rotary calyx boring machine.

4.2. Standard Penetration Tests

Standard Penetration Test (SPT), which is conducted to determine penetration resistance was conducted in the boreholes using the procedure as described in IS: 2131. In this method, split spoon sampler (50.8mm OD and 35mm ID) is driven by dropping 63.5kg weight with a free fall of 75cm. The length of the sampler is 60cm. The sampler is first driven through 15cm as a "seating drive ". It is further driven through 30cm. The number of blows required for drilling the sampler for next 30 cm beyond seating drive is termed as "penetration resistance," N". Respective "N" values are presented in bore log sheets.

The SPT tests were conducted at regular intervals of 1.5m/change in strata in the boreholes.

The disturbed soil samples were collected in polythene bags and labeled.

The samples thus recovered were transported to the laboratory for testing purposes.

The depth at which SPT tests were conducted, disturbed sample taken at each location and borehole terminated is as follows

Table-1 Depth of SPT and UDS Samples

SL. No.	Borehole No.	Depth (m)		
		SPT Samples	UDS	Terminated
1	BH1	1.50, 3.0, 4.50, 6.0	–	10.0
2	BH2	1.50, 3.0, 4.50, 6.0	–	10.0

4.3. Standing Water Level

Standing water level (SWL) is observed at a depth of about 2.50 – 3.0m in the boreholes during field investigation. Field investigation has been carried out on 09th and 10th October 2024.

5. Results and Discussion

Sub Soil Stratification

The subsoil strata at BH1 & BH2 were more or less similar and consists of Silty Sand up to a depth of about 1.0 – 3.80m below existing ground level (EGL), followed by Sand with Gravel up to a depth of about 5.50 – 6.0m and underlain by Dense Gravel at explored depth.

Stratum I : Silty Sand

Stratum II : Sand with Gravel

Stratum III : Dense Gravel

BH No	Strata depth in m		
	Stratum I	Stratum II	Stratum III
BH1	0.0 – 1.0	1.0 – 5.50	5.50 – 10.0^{E} and above
BH2	0.0 – 3.80	3.80 - 6.0	6.0 – 10.0^{E} and above

E- Explored depth

Design Parameters

The parameters required for the design of the foundation system for the proposed building are:

a) Type of foundation to be adopted

b) Depth at which the foundations have to be laid

c) Allowable bearing pressure on the soil at the foundation level



Type of Foundation

The type of foundation for any structure depends upon the subsoil conditions, type of structure, as well as the loading intensity on the foundations.

The subsoil encountered at average founding levels is loose strata. Hence, Pile Foundation can be adopted for the proposed building. The Pile Foundations may be group pile with pile cap depending upon the column loads, their spacing and configuration. The Shallow Foundations (Isolated / Combined Footing) may also be considered depending upon the column loads, their spacing and configuration.

Depth of Foundations

The minimum depth of foundations depends upon the following factors:

a) Top loose zone, if any

b) Adequate depth of soil above founding level, to ensure mobilization of full safe bearing capacity

c) Adequate depth of soil strata below founding level of requisite strength to mobilize the safe bearing capacity.

Allowable Bearing Pressure

The allowable bearing pressure on foundations of any structure is evaluated based on the following criteria:

a) Shear failure criterion using average soil data

b) Settlement criterion

The ground water table was not encountered at a depth of about 2.50 – 3.0m in the boreholes up to the investigated depth.

The Reduction Factor for Water Table R'w=0.5 has been applied. For calculation of overburden pressure, Bulk Density has been considered.

For calculations based on settlement criteria, the allowable total settlement has been taken as 25mm.The soil parameters considered for evaluation of net SBC/ allowable bearing pressure based on the test results on undisturbed soil samples collected at the site are as given below.

Shear failure Criteria, c-ϕ soil

Design Parameters

Cohesion c = 0.01 kg/cm^2

Angle of internal friction $\phi = 30^0$

Density of soil γ_b = 1.65 g/cm^3

Factor of Safety = 3.0



Consolidation settlement criteria

Method: IS: 8009 (Part I approach), consolidation settlement criteria and normally consolidated soils.

Settlement has been computed for Minimum SBC value from shear approach. It is seen that settlements are within 25mm.

On the basis of the above analysis, the Net Allowable Bearing Pressures for 1.5m width of isolated footings, has been evaluated and is given below:

The strata available at the founding level will be generally Sand with Gravel.

The Load Carrying Capacity of Piles and Safe bearing capacity so determined are tabulated below.

5.1. Safe Load Carrying Capacity of Piles

Recommendations are provided at depth 8.0m below NGL.

Table - 3 Recommended Safe Load Carrying Capacities for Piles

Sl.No	Bore hole No	Length of Pile below E.G.L(m)	Dia of Pile(mm)	Safe Load carrying Capacity(kN)
1	BH1 to BH2	**8.0**	230	125
2			**300**	**190**
3			400	305
4			450	380

5.2. Net Safe Bearing Capacity/Allowable Bearing Pressures

Recommendations are provided at depth of 2.0m, 3.0m, 4.0m, 4.50m & 5.0m below NGL considering shear failure and settlement.

Table-2 Allowable Safe Bearing Capacity

Borehole No.	Depth below EGL (m)	Type of Strata	SBC (kN/m²)
			B=1.5m
BH1 & BH2	2.0	**Sand with Gravel**	175
	3.0		250
	4.0		300
	4.50		**330**
	5.0		365



Site location is shown in Appendix-I. The schematic plan showing the location of boreholes is shown in Appendix-II - Fig. 1.The index & engineering properties of soil samples at various depths in the boreholes are indicated in Appendix – III - Table 3. The particle size distribution curves for the soil samples are shown in Fig. 2. Bore logs are shown in Appendix – IV. Photographs showing field investigation are shown in Appendix – V.

6. Conclusion & Recommendations

- The subsoil consists of Silty Sand followed by Sand with Gravel and underlain by Dense Gravel at explored depth.
- Standing water level is observed at a depth of about 2.50 – 3.0m in the boreholes during field investigation.
- Based on the subsoil profile open foundation of **isolated / combined footing** with tie beam is recommended.
- The recommended depth of pile foundation is **8.0m** below EGL with a **Safe load Carrying Capacity** of **190kN** for a pile dia of **300mm**.
- However, the diameter of pile, depth of pile, number of piles & grouping of piles shall be the sole discretion of the structural engineer.
- During execution, it's ensured that, the pile **must be embedded minimum 1.0 m** in dense strata.
- In case of isolated footing, the recommended depth of foundation is **4.50m** below EGL with an SBC of **330kN/m²** for a footing width of **1.50m.**
- However, depth of footing is the sole discretion of the in-charge engineer based on structural design depending upon the structure and respective SBC shall be considered since depth variation of strata is there.
- Proper strutting / shuttering shall be adopted during excavation to avoid caving the soil in.
- Over excavation and loosening of strata at final founding level shall be avoided & if occurs, the same shall be replaced by lean concrete.
- Prior to laying PCC, all loose soil in the foundation shall be removed and the base should be thoroughly compacted followed by a layer of 300mm thick sand-boulder filling with proper compaction is recommended. Soil replacement and proper compaction below the founding depth shall be done upto 1.0B where B = width of the foundation.



- It is recommended that the back filling in the foundation should be carried out with proper compaction.
- Recommendations given are based on the assumption that same strata or harder strata extends below the explored depth.
- Interpretations are provided based on factual field &laboratory test results & shall be applicable for investigated locations. There is a possibility that strata variations could occur. If any variations indicate significant deviation from the findings of this report, the same shall be brought to our notice.

For

GEOCARE Engineering Consultancy

Authorized Signatory

S.PRABHAVATHI, M.E (Geotech) M.I.G.S
Registered Geotechnical Engineer
Reg No: Coimbatore LPA / GTE /19/04/002,
No: 17/25-B, Annur Road, Karumathampatti,
Coimbatore - 641 659.
Ph: 0422-4399772, Mob: +91 9600558700

APPENDIX – I

BOREHOLE LOCATION PLAN



Figure 1 - Borehole Location (Approx) of Site

Note: Dimensions are not to scale

APPENDIX – II
SITE LOCATION

Google Maps 11°02'24.3"N 76°55'58.4"E



Imagery ©2024 Airbus, CNES / Airbus, Maxar Technologies, Map data ©2024 20 m

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APPENDIX – III

LABORATORY TEST RESULTS

Table 3 – Index and Engineering properties of soil samples:

BORE HOLE	SAMPLE TYPE	DEPTH (m)	NMC (%)	N-VALUE	GRAIN SIZE DISTRIBUTION (%)					SHEAR STRENGTH PARAMETER		IS CLASSIFICATION
					GRAVEL	SAND			SILT + CLAY	c (kg/cm²)	ø (Degrees)	
						COARSE	MEDIUM	FINE				
BH1	SS	1.5	6.2	>50	21.7	3.5	31.0	26.8	17.1	0.01	30	SG
BH1	SS	3.0	10.2	>50	27.9	3.9	28.6	30.1	9.5			SG
BH1	SS	4.5	5.3	>50	20.3	15.3	35.8	12.4	16.2			SG
BH1	SS	6.0	7.1	>50	86.3	2.9	1.3	1.3	8.3			GS
BH2	SS	1.5	11.1	40	1.7	1.8	25.8	22.7	48.1	0.01	30	SM
BH2	SS	3.0	8.1	>50	5.0	1.3	5.0	41.5	47.2			SM
BH2	SS	4.5	11.1	>50	15.0	7.3	15.8	23.2	38.7			Sand
BH2	SS	6.0	5.3	>50	10.2	7.4	15.1	22.4	44.9			Sand

SS – Split Spoon Sample, ST – Shelby Tube Sample, GS – Sandy Gravel; SM – Silty Sand; SG – Sand with Gravel

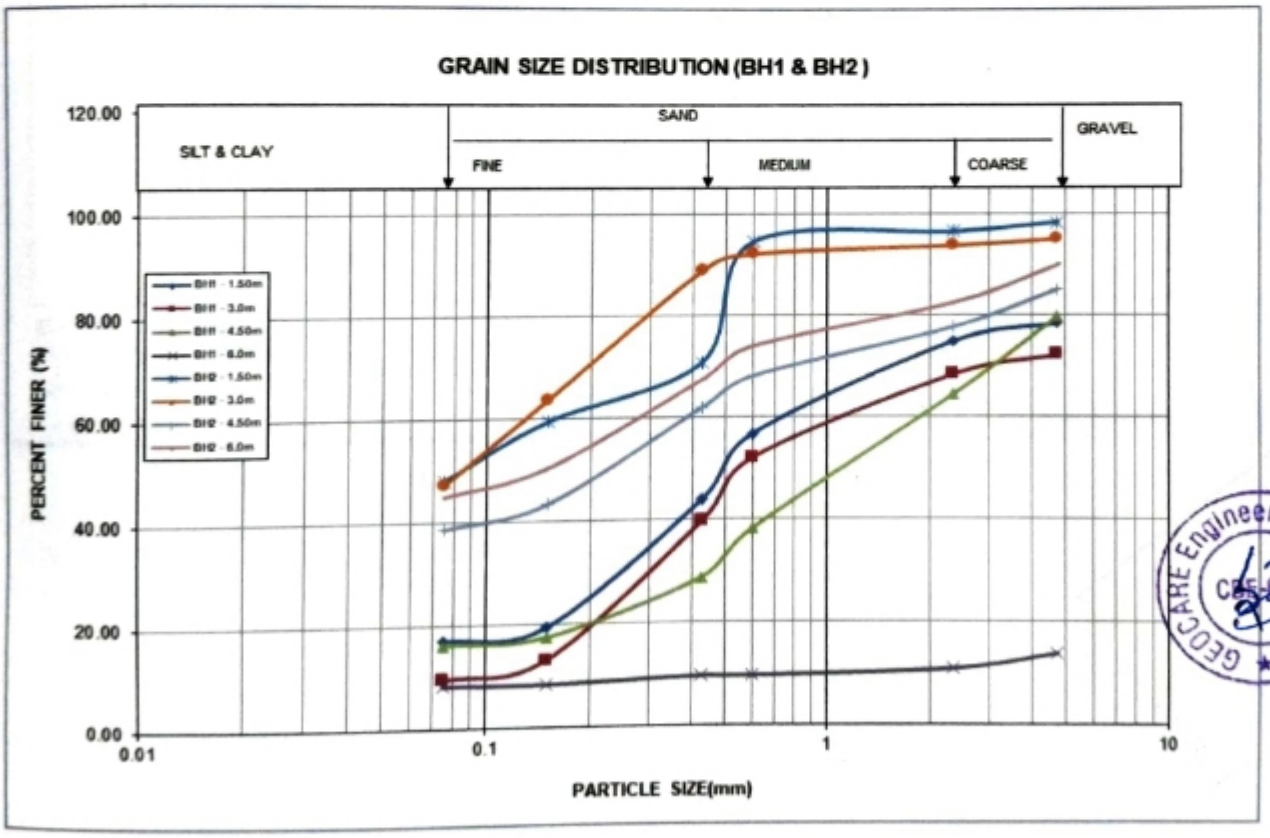


Fig. 2 - Grain Size Distribution Curve

APPENDIX – IV

BORE LOGS

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BORE LOG

PROJECT : Geotechnical investigation of Residential Apartment building

CLIENT : M/s. Dhiya Foundation Private Limited,
BORE HOLE NO. : BH-01

LOCATION : Koundampalayam, Coimbatore North, Coimbatore Corporation.

TYPE OF BORING : Machine Boring
BORE HOLE DIA. : 100mm
GWT FROM G.L. 2.50 - 3.0m
CASING DEPTH :
DATE OF COMMENCEMENT : 09-10-2024
DATE OF COMPLETION : 09-10-2024

SOIL DESCRIPTION	DEPTH BELOW GL (m)	STANDARD PENETRATION TEST N-VALUE	10	20	30	40	>50	SAMPLE TYPE	ROCK DRILLING C R (%)	ROCK DRILLING RQD (%)	REMARKS
Silty Sand	1.00	>50 (12,26,31)						SS			
Sand with Gravel	2.00	>50 (15,23,28)						SS			
	3.00										
	4.00	>50 (17,32,36)						SS			
	5.00										
	6.00	>100 (50+R)						SS			
Dense Gravel	7.00										
	8.00										
	9.00										
	10.00										

SS = SPLIT SPOON SAMPLE
ST = SHELBY TUBE SAMPLE
WS = WASH SAMPLE
R = REFUSAL

BH - 01 Terminated at 10.0m depth

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BORE LOG

PROJECT : Geotechnical investigation of Residential Apartment building

CLIENT : M/s. Dhiya Foundation Private Limited,
BORE HOLE NO. : BH-02

LOCATION : Koundampalayam, Coimbatore North, Coimbatore Corporation.

TYPE OF BORING : Machine Boring
BORE HOLE DIA. : 100mm
GWT FROM G.L. 2.50 - 3.0m
CASING DEPTH :
DATE OF COMMENCEMENT : 10-10-2024
DATE OF COMPLETION : 10-10-2024

SOIL DESCRIPTION	DEPTH BELOW GL (m)	STANDARD PENETRATION TEST N-VALUE	10	20	30	40	>50	SAMPLE TYPE	ROCK DRILLING CR (%)	ROCK DRILLING RQD (%)	REMARKS
Silty Sand	1.00	40 (8,17,23)						SS			
	2.00	>50 (14,23,27)						SS			
	3.00										
	4.00	>100 (50+R)						SS			
Sand with Gravel	5.00										
	6.00	>100 (50+R)						SS			
Dense Gravel	7.00										
	8.00										
	9.00										
	10.00										

SS = SPLIT SPOON SAMPLE
ST = SHELBY TUBE SAMPLE
WS = WASH SAMPLE
R = REFUSAL

BH - 02 Terminated at 10.0m depth

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APPENDIX – V

PHOTOGRAPHS SHOWING FIELD INVESTIGATION



Bore Hole - 01



Bore Hole –02



Sample Collection BH - 02

REFERENCES

LIST OF IS CODES

Field Investigation

1. IS: 1892 - 1979: Code of practice for sub surface investigations for foundations (First revision)
2. IS: 2131 - 1981: Method of Standard Penetration Tests for soils (First revision)
3. IS: 2132 - 1986: Code of practice for thin walled tube sampling of soils (Second revision)
4. Field Permeability test in Boreholes as per IS 5529 part – 1

Laboratory Tests

5. IS: 2720 - 1983 (Part 1): Methods of test for soils: Preparation of dry soil samples for various tests (Second revision)
6. IS: 2720 - 1980 (Part 2): Method of test for soils: Determination of water content (Second revision) Amendment 1
7. IS: 2720 - 1985 (Part 4): Method of test for soils: Grain size analysis (Second revision)
8. IS: 2720-1981 Part 13: Method of tests for soils: Determination of shear strength parameters using Direct Shear apparatus.
9. Determination of Specific gravity as per IS 2720 part – 3.
10. Determination of Free swell Index as per IS 2720 part - 40.

Foundation Construction

11. IS: 1080 - 1986: Code of practice for design and construction of shallow foundations on soils (other than raft, ring and shell) (Second revision)
12. IS: 1904 - 1986: Code of practice for design and construction of foundation in soils: General requirements (Third revision)
13. IS: 6403 - 1981: Code of practice for determination of bearing capacity of shallow foundations: First revision (Amendment 1)
14. IS: 8009 - 1976 (Part 1): Code of practice for calculation of settlements of foundations: Shallow foundations subject to symmetrical static vertical loads (Amendment 2)





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Home Vastu Shastra, Plot Vastu Shastra, Property Management

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hello@verified.realestate
+91 9996 12 9996
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