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STRUCTURAL DESIGN BRIEF

THE PROPOSED RESIDENTIAL VILLA GROUP DEVELOPMENT

BUILDINGS AT S.F.NO:527/1B2, 527/1C2,527/1D2,541/2A2A,

541/2A3A & 541/3, CHINNAVEDAMPATTI VILLAGE, NORTH ZONE

& TALUK, COIMBATORE CORPORATION.

" SREEVATSA URBAN VILLAGE "

SANKAR & ASSOCIATES, ARCHITECTS & PLANNERS,
COIMBATORE-641 009

CLIENT:	M/s. SREEVATSA URBAN VILLAGE	DESIGN:	G + 2 Floors	ARCHITECT & PLANNERS **SANKAR & ASSOCIATES** 27 SENGUPTA STREET, RAMNAGAR, COIMBATORE.
STRUCTURE NAME :	RESIDENTIAL BUILDING	TITLE: REPORT ON STRUCTURAL DESIGN.		
PLACE:	COIMBATORE			

Name of Project:		Name of the Structure:
RESIDENTIAL VILLA		RESIDENTIAL GROUP DEVELOPMENT
Report by:	**Checked by:**	**Approved by:**
N.GOWDAMAN	S.MURUGESH	V.P.PONNUSWAMI

DESCRIPTION OF THE PROJECT:

The Proposed structure belongs to M/s Sreevatsa Urban Village. Purpose of this structure is to house "Residential Villa". The structure is designed for Ground floor + Two Upper floors. The structural design was carried out as per IS 456-2000. The dead load and live load are calculated based on IS 875: Part I & II. The structure was analyzed using the software STAAD-PRO V8i.

STRUCTURAL DESIGN CALCULATION

I DESIGN PARAMETERS:

(1) This structure is designed for Ground floor + Two Upper floors.

(2) SBC Adopted as 390 KN/sqm. (As per soil test Report provided)

(3) Concrete grade adopted:

For Footing & Columns : M25

For Beams & Slabs : M25

Grade of steel adopted : Fe 500.

(4) Dimension of the building : 35'-8" x 62'-8"

(5) Floor height is 3.35m. (Typical)

II LOAD DETAILS:

DEAD LOAD:

1. Self weight of slab (Assumed as 130mm Tk.) - 3.25 KN/m^2.
2. Self weight of slab (Assumed as 150mm Tk.) - 3.750 KN/m^2.

SUPER IMPOSED DEAD LOAD:

1. Floor finish - 1.5 KN/m^2.
2. Unit wt. of brick * Tk of wall * Ht of wall
   (19 x 0.23 x (0.80 x (3.35-0.38)) - 10.00 KN/m.

SUPER IMPOSED LIVE LOAD:

1. All Rooms and Kitchen - 2 KN/m^2
2. Terrace Floor - 1.5 KN/m^2

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III LOAD COMBINATION

Loads considered for calculation of horizontal seismic load = DL + 25 % LL

Load case:

1. Seismic Load in 'X' direction (EQX)
2. Seismic load in 'Y' direction (EQY)
3. Dead Load (DL)
4. Live Load (LL)

Load combination:

1. LOAD COMB (1.0 DL + 1.0 LL)
2. LOAD COMB (1.5 DL + 1.5 LL)
3. LOAD COMB (1.2(DL + LL + EQX))
4. LOAD COMB (1.2(DL + LL + EQY))
5. LOAD COMB (1.2(DL + LL - EQX))
6. LOAD COMB (1.2(DL + LL - EQY))
7. LOAD COMB (1.5(DL + EQX))
8. LOAD COMB (1.5(DL + EQY))
9. LOAD COMB (1.5(DL - EQX))
10. LOAD COMB (1.5(DL - EQY))
11. LOAD COMB (0.9DL + 1.5 EQX)
12. LOAD COMB (0.9DL + 1.5 EQY)
13. LOAD COMB (0.9DL - 1.5 EQX)
14. LOAD COMB (0.9DL - 1.5 EQY)

Location and thickness of walls as depicted in the architectural drawings were considered for loads at appropriate locations in the analysis.

IV ANALYSIS:

The analysis is performed using software STAAD Pro V8i.

V DESIGN OF STRUCTURE:

From analysis the columns and beams were grouped as shown in centerline drawing depending on the forces. The footing, column and beams were designed for the maximum forces in-group as per IS456-2000.

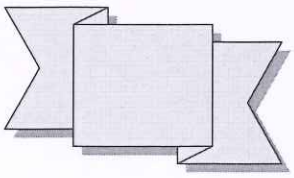
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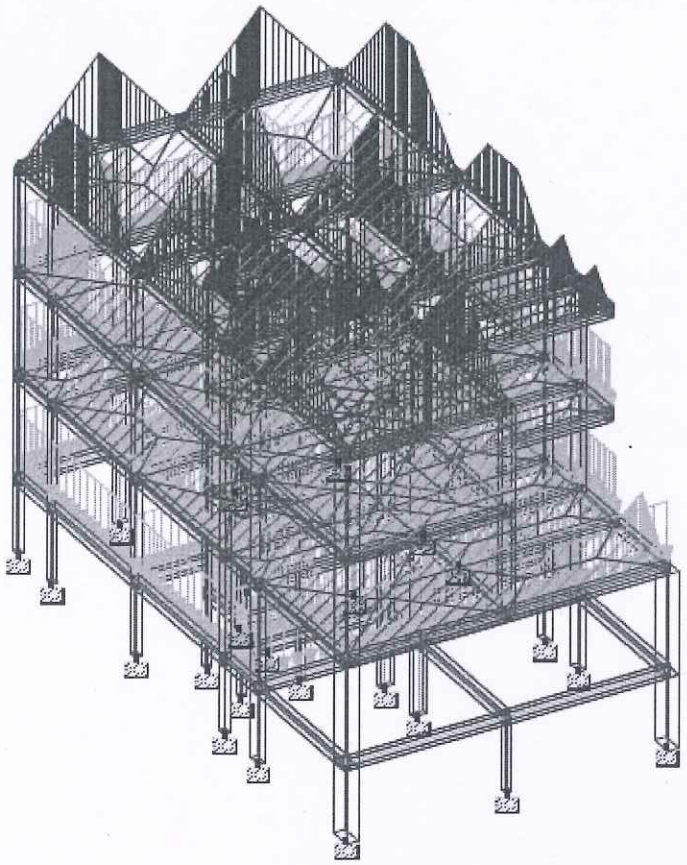
VI STRUCTURAL ARRANGEMENT:

Column arrangements, Beam & Slab layout were presented, drawing enclosed here for reference in Annexure (I) , Typical Design Calculation for Column, footing , Beam and Slab in Annexure (II)

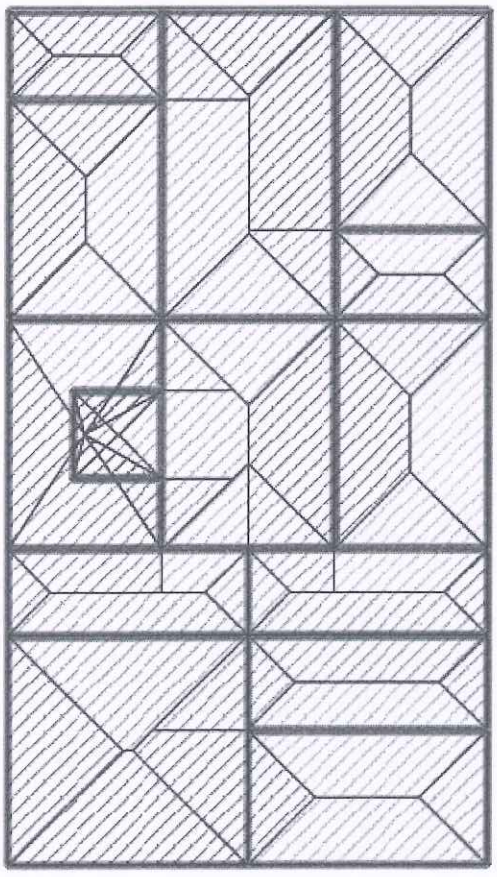


ANNEXURE – I





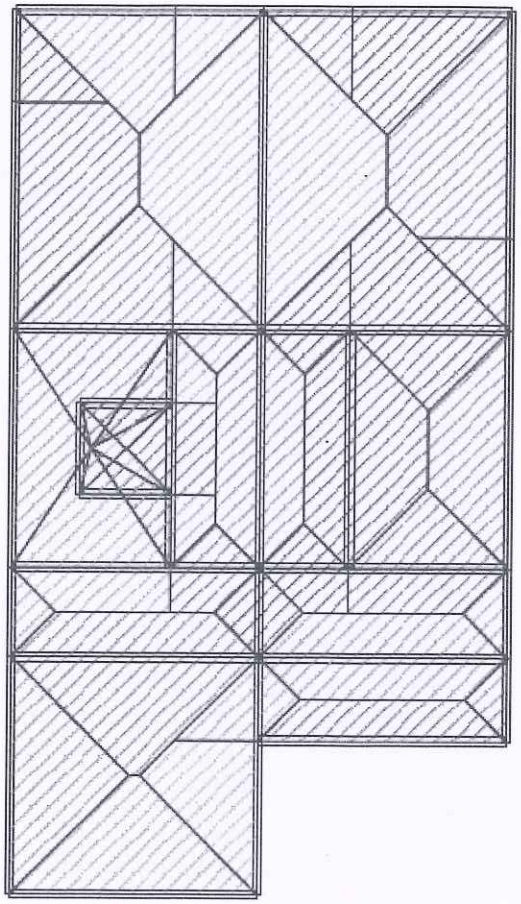
ISOMETRIC VIEW FROM STAAD MODEL



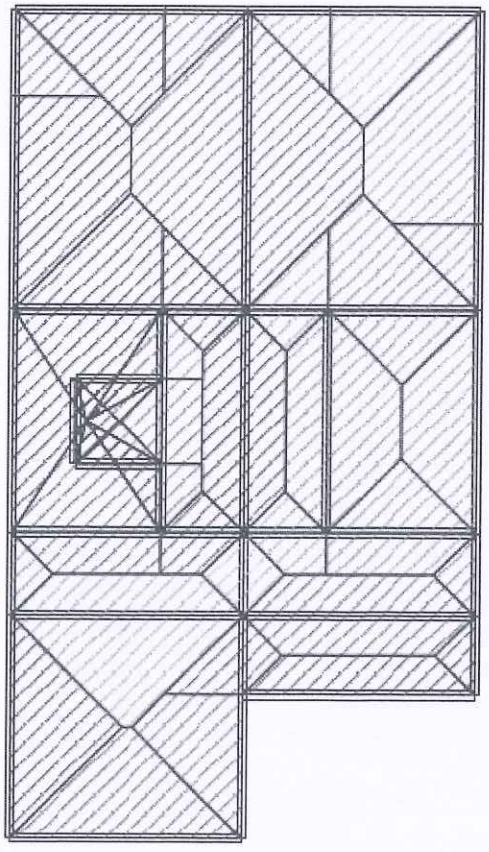
GROUND FLOOR ROOF LAYOUT

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FIRST FLOOR ROOF LAYOUT

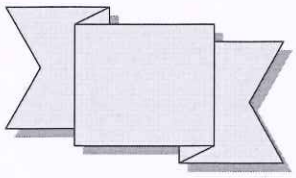


SECOND FLOOR ROOF LAYOUT

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ANNEXURE – II



TYPICAL COLUMN DESIGN

Input Data

Breadth	b	=	200.00	mm
Depth	D	=	460.00	mm
Clear cover	cc	=	40.00	mm
Effective cover	d'	=	58.00	mm
Effective Breadth	b'	=	142.00	mm
Effective Depth	D'	=	402.00	mm
Length of column	L	=	3.35	m
Depth of Beam/Slab framing into coln (Major)	Bd	=	0.38	m
Depth of Beam/Slab Framing into coln (Minor)	Bd	=	0.38	m
Clear length in major axis	Lc or Lo	=	2.97	m
Clear height in minor axis	Lc or Lo	=	2.97	m
Effective length factor	β	=	0.80	
Effective length of column	Leff (x-dir)	=	2.38	m
Effective length of column	Leff (y-dir)	=	2.38	m
Axial Load (Factored)	N	=	1190.00	KN
Moment about major axis (Factored)	Mx	=	5.00	KN-m
Moment about minor axis (Factored)	My	=	10.00	KN-m
Characteristic strength of concrete	fck	=	25.00	N/mm^2
Characteristic strength of steel	fy	=	500	N/mm^2

Design Calculations:

Step1: Check for Slenderness ratio : Leff/D = 5.17 <12

Check for Slenderness ratio : Leff/b = 11.88 <12

Design a: Short Column

eax = 0.00 mm

eay = 0.00 mm

Slenderness limits for column Lo < 60 b 2.97 m < 12.00 m

Step:2 Check for moment due to minimum eccentricity

Minimum Eccentricity (L/500 + d or b/30) or 20 (whichever is< ex = 20.00 mm

ey = 12.61 mm

A.) Moments due to Min eccentricity Mex = 23.80 KN-m

Mey = 15.00 KN-m

Step:3 Calculation of Additional Moments

Percentage of Reinforcement IN Staad pro v8i p = **2.30 %**

Reinforcement Equally Distributed on **Four Face**

Puz = 1828.50 KN

d'/d along x-dir = 0.15

d'/b along y-dir = 0.20

Along x-direction

k1 = 0.196

k2 = 0.256

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Along y-direction

	k1	=	0.184	
	k2	=	0.04	
Pb=(k1 + k2 p/Fck)Fck b d)	Pbx	=	504.97	KN
	Pby	=	431.66	KN
k = (Puz - Pu)/(Puz - Pb)	Kx	=	0.48	
	Ky	=	0.46	
B.) Additional Moments	Max	=	0.00	KN-m
	May	=	0.00	KN-m

Step:4 Total Moments for which the column is to be designed are

D.) Final Moment	**Mux**	**=**	**23.80**	KN-m
	Muy	**=**	**15.00**	KN-m

Step:5

Pu / Fck bd		=	0.52	
p/Fck		=	0.09	

*Refer CHART 49 From SP 16 : 1980 for red values

Pu / Fck bD= 0.52 p/Fck= 0.092 d'/D = 0.15

	Mux1 / Fck bD^2	=	0.09	
	Mux1	=	**95.22**	KN-m

*Refer CHART 50 From SP 16 : 1980 for red values

Pu / Fck 0.52 p/Fck= 0.092 d'/b = 0.20

	Muy1 / Fck bD^2	=	0.07	
	Muy1	=	**32.20**	KN-m
	Mux/Mux1	=	0.25	
	Muy/Muy1	=	0.47	
	Pu/Puz	=	0.65	
	αn	=	1.75	
	(Mux/Mux1)^αn + (Muy/Muy1)^αn	=	**0.35**	**<1**

Step:6 Longitudinal Reinforcemer Area of steel Required Asc = 2116.00 mm^2

Larger dia			**Smaller dia**		
Select Bar dia	20 mm		Select Bar dia	=	16 mm
No. of bars Prov	4		No. of bars Prov	=	6
Area Prov.	1256.64		Area Prov.	=	1206

	Area of Steel Provided Asc (pro)	=	2463.01	mm^2
Percentage of Steel Provided		=	2.68	%

Provide 4Y20+ 6Y16 as longitudinal reinforcement

Step:7 Lateral Ties

1. Pitch: (take whichever is lesser)

i. 16x smaller dia of main steel		=	250.00	mm
ii. 200 mm		=	200.00	mm
	Pi	=	200.00	mm

2. Diameter (whichever is greater)

i. 1/4 x largest dia of main steel		=	5.00	mm
ii. 10mm		=	8.00	mm
	Dia	=	8.00	mm

Therefore provide Y8 @200c/c as Lateral ties.

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PLACE:	COIMBATORE	STRUCTURAL DESIGN.	

ARCHITECT & PLANNERS
SANKAR & ASSOCIATES
27 SENGUPTA STREET,
RAMNAGAR,
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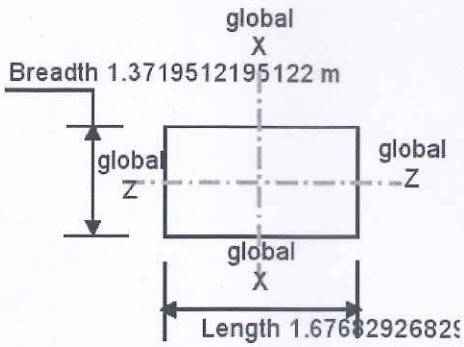
TYPICAL FOOTING DESIGN

COLUMN

Length (l, dim. || Z axis) =	460	mm
Breadth (b, dim. || X axis) =	200	mm

FOOTING

Foot length (L, dim. || Z axis) =	1.68	m
Foot Breadth (B, dim. || X axis) =	1.37	m
Thickness of footing (t) =	530	mm
Clear cover of footing =	50	mm
Main bar dia of footing =	12	mm
Effective depth of footing =	**474**	mm
Selfweight of the footing =	**30.48**	KN
Area of Footing(A) =	**2.30**	m^2
Sect mod of foot about Z axis (Zz) =	**0.53**	m^3
Sec mod of foot about X axis (Zx) =	**0.64**	m^3



Footing Dimensions

MATERIALS OF CONSTRUCTION

Grade of concrete f_{ck} =	25	N/mm^2
Grade of steel f_y =	500	N/mm^2

CHECK FOR GROSS BEARING PRESSURE

Safe **NET** bearing pressure =	390	KN/m^2
Safe gross bearing pr. =	**460.87**	KN/m^3
Unfactored load case number =	0	
Axial load from output (P1) =	1190.00	KN
Moment about Z axis (M_z) =	5.00	KN-m
Moment about X axis (M_x) =	10.00	KN-m
Depth of top of foot. from ground =	3.2	m
Unit wt of soil =	19	KN/m^3
Weight of soil retained above foot =	**134.28**	KN
P = (P1+soil+foot self wt) =	**1354.76**	KN
Maximum bearing pressure =	**613.95**	KN/m^2
Minimum bearing pressure =	**563.83**	KN/m^2

(net pr. + depth of foot * soil unit wt)

$$\frac{P}{A} \pm \frac{M_y}{Z_y} \pm \frac{M_x}{Z_x}$$

Footing is not safe against max bearing pressure.

DESIGN FORCES

Factored load comb. no.	5	
Axial load:(P_u) =	1785	**KN**
Moment about Z axis (M_{uz}) =	7.5	**KN-m**
Moment about X axis (M_{ux}) =	15	**KN-m**

Maximum effective soil pressure $p_{e\,max}$

(Pu/Area+ Muz/Zz + Mux/Zx) = **813.50** KN/m^2

Minimum effective soil pressure $p_{e\,min}$

(Pu/Area - Muz/Zz - Mux/Zx) = **738.32** KN/m^2

Design of footing is done using above maximum effective soil pressure

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CALCULATION FOR BOTTOM STEEL

Mu about X1 X1 = (pe max x length^2/2)= **150.57** KN-m per meter

Mulimit = **747.19** KN-m per meter

The section is singly reinforced

$$A_{st} = \frac{0.5 f_{ck}}{f_y}\left[1 - \sqrt{1 - \frac{4.6 M_u}{f_{ck} b d^2}}\right] bd$$

Hence, Ast = **754.619** mm^2

Min Ast = **568.800** mm^2 (0.12 % for slab, cl 26.5.2.1)

Spacing = **149.87** mm (considering max of above two calculated values of Ast)

pt provided = **0.16** %

Hence provide 12 mm dia bar @ 149 mm c/c parellel to length of footing (|| to Z)

Mu about N1 N1 = (pe max x length^2/2)= **139.66** KN-m per meter

Calc. Ast = **698.268** mm^2 The section is singly reinforced

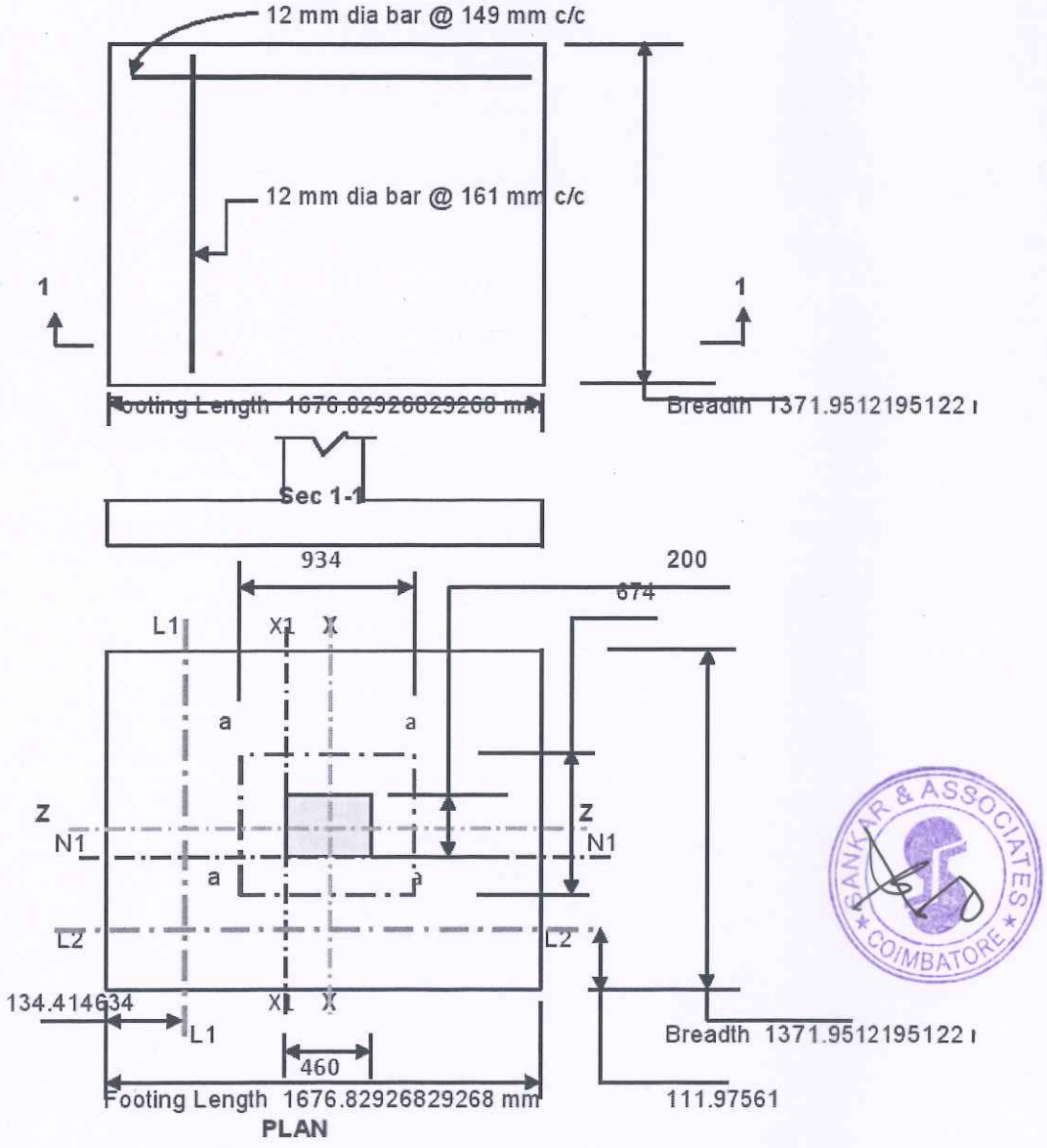
Min Ast = **568.8** mm^2 (0.12 % for slab, cl 26.5.2.1)

Spacing = **161.97** mm (considering max of above two calculated values of Ast)

pt provided = **0.14731401** %

Hence provide 12 mm dia bar @ 161 mm c/c parellel to breadth of footing (|| to X)

Arrangement of bottom reinforcement as per above design is shown below



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CHECK FOR ONE WAY SHEAR :

One way shear at critical section L1- L1

Distance of critical sec. from edge of footing = **0.13441463** m

Shear force Vs =pe max x 0.134414634146342 x 1m width of fc **109.346** KN

Shear stress τ_v = Vs/bd = **0.231** N/mm^2

τ_c = **0.30** N/mm^2

tv < tc hence O.K.

One way shear at critical section L2- L2

Distance of critical sec. from edge of footing = **0.11197561** m

Shear force Vs =pe max x 0.111975609756098 x 1m width of fc **91.092** KN

Shear stress τ_v = Vs/bd = **0.192** N/mm^2

τ_c = **0.29** N/mm^2

tv < tc hence O.K.

CHECK FOR TWO WAY SHEAR

Ref. cl 34.2.4 and cl.31.6.3 of IS 456 : 2000

Allowable shear stress $\tau_{v\ allowable}$ = $k_s \tau_c$

k_s = (0.5 + bc) = **1.72222** >1

Hence, k_s= **1**

$\tau_c = 0.25\ (fck)^{0.5}$ = **1.25** N/mm^2

$\tau_{v\ allowable} = k_s \times \tau_c$ = **1.25** N/mm^2

Shear force Vs = 813.497 (1.67682926829268 x 1.371951219 **1359.36** KN

Length of critical section = 2 x (934 + 674) = **3216** mm

Area of the critical section (length of critical sec x eff. d) = **1524384** mm^2

Hence shear stress τ_v = **0.892** N/mm^2

tv < allowable hence O.K.

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TYPICAL BEAM DESIGN

SECTION DETAILS — Floor Level :

Width of the beam	B	=	200.00	mm
Depth of the beam	D	=	380.00	mm
Clear cover of the beam	C_v	=	25	mm
Grade of concrete	f_{ck}	=	25	N/mm^2
Grade of steel	F_y	=	500	N/mm^2
Effective length of the beam	L	=	5.33	m

FORCE DETAILS FROM STAAD :

Max. sagging moment of the beam	=	72.00	kN m
LHS Support moment of the beam	=	133.00	kN m
RHS Support moment of the beam	=	133.00	KN m
Torsional Moment	=	0.01	KN m
Shear force at LHS of the beam	=	122.00	KN
Shear force at RHS of the beam	=	122.00	KN
Shear force at concentrated load location	=	0.00	KN

Design of beam at LHS section

Tension Reinf.

Bending Mt $(Mu)_L$	=	133.02	kN m
Breath of the beam (B	=	200	mm
Effective cover $(d')_L$	=	43.0	mm
Effective depth $(d)_L$	=	337.00	mm
x_u max/d	=	0.4560	
$K_L = M_{UL}/(bd^2 f_{ck})$	=	0.2928	(5.857)
K'L	=	0.1338	

Hence Comp. reinf. Reqd.

Eff. Cover d' (Comp)	=	43.0	mm
A_{st} lim	=	512	mm^2
ΔA_{st}	=	565	mm^2
fsc	=	402.4	N/mm^2
A_{st} reqd	=	1077	mm^2

Layer	1st	2nd	3rd
Dia of reinf	20	16	12
Reqd No.	3.5	0	0
Provided	4	0	0
Ast prov.	1257	0	0

$A_{st\,provided}$ = 1257 mm^2

Compression Reinf.

$A_{st\,required}$	=	824	mm^2
$A_{st\,min.}$	=	628	mm^2

Layer	1st	2nd	3rd
Dia of reinf	20	12	12
Reqd No.	2.7	0	0
Provided	4	0	0
Ast prov.	1257	0	0

$A_{st\,provided}$ = 1257 mm^2

Shear Check

Shear force V_{UR}	=	122.00	kN
$\zeta_v = V_{UL}/bd$	=	1.81	N/mm^2
% of reinf provided	=	1.86	%
Design Stress	=	0.77	N/mm^2
ζ_c max	=	2.77	N/mm^2

2 Legged 8 mm Dia stirrups

Design Shear reinf.

Reqd. stirrup spac.	=	209.86	mm
Proived stirrup spac.	=	150	mm

Design of beam at Max sagging section

Tension Reinf.

Bending Mt $(Mu)_M$	=	72.00	kN m
Breath of the beam (B	=	200	mm
Effective cover $(d')_M$	=	59.8	mm
Effective depth $(d)_M$	=	320.22	mm
x_u max/d	=	0.4560	
$K_M = M_{UM}/(bd^2 f_{ck})$	=	0.1755	(3.511)
K'M	=	0.1338	

Hence Comp. reinf. Reqd.

Eff. Cover d' (Comp)	=	43.0	mm
A_{st} lim	=	486	mm^2
ΔA_{st}	=	142	mm^2
fsc	=	400.2	N/mm^2
A_{st} reqd	=	824	mm^2

Layer	1st	2nd	3rd
Dia of reinf	20	16	12
Reqd No.	2.7	0.98	0
Provided	2	2	0
Ast prov.	628	402	0

$A_{st\,provided}$ = 1030 mm^2

Compression Reinf.

$A_{st\,required}$	=	605	mm^2
$A_{st\,min.}$	=	314	mm^2

Layer	1st	2nd	3rd
Dia of reinf	20	12	12
Reqd No.	2	0	0
Provided	2	0	0
Ast prov.	628	0	0

$A_{st\,provided}$ = 628 mm^2

Torsional Reinforcement

Torsional Moment	=	0.01	kNm
ζ_{ve}	=	1.81	N/mm^2
ζ_c max	=	2.77	N/mm^2
M_t	=	0.02	kNm
M_{s2}	=	0.00	kNm
Stirrup spacing reqd	=	209.50	mm
Sideface. Reinf reqd	=	0.00	mm^2
Provide 0 Nos of		12	mm on EVF
Ast prov.	=	0.0	mm^2

Design of beam at RHS section

Tension Reinf.

Bending Mt $(Mu)_R$	=	133.02	kN m
Breath of the beam (B	=	200	mm
Effective cover $(d')_R$	=	43.0	mm
Effective depth $(d)_R$	=	337.00	mm
x_u max/d		0.4560	
$K_R = M_{UR}/(bd^2 f_{ck})$	=	0.2928	(5.857)
K'R	=	0.1338	

Hence Comp. reinf. Reqd.

Eff. Cover d' (Comp)	=	43.0	mm
A_{st} lim	=	512	mm^2
ΔA_{st}	=	565	mm^2
fsc	=	402.4	N/mm^2
A_{st} reqd	=	1077	mm^2

Layer	1st	2nd	3rd
Dia of reinf	20	16	12
Reqd No.	3.5	0	0
Provided	4	0	0
Ast prov.	1257	0	0

$A_{st\,provided}$ = 1257 mm^2

Compression Reinf.

$A_{st\,required}$	=	824	mm^2
$A_{st\,min.}$	=	628	mm^2

Layer	1st	2nd	3rd
Dia of reinf	20	12	12
Reqd No	2.7	0	0
Provided	4	0	0
Ast prov.	1257	0	0

$A_{st\,provided}$ = 1257 mm^2

Shear Check

Shear force V_{UL}	=	122.00	kN
$\zeta_v = V_{UL}/bd$	=	1.81	N/mm^2
% of reinf provided	=	1.86	%
Design Stress	=	0.77	N/mm^2
ζ_c max	=	2.77	N/mm^2

2 Legged 8 mm Dia stirrups

Design Shear reinf.

Reqd. stirrup spac.	=	209.86	mm
Proived stirrup spac.	=	150	mm

CHECK FOR DEFLECTION : As per IS 456

Provided area of reinforcement	=	1030	
Mu/bd^2	=	3.51	
Actual Span / Effective depth Ratio	=	16.64	
Support Condition	=	Continuous	
Design service stress	=	231.83	
Modification factor - Tension reinf.	=	1.01	
Modification factor - Comp. reinf.	=	1.25	
Allowable Span /Effective depth Ratio	=	32.84	

===<<< Hence SAFE in Deflection >>>===

Beam No	Size	Bottom steel		Top steel			Stirrups		Side Face Reinf.
		Straight	Curtail	Middle	LHS	RHS	Ends	Mid	
RB8	200 x 380	2Y20+2Y16	---	2Y20	4Y20	4Y20	2L Y8/150	2L Y8/200	

CLIENT:	M/s. SREEVATSA URBAN VILLAGE	DESIGN:	G + 2 Floors	ARCHITECT & PLANNERS SANKAR & ASSOCIATES 27 SENGUPTA STREET, RAMNAGAR, COIMBATORE.
STRUCTURE NAME :	RESIDENTIAL BUILDING	TITLE:	REPORT ON STRUCTURAL DESIGN.	
PLACE:	COIMBATORE			

TYPICAL SLAB DESIGN

SPANS :

Shorter span c/c w.r.to beam	B	=	1.90	m
Longer span c/c w.r.to beam	L	=	5.25	m
Width of support - shorter span - Left	bxl	=	200	mm
Width of support - shorter span - Right	bxr	=	200	mm
Width of support - longer span - Left	byl	=	200	mm
Width of support - longer span - Right	byr	=	200	mm
Clear Cover of slab	cv	=	20	mm
Ratio Ly / Lx	ly/lx	=	2.77	> 2

LOAD CALCULATION :

Assume the thickness of the slab	=	125.00	mm
Effective Depth of the of the slab	=	101.00	mm
Self Weight of slab	=	3.125	KN / m^2
Weathering course / Floor finishing	=	2.500	KN / m^2
Wall load	=	1.000	KN / m^2
Live Load	=	3.000	KN / m^2
D L+L L	=	9.625	KN / m^2
1.5D.L+1 5L L	=	14.438	KN / m^2

DESIGN AS ONE WAY SLAB

Support Condition — ONE SHORT END CONTINUOUS

EFFECTIVE SPANS :

Shorter span: (Lx)

Clear span + effective depth	=	1.80	m
Center to centre of supports	=	1.90	m
Effective span	=	1.90	m

Longer span: (Ly)

Clear span + effective depth	=	5.15	m
Center to centre of supports	=	5.25	m
Effective span	=	5.25	m
Aspect Ratio Ly / Lx	=	2.76	> 2

+ ve Alpha - x	=	0.100
- ve Alpha - x	=	0.100
+ ve Alpha - y	=	0.000
- ve Alpha - y	=	0.000

Compressive Stength of Concrete	=	25.00	N/mm^2
Grade of steel	=	500.00	N/mm^2

BENDING MOMENTS:

EFFECTIVE DEPTH OF SLAB

Positive moment along Shorter dir	Mx +ve =	5.21	KNm / m	dx+	=	101.00	mm	Kx+	=	0.026	Zx+	=	95.950	mm
Negative moment along Shorter dir.	Mx -ve =	5.21	KNm / m	dx-	=	101.00	mm	Kx-	=	0.026	Zx-	=	95.950	mm
Positive moment along Longer dir.	My +ve =	0.00	KNm / m	dy+	=	93.00	mm	Ky+	=	0.000	Zy+	=	88.350	mm
Negative moment along Longer dir.	My -ve =	0.00	KNm / m	dy-	=	101.00	mm	Ky-	=	0.000	Zy-	=	95.950	mm

REQUIRED AREA OF STEEL :

Ast req. at midspan along x- dir.	ast +ve x =	122.34	mm^2
Ast req. at midspan along y- dir.	ast +ve y =	0.00	mm^2
Ast req. at support along x- dir.	ast -ve x =	122.34	mm^2
Ast req. at support along y- dir.	ast -ve y =	0.00	mm^2
Minimum Ast required	ast min =	150.00	mm^2

REQUIRED SPACING OF STEEL:

8	mm dia at midspan along x- dir.	=	335 mm c/c
8	mm dia at midspan along y- dir.	=	335 mm c/c
8	mm dia at support along x- dir.	=	335 mm c/c
8	mm dia at support along y- dir.	=	335 mm c/c
8	mm dia for distribution steel	=	335 mm c/c
0	mm dia for top of mid span	=	150 mm c/c

PROVIDED SPACING OF STEEL

Bottom reinforcement along x- dir.	Y	8	150	mm c/c
Bottom reinforcement along y- dir.	Y	8	200	mm c/c
Top reinforcement along x- dir.	Y	8	150	mm c/c
Top reinforcement along y- dir.	Y	8	200	mm c/c
Distribution Steel	Y	8	150	mm c/c

CHECK FOR DEFLECTION :

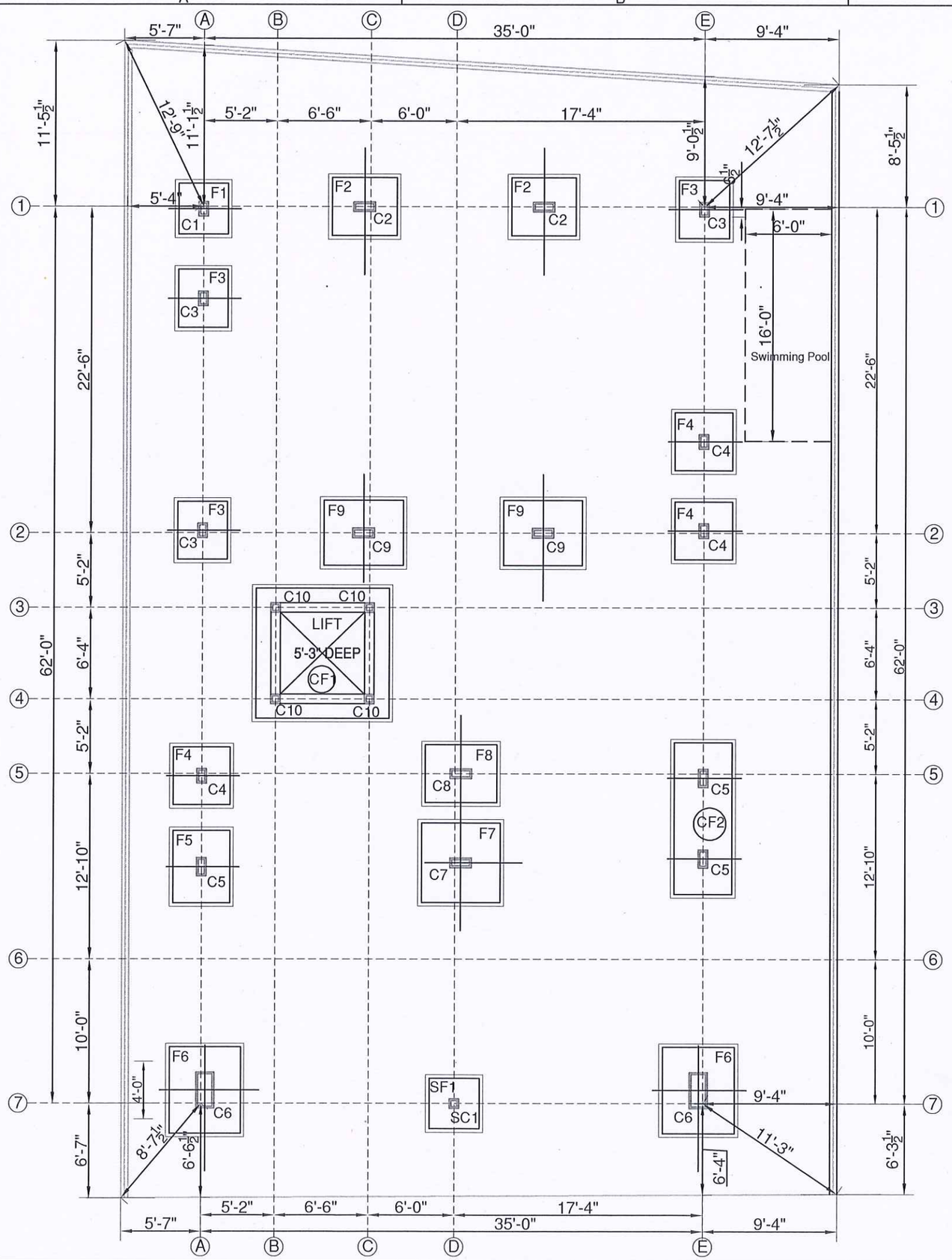
Provided Ast (Bottom)	=	335.10	mm^2
Provided Ast (Top)	=	0.00	mm^2
Mu/bd^2	=	0.51	
Actual Span / Effective depth Ratio	=	18.81	
Design service stress	=	105.87	
Modification factor - Tension reinf.	=	2.00	
Modification factor - Comp reinf.	=	1.00	
Allowable Span /Effective depth Ratio	=	46.00	

===<<< Hence SAFE in Deflection >>>===

SANKAR & ASSOCIATES COIMBATORE

CLIENT:	M/s. SREEVATSA URBAN VILLAGE	DESIGN:	G + 2 Floors
STRUCTURE NAME :	RESIDENTIAL BUILDING	TITLE:	REPORT ON STRUCTURAL DESIGN.
PLACE:	COIMBATORE		

ARCHITECT & PLANNERS
SANKAR & ASSOCIATES
27 SENGUPTA STREET,
RAMNAGAR,
COIMBATORE.



SCHEDULE OF COLUMNS AND FOOTINGS

COLUMN TYPE	COLUMN SIZE	FOOTING TYPE	FOTING SIZE	PIT SIZE
C1	8" x 1'-0"	F1	3'-6" x 3'-6"	4'-0" x 4'-0"
C2	8" x 1'-6"	F2	4'-0" x 4'-6"	4'-6" x 5'-0"
C3	8" x 1'-0"	F3	3'-6" x 4'-0"	4'-0" x 4'-6"
C4	8" x 1'-0"	F4	4'-0" x 4'-0"	4'-6" x 4'-6"
C5	8" x 1'-3"	F5	4'-0" x 5'-0"	4'-6" x 5'-6"
*C6	1'-3" x 2'-6" Up to Ground Roof	F6	5'-0" x 6'-0"	5'-6" x 6'-6"
C7	8" x 1'-6"	F7	5'-6" x 5'-6"	6'-0" x 6'-0"
C8	8" x 1'-6"	F8	4'-0" x 5'-0"	4'-6" x 5'-6"
C9	8" x 1'-6"	F9	4'-6" x 5'-6"	5'-0" x 6'-0"
C10	8" x 8"	F10	Refer Combined Footing	
**SC1	8" x 8" Terminated @ Plinth Level	**SC1	3'-6" x 3'-6"	4'-0" x 4'-0"
4 # C10		CF1	9'-0" x 9'-3"	9'-6" x 9'-9"
2 # C5		CF2	4'-0" x 10'-6"	4'-6" x 11'-0"

NOTES :

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2. ALL DIMENSIONS ARE TO BE VERIFIED PROPERLY AT SITE.
3. ALL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT DRAWINGS.
4. ϕ - DIA OF TOR STEEL
5. CLEAR COVER TO MAIN REINFORCEMENT
   a) FOR COLUMNS = 40 mm, b) FOR FOOTINGS : BOTTOM : 50mm, SIDES : 75mm
6. LAPPING:
   a) LAP LENGTH SHALL BE = 50 x DIA OF BAR.
   b) DEVELOPMENT LENGTH SHALL BE = 48 x DIA OF BAR.
7. ALL ISOLATED FOOTINGS SHALL BE CONCENTRIC WITH COLUMN UNLESS OTHERWISE SPECIFIED
8. GRADE OF CONCRETE IS M25
9. GRADE OF REINFORCING STEEL IS Fe 500
10. ASSUMED SBC OF SOIL = 390 KN / Sqm.
11. MINIMUM DEPTH OF FOUNDATION = 3.2M BELOW NGL
12. THIS STRUCTURE IS DESIGNED FOR GROUND + 2 FLOORS

PLAN SHOWING THE PROPOSED RESIDENTIAL VILLA GROUP DEVELOPMENT BUILDINGS AT S.F.NO:527/1B2, 527/1C2,527/1D2,541/2A2A, 541/2A3A & 541/3, CHINNAVEDAMPATTI VILLAGE, NORTH ZONE & TALUK, COIMBATORE CORPORATION.

REV.,	DESCRIPTION	REV BY.,	DATE	APP BY.,

RELEASE STATUS	DATE	SIGNATURE
PRELIMINARY/ TENDER		
FOR CONSTRUCTION		

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STRUCTURAL ENGINEER

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SREEVATSA URBAN VILLAGE
AT COIMBATORE

CLIENT:-

REFERENCE:

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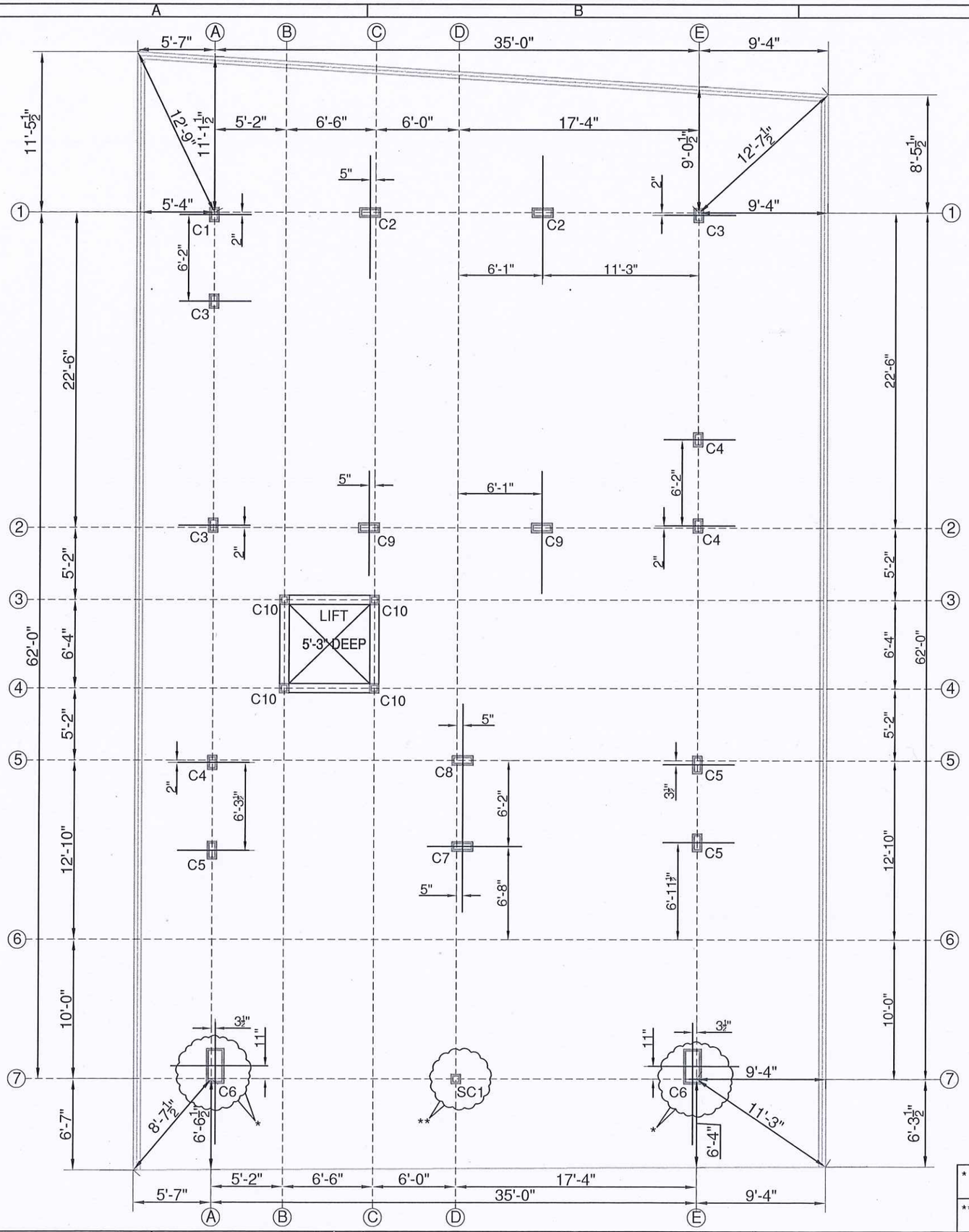
BUILDING: SOUTH FACING VILLA

DETAIL: COLUMN AND FOOTING LAYOUT

SCALE: 1"= 8'-0"	JOB NO: 17-SRE-GV-02	REV	DRN BY: UMA
DATE: 18.11.2023	DRG NO: ST-FL-001		CKD BY: GOWDAMAN
	SHEET NO:		APD BY: MURUGESH

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E-mail : admin@sankarassociates.in
Website : www.sankarassociates.in

A3 - F



SCHEDULE OF COLUMNS

TYPE	COLUMN SIZE
C1	8" x 1'-0"
C2	8" x 1'-6"
C3	8" x 1'-0"
C4	8" x 1'-0"
C5	8" x 1'-3"
*C6	1'-3" x 2'-6" Upto Ground Roof
C7	8" x 1'-6"
C8	8" x 1'-6"
C9	8" x 1'-6"
C10	8" x 8"
**SC1	8" x 8" Terminated @ Plinth Level

NOTES :

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3. ALL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT DRAWINGS.
4. ℄ - DIA OF TOR STEEL
5. CLEAR COVER TO MAIN REINFORCEMENT
   a) FOR COLUMNS = 40 mm
6. LAPPING:
   a) LAP LENGTH SHALL BE = 50 x DIA OF BAR.
   b) DEVELOPMENT LENGTH SHALL BE = 48 x DIA OF BAR.
7. ALL ISOLATED FOOTINGS SHALL BE CONCENTRIC WITH COLUMN UNLESS OTHERWISE SPECIFIED
8. GRADE OF CONCRETE IS M25
9. GRADE OF REINFORCING STEEL IS Fe 500
10. ASSUMED SBC OF SOIL = 390 KN / Sqm.
11. MINIMUM DEPTH OF FOUNDATION = 3.2M BELOW NGL
12. THIS STRUCTURE IS DESIGNED FOR GROUND + 2 FLOORS

PLAN SHOWING THE PROPOSED RESIDENTIAL VILLA GROUP DEVELOPMENT BUILDINGS AT S.F.NO:527/1B2, 527/1C2,527/1D2,541/2A2A, 541/2A3A & 541/3, CHINNAVEDAMPATTI VILLAGE, NORTH ZONE & TALUK, COIMBATORE CORPORATION.

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SREEVATSA URBAN VILLAGE
AT COIMBATORE

CLIENT:-

REFERENCE:

FOR APPROVAL PURPOSE ONLY

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BUILDING: SOUTH FACING VILLA

DETAIL: COLUMN CENTRE LINE (UPTO GROUND ROOF)

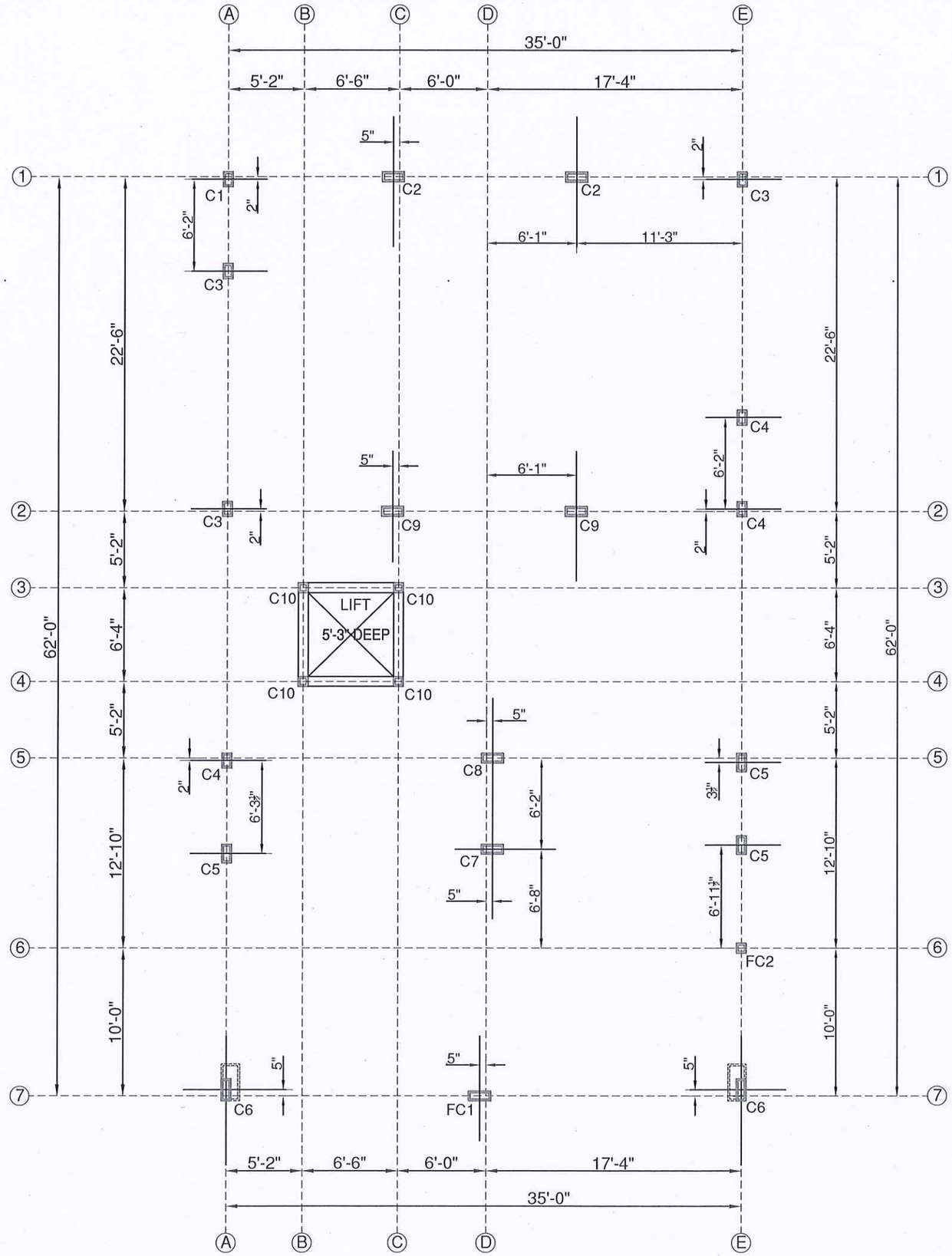
SCALE: 1"= 8'-0"	JOB NO: 17-SRE-GV-02	REV	DRN BY: UMA
DATE: 18.11.2023	DRG NO: ST-CL-001		CKD BY: GOWDAMAN
	SHEET NO: 01 OF 02		APD BY: MURUGESH

* COLUMN SIZE TO BE REDUCE ABOVE GROUND ROOF

** COLUMN TO BE TERMINATED AT PLINTH LEVEL

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Website : www.sankarassociates.in

A3 - F



SCHEDULE OF COLUMNS

TYPE	COLUMN SIZE
C1	8" x 1'-0"
C2	8" x 1'-6"
C3	8" x 1'-0"
C4	8" x 1'-0"
C5	8" x 1'-3"
*C6	8" x 1'-6"
C7	8" x 1'-6"
C8	8" x 1'-6"
C9	8" x 1'-6"
C10	8" x 8"
FC1	8" x 1'-6"
FC2	8" x 8"

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4. ϕ - DIA OF TOR STEEL
5. CLEAR COVER TO MAIN REINFORCEMENT (MILD EXPOSURE)
   a) FOR COLUMNS = 40 mm
6. LAPPING:
   a) LAP LENGTH SHALL BE = 50 x DIA OF BAR.
   b) DEVELOPMENT LENGTH SHALL BE = 48 x DIA OF BAR.
7. ALL ISOLATED FOOTINGS SHALL BE CONCENTRIC WITH COLUMN UNLESS OTHERWISE SPECIFIED
8. GRADE OF CONCRETE IS M25
9. GRADE OF REINFORCING STEEL IS Fe 500
10. ASSUMED SBC OF SOIL = 390 KN / Sqm.
11. MINIMUM DEPTH OF FOUNDATION = 3.2M BELOW NGL
12. THIS STRUCTURE IS DESIGNED FOR GROUND + 2 FLOORS

PLAN SHOWING THE PROPOSED RESIDENTIAL VILLA GROUP DEVELOPMENT BUILDINGS AT S.F.NO:527/1B2, 527/1C2,527/1D2,541/2A2A, 541/2A3A & 541/3, CHINNAVEDAMPATTI VILLAGE, NORTH ZONE & TALUK, COIMBATORE CORPORATION.

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SREEVATSA URBAN VILLAGE AT COIMBATORE

CLIENT:-

REFERENCE:

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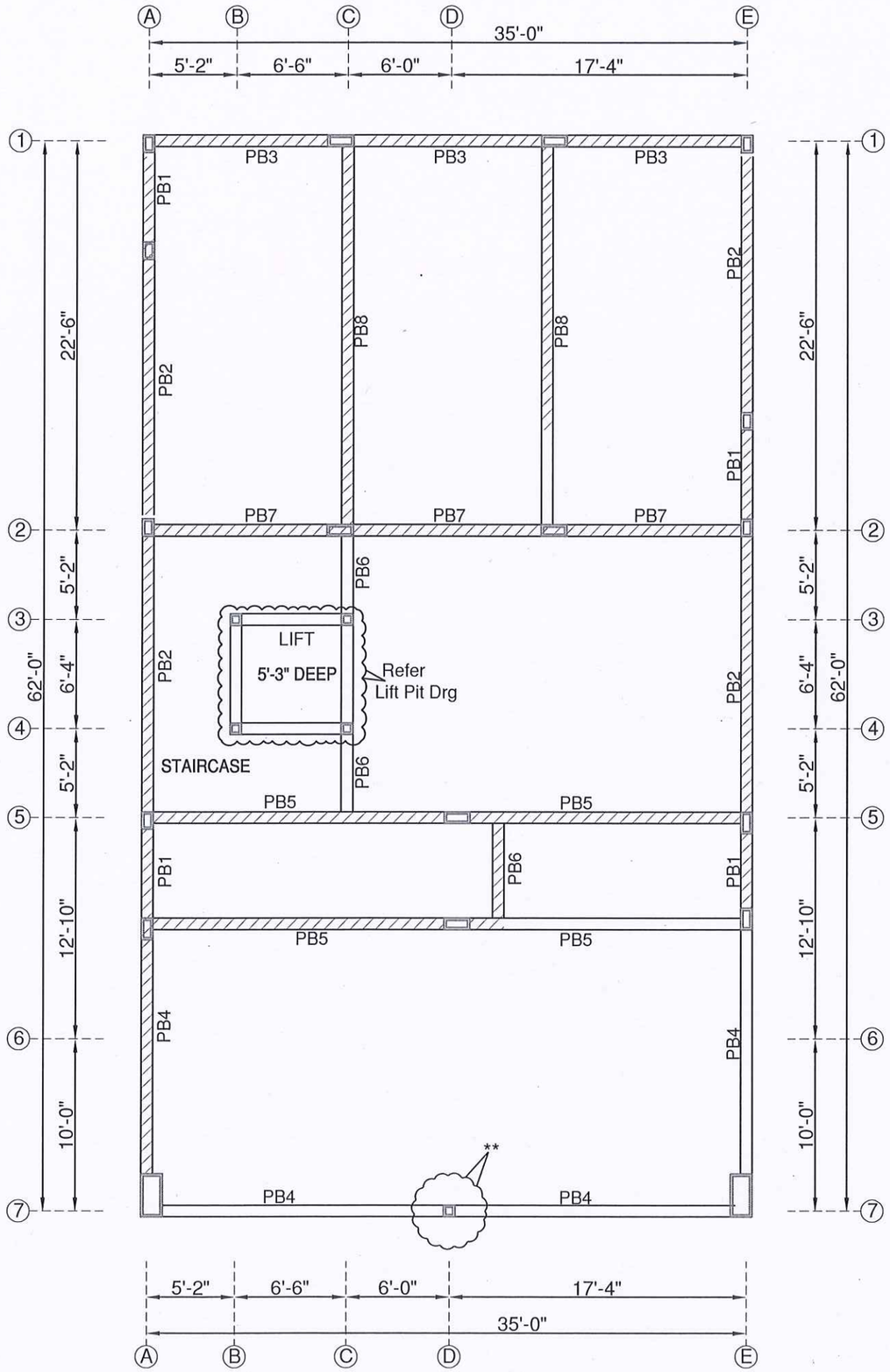
BUILDING: SOUTH FACING VILLA

DETAIL: COLUMN CENTRE LINE (ABOVE GROUND ROOF)

SCALE: 1"= 8'-0"	JOB NO: 17-SRE-GV-02	REV	DRN BY: UMA
DATE: 18.11.2023	DRG NO: ST-CL-001		CKD BY: GOWDAMAN
	SHEET NO: 02 OF 02		APD BY: MURUGESH

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A3 - F



SCHEDULE OF PLINTH BEAMS	
TYPE	BEAM SIZE
PB1	8" x 1'-3"
PB2	8" x 1'-3"
PB3	8" x 1'-3"
PB4	8" x 1'-3"
PB5	8" x 1'-3"
PB6	8" x 1'-3"
PB7	8" x 1'-3"
PB8	8" x 1'-3"

** COLUMN TO BE TERMINATED AT THIS LEVEL

NOTES :

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3. ALL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT DRAWINGS.
4. Ø - DIA OF TOR STEEL
5. CLEAR COVER TO MAIN REINFORCEMENT
   a) FOR BEAMS = 25 mm b) FOR SLABS = 20 mm
6. LAPPING:
   a) LAP LENGTH SHALL BE = 50 x DIA OF BAR.
   b) DEVELOPMENT LENGTH SHALL BE = 50 x DIA OF BAR.
7. PROVIDE CHAIR RODS WHEREVER NECESSARY
8. PROVIDE SPACER RODS OF 25mm ø (min) AT 1000mm C/C
9. GRADE OF CONCRETE IS M25
10. GRADE OF REINFORCING STEEL IS Fe 500
11. ALL THE LEVELS MENTIONED ARE FROM THE F.F.L. OF RESPECTIVE FLOOR

PLAN SHOWING THE PROPOSED RESIDENTIAL VILLA GROUP DEVELOPMENT BUILDINGS AT S.F.NO:527/1B2, 527/1C2,527/1D2,541/2A2A, 541/2A3A & 541/3, CHINNAVEDAMPATTI VILLAGE, NORTH ZONE & TALUK, COIMBATORE CORPORATION.

REV.,	DESCRIPTION	REV BY.,	DATE	APP BY.,

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SREEVATSA URBAN VILLAGE AT COIMBATORE

CLIENT:-

REFERENCE:

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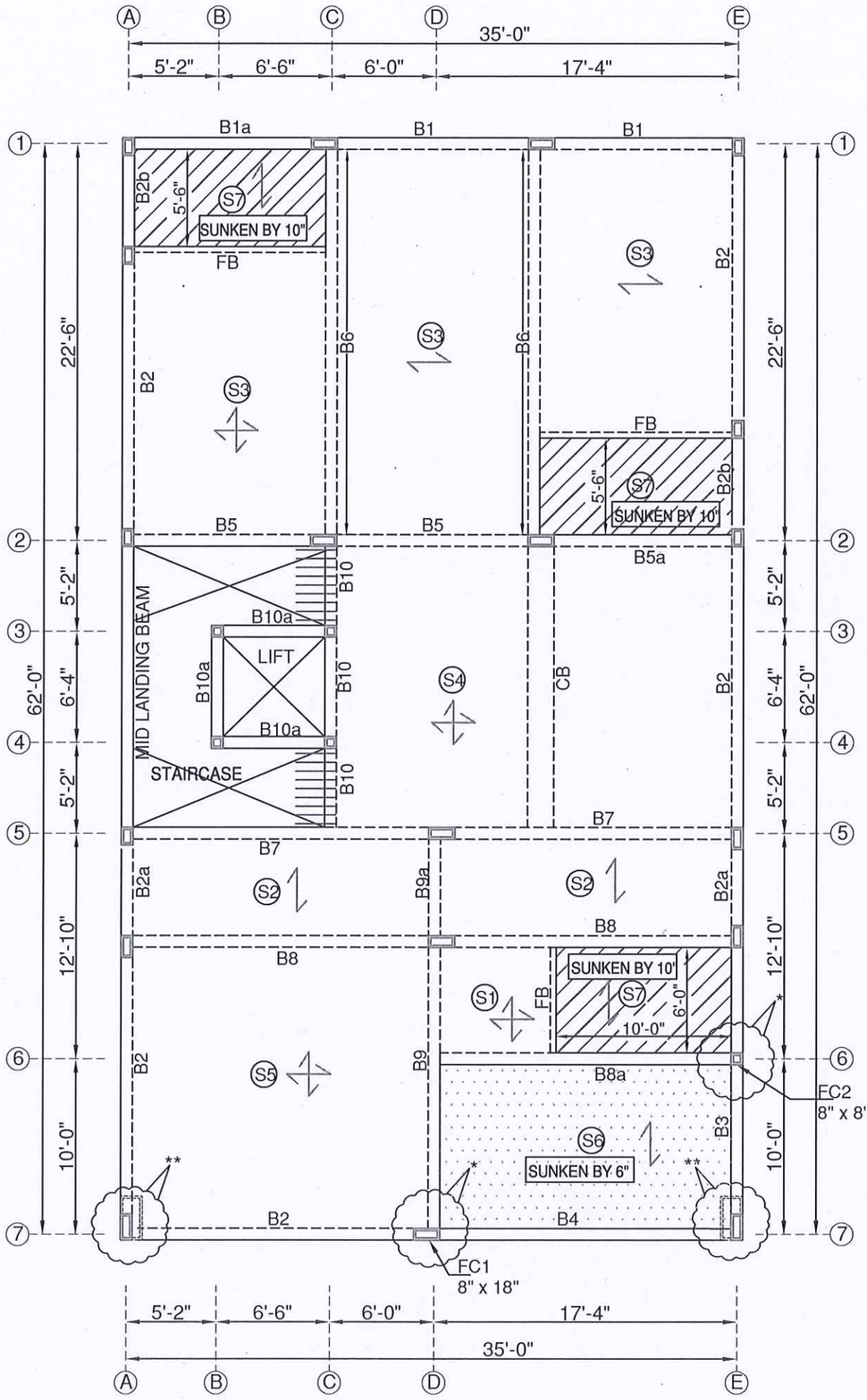
BUILDING: SOUTH FACING VILLA -TYPE -03

DETAIL: PLINTH BEAM LAYOUT AND DETAILS

SCALE: 1"= 8'-0"	JOB NO: 17-SRE-GV-02	REV	DRN BY: UMA
DATE: 17.11.2023	DRG NO: ST-PL-001		CKD BY: GOWDAMAN
	SHEET NO: 01 OF 02		APD BY: MURUGESH

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27, Senguptha Street, Ramnagar,
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Phone : +91-422-2230890
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Ashok Nagar, Chennai - 600 083
E-mail : admin@sankarassociates.in
Website : www.sankarassociates.in

A3 - P



SCHEDULE OF GROUND ROOF BEAMS

FLOOR TO FLOOR HEIGHT : 11'-3", SLAB TOP LVL -11'-0", FLOOR FINISH : 3"

BEAM NO.	BEAM SIZE	BEAM BOTTOM LVL	BEAM NO.	BEAM SIZE	BEAM BOTTOM LVL
B1	8" × 1'-3"	+ 9'-9" LVL	B8	8" × 1'-3"	+ 9'-9" LVL
B1a	8" × 1'-3"	+ 9'-9" LVL	B8a	8" × 1'-3"	+ 9'-9" LVL
B2	8" × 1'-3"	+ 9'-9" LVL	B9	8" × 1'-3"	+ 9'-9" LVL
B2a	8" × 1'-3"	+ 9'-9" LVL	B9a	8" × 1'-3"	+ 9'-9" LVL
B2b	8" × 1'-3"	+ 9'-9" LVL	B10	8" × 1'-3"	+ 9'-6" LVL
B3	8" × 1'-3"	+ 9'-9" LVL	B10a	8" × 1'-3"	+ 9'-6" LVL
B4	8" × 1'-3"	+ 9'-9" LVL	FB	4" × 1'-3"	+ 9'-6" LVL
B5	8" × 1'-3"	+ 9'-9" LVL	CB	1'-3" Slab Tk	
B5a	8" × 1'-3"	+ 9'-9" LVL			
B6	8" × 1'-6"	+ 9'-6" LVL			
B7	8" × 1'-3"	+ 9'-9" LVL			

SLAB TYPE	SLAB THICK	SLAB BOTTOM LVL
S1	5"	+ 10'-7" LVL
S2	5"	+ 10'-7" LVL
S3	5"	+ 10'-7" LVL
S4	6"	+ 10'-6" LVL
S5	6"	+ 10'-6" LVL
S6	5"	+ 10'-1" LVL
S7	5"	+ 9'-9" LVL

ONE WAY SLAB

TWO WAY SLAB

Toilet Sunken 10"

Terrace Sunken 6"

NOTES :

1. THE DIMENSIONS ARE NOT TO BE SCALED.
2. ALL DIMENSIONS ARE TO BE VERIFIED PROPERLY AT SITE.
3. ALL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT DRAWINGS.
4. Ø - DIA OF TOR STEEL
5. CLEAR COVER TO MAIN REINFORCEMENT
   a) FOR BEAMS = 25 mm b) FOR SLABS = 20 mm
6. LAPPING:
   a) LAP LENGTH SHALL BE = 50 x DIA OF BAR.
   b) DEVELOPMENT LENGTH SHALL BE = 50 x DIA OF BAR.
7. PROVIDE CHAIR RODS WHEREVER NECESSARY
8. PROVIDE SPACER RODS OF 25mm ø (min) AT 1000mm C/C
9. GRADE OF CONCRETE IS M25
10. GRADE OF REINFORCING STEEL IS Fe 500
11. ALL THE LEVELS MENTIONED ARE FROM THE F.F.L. OF RESPECTIVE FLOOR

* 8" x 1'-6", 8" x 8" FLOATING COLUMN ABOVE THIS ROOF

**COLUMN SIZE TO BE REDUCED ABOVE THIS ROOF SLAB

PLAN SHOWING THE PROPOSED RESIDENTIAL VILLA GROUP DEVELOPMENT BUILDINGS AT S.F.NO:527/1B2, 527/1C2,527/1D2,541/2A2A, 541/2A3A & 541/3, CHINNAVEDAMPATTI VILLAGE, NORTH ZONE & TALUK, COIMBATORE CORPORATION.

REV.	DESCRIPTION	REV BY.	DATE	APP BY.

RELEASE STATUS	DATE	SIGNATURE
PRELIMINARY/ TENDER		
FOR CONSTRUCTION		

IMPORTANT NOTES:-

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S. MURUGESH, M.E.,FIE.,
Chartered Engineer F-1279323
Structural Engineer Grade-I
(Coimbatore LPA/SE/Gr.-I/19-04-19)

STRUCTURAL ENGINEER

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SREEVATSA URBAN VILLAGE
AT COIMBATORE

CLIENT:-

REFERENCE:

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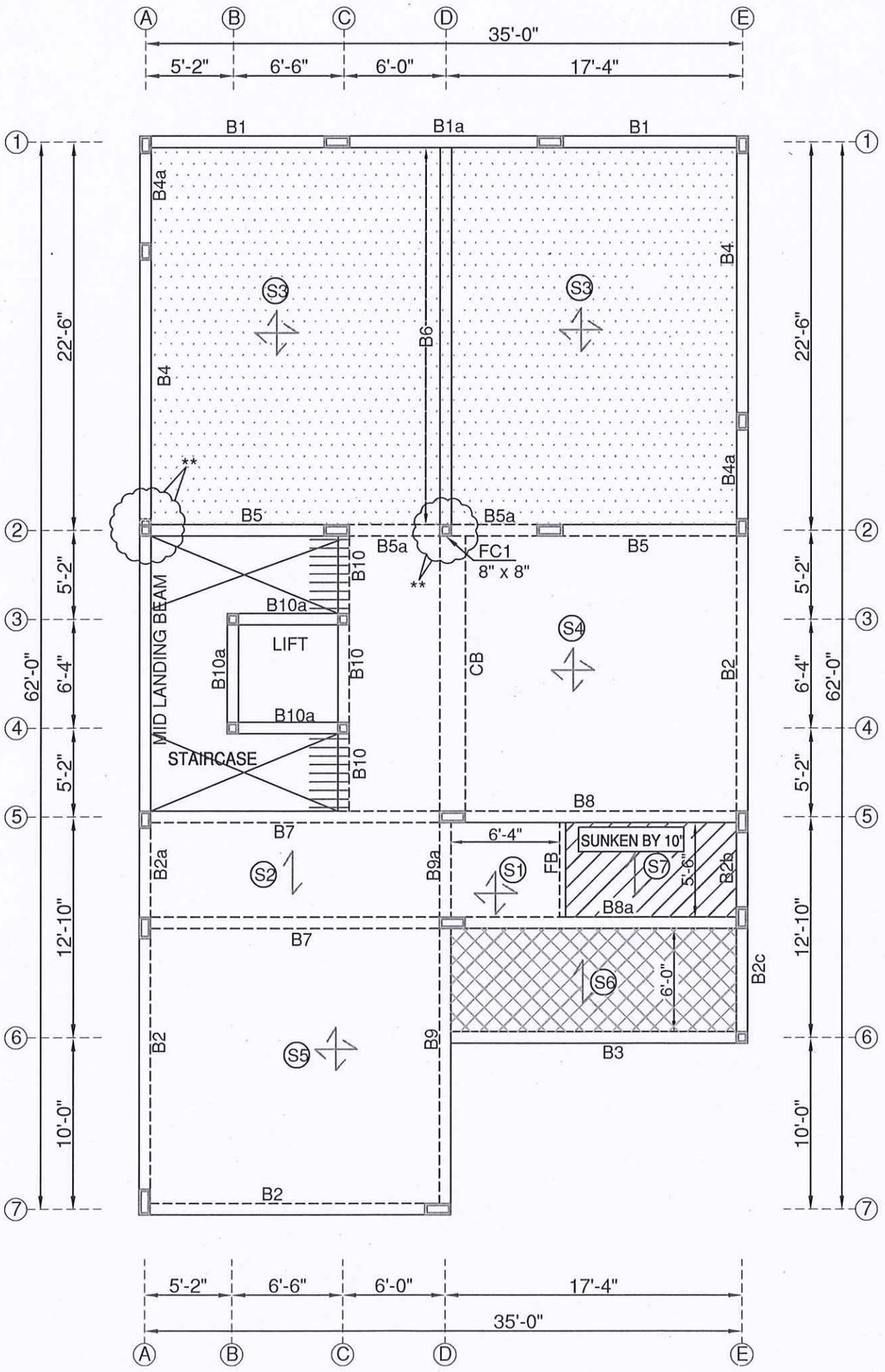
BUILDING: SOUTH FACING VILLA

DETAIL: GROUND FLOOR ROOF SLAB & BEAM SHUTTERING LAYOUT

SCALE: 1"= 8'-0"	JOB NO: 17-SRE-GV-02	REV	DRN BY: UMA
DATE: 18.11.2023	DRG NO:ST-RF-00F-001		CKD BY: GOWDAMAN
	SHEET NO:		APD BY: MURUGESH

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A3 - R



SCHEDULE OF FIRST ROOF BEAMS

FLOOR TO FLOOR HEIGHT : 11'-3", SLAB TOP LVL -11'-0", FLOOR FINISH : 3"

BEAM NO.	BEAM SIZE	BEAM BOTTOM LVL	BEAM NO.	BEAM SIZE	BEAM BOTTOM LVL
B1	8" x 1'-3"	+ 9'-9" LVL	B8	8" x 1'-3"	+ 9'-9" LVL
B1a	8" x 1'-6"	+ 9'-6" LVL	B8a	8" x 1'-3"	+ 9'-9" LVL
B2	8" x 1'-3"	+ 9'-9" LVL	B9	8" x 1'-3"	+ 9'-9" LVL
B2a	8" x 1'-3"	+ 9'-9" LVL	B9a	8" x 1'-3"	+ 9'-9" LVL
B2b	8" x 1'-3"	+ 9'-9" LVL	B10	8" x 1'-3"	+ 9'-6" LVL
B3	8" x 1'-3"	+ 9'-9" LVL	B10a	8" x 1'-3"	+ 9'-6" LVL
B4	8" x 1'-6"	+ 9'-6" LVL	FB	4" x 1'-3"	+ 9'-6" LVL
B5	8" x 1'-3"	+ 9'-9" LVL	CB	1'-6" x Slab Tk	
B5a	8" x 1'-3"	+ 9'-9" LVL			
B6	8" x 1'-6"	+ 9'-6" LVL			
B7	8" x 1'-3"	+ 9'-9" LVL			

SLAB TYPE	SLAB THICK	SLAB BOTTOM LVL
S1	5"	+ 10'-7" LVL
S2	5"	+ 10'-7" LVL
S3	6"	+ 10'-0" LVL
S4	6"	+ 10'-6" LVL
S5	6"	+ 10'-6" LVL
S6	5"	+ 10'-1" LVL
S7	5"	+ 9'-9" LVL

ONE WAY SLAB

TWO WAY SLAB

Toilet Sunken 10"

Terrace Sunken 6"

Balcony Sunken 6"

NOTES :

1. THE DIMENSIONS ARE NOT TO BE SCALED.
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3. ALL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT DRAWINGS.
4. ℞ - DIA OF TOR STEEL
5. CLEAR COVER TO MAIN REINFORCEMENT
   a) FOR BEAMS = 25 mm b) FOR SLABS = 20 mm
6. LAPPING:
   a) LAP LENGTH SHALL BE = 50 x DIA OF BAR.
   b) DEVELOPMENT LENGTH SHALL BE = 50 x DIA OF BAR.
7. PROVIDE CHAIR RODS WHEREVER NECESSARY
8. PROVIDE SPACER RODS OF 25mm ø (min) AT 1000mm C/C
9. GRADE OF CONCRETE IS M25
10. GRADE OF REINFORCING STEEL IS Fe 500
11. ALL THE LEVELS MENTIONED ARE FROM THE F.F.L. OF RESPECTIVE FLOOR

* 8" x 8" FLOATING COLUMN ABOVE THIS ROOF
**COLUMN SIZE TO BE REDUCED ABOVE THIS ROOF SLAB

PLAN SHOWING THE PROPOSED RESIDENTIAL VILLA GROUP DEVELOPMENT BUILDINGS AT S.F.NO:527/1B2, 527/1C2,527/1D2,541/2A2A, 541/2A3A & 541/3, CHINNAVEDAMPATTI VILLAGE, NORTH ZONE & TALUK, COIMBATORE CORPORATION.

REV.,	DESCRIPTION	REV BY.,	DATE	APP BY.,

RELEASE STATUS	DATE	SIGNATURE
PRELIMINARY/ TENDER		
FOR CONSTRUCTION		

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Chartered Engineer F-1279323
Structural Engineer Grade-I
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SREEVATSA URBAN VILLAGE
AT COIMBATORE

CLIENT:-

REFERENCE:

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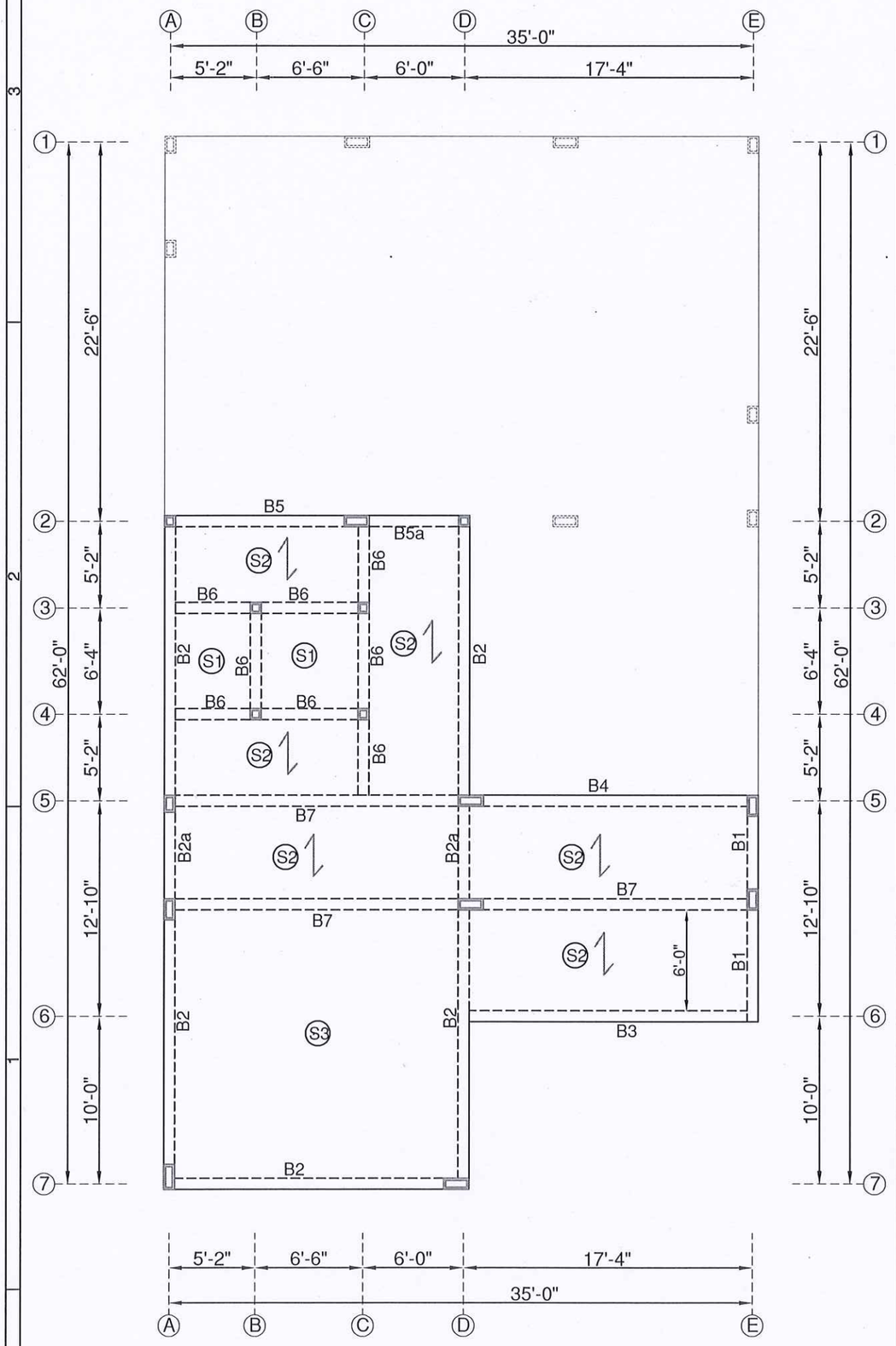
BUILDING: SOUTH FACING VILLA -TYPE -03

DETAIL: FIRST FLOOR ROOF SLAB & BEAM SHUTTERING LAYOUT

SCALE: 1"= 8'-0"	JOB NO: 17-SRE-GV-02	REV	DRN BY: UMA
DATE: 18.11.2023	DRG NO:ST-RF-01F-001		CKD BY: GOWDAMAN
	SHEET NO:		APD BY: MURUGESH

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A3 - R



SCHEDULE OF FIRST ROOF BEAMS

FLOOR TO FLOOR HEIGHT : 11'-3", SLAB TOP LVL -11'-0", FLOOR FINISH : 3"

BEAM NO.	BEAM SIZE	BEAM BOTTOM LVL
B1	8" × 1'-3"	+ 9'-9" LVL
B2	8" × 1'-3"	+ 9'-9" LVL
B3	8" × 1'-3"	+ 9'-9" LVL
B4	8" × 1'-6"	+ 9'-6" LVL
B5	8" × 1'-3"	+ 9'-9" LVL
B5a	8" × 1'-3"	+ 9'-9" LVL
B6	8" × 1'-3"	+ 9'-6" LVL
B7	8" × 1'-3"	+ 9'-9" LVL

SLAB TYPE	SLAB THICK	SLAB BOTTOM LVL
S1	6"	+ 10'-6" LVL
S2	5"	+ 10'-7" LVL
S3	6"	+ 10'-6" LVL

ONE WAY SLAB

TWO WAY SLAB

NOTES :

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4. ϕ - DIA OF TOR STEEL
5. CLEAR COVER TO MAIN REINFORCEMENT
   a) FOR BEAMS = 25 mm b) FOR SLABS = 20 mm
6. LAPPING:
   a) LAP LENGTH SHALL BE = 50 x DIA OF BAR.
   b) DEVELOPMENT LENGTH SHALL BE = 50 x DIA OF BAR.
7. PROVIDE CHAIR RODS WHEREVER NECESSARY
8. PROVIDE SPACER RODS OF 25mm ø (min) AT 1000mm C/C
9. GRADE OF CONCRETE IS M25
10. GRADE OF REINFORCING STEEL IS Fe 500
11. ALL THE LEVELS MENTIONED ARE FROM THE F.F.L. OF RESPECTIVE FLOOR

PLAN SHOWING THE PROPOSED RESIDENTIAL VILLA GROUP DEVELOPMENT BUILDINGS AT S.F.NO:527/1B2, 527/1C2,527/1D2,541/2A2A, 541/2A3A & 541/3, CHINNAVEDAMPATTI VILLAGE, NORTH ZONE & TALUK, COIMBATORE CORPORATION.

REV.,	DESCRIPTION	REV BY.,	DATE	APP BY.,

RELEASE STATUS	DATE	SIGNATURE
PRELIMINARY/ TENDER		
FOR CONSTRUCTION		

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S. MURUGESH, M.E.,FIE.,
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Structural Engineer Grade-I
(Coimbatore LPA/SE/Gr.-I/19-04-19)

STRUCTURAL ENGINEER

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SREEVATSA URBAN VILLAGE
AT COIMBATORE

CLIENT:-

REFERENCE:

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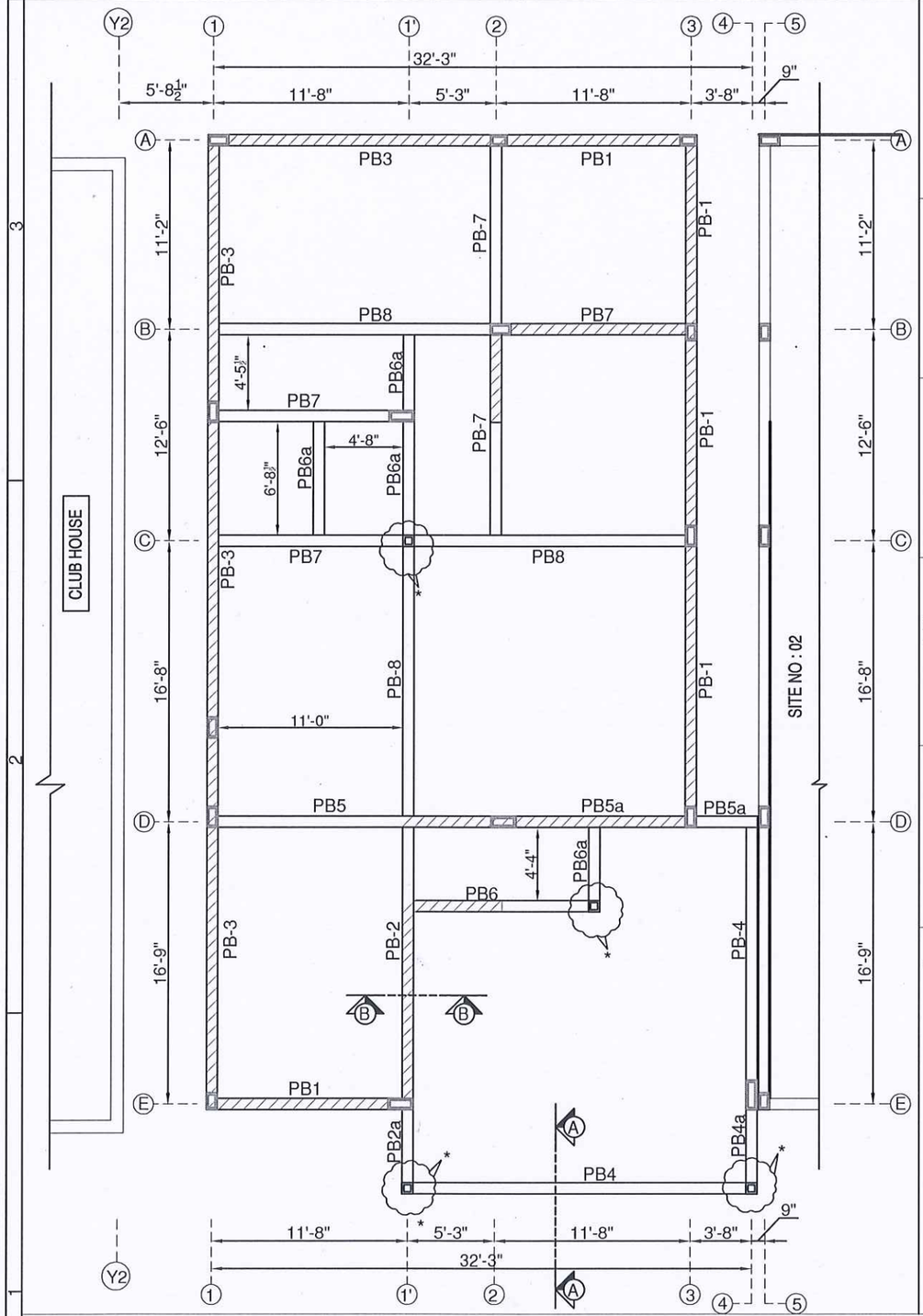
BUILDING: SOUTH FACING VILLA -TYPE -03

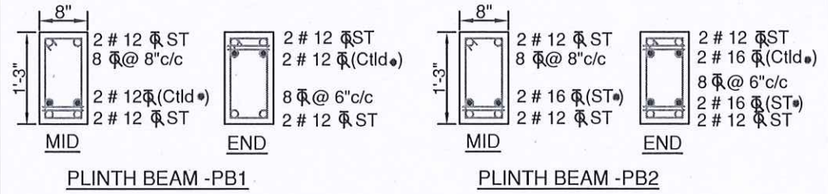
DETAIL: SECOND FLOOR ROOF SLAB & BEAM SHUTTERING LAYOUT

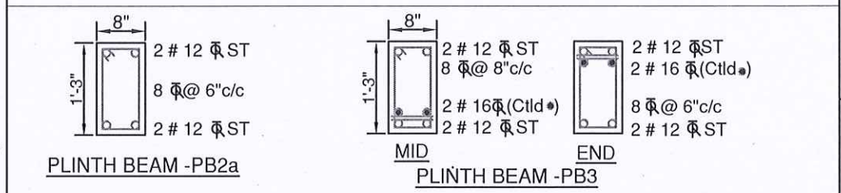
SCALE: 1"= 8'-0"	JOB NO: 17-SRE-GV-02	REV	DRN BY: UMA
DATE: 18.11.2023	DRG NO: ST-RF-02F-001		CKD BY: GOWDAMAN
	SHEET NO:		APD BY: MURUGESH

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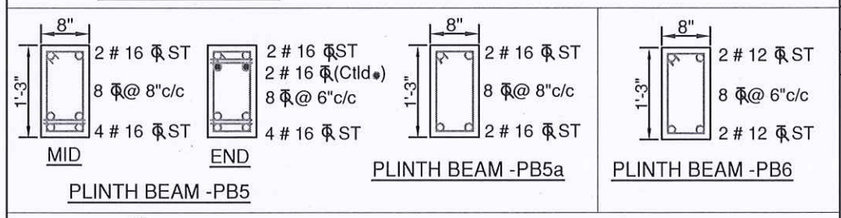
A3 - R

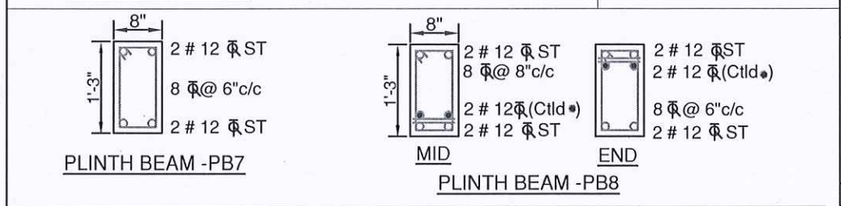


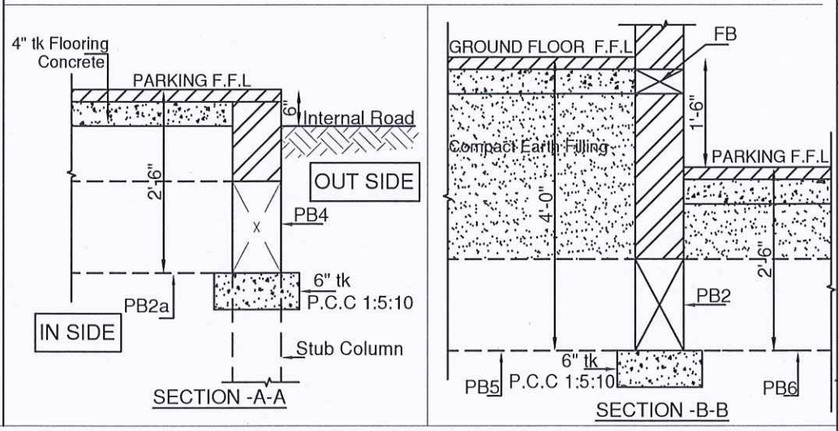












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9. GRADE OF CONCRETE IS M25
10. GRADE OF REINFORCING STEEL IS Fe 500
11. ALL THE LEVELS MENTIONED ARE FROM THE F.F.L. OF RESPECTIVE FLOOR

PLAN SHOWING THE PROPOSED RESIDENTIAL GROUP DEVELOPMENT BUILDINGS IN S.F.NO:365/2F AT THUDIYALUR VILLAGE, NORTH ZONE & TALUK, COIMBATORE CORPORATION.

* COLUMN TO BE TERMINATED AT PLINTH TOP LEVEL

REV.,	DESCRIPTION	REV BY.,	DATE	APP BY.,

RELEASE STATUS	DATE	SIGNATURE
PRELIMINARY/ TENDER		
FOR CONSTRUCTION		

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S. MURUGESH, M.E.,FIE.,
Chartered Engineer F-1279323
Structural Engineer Grade-I
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STRUCTURAL ENGINEER

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SREEVATSA POORNA
AT
THUDIYALUR

CLIENT:-

REFERENCE:

FOR APPROVAL PURPOSE ONLY

N

BUILDING: EAST FACING VILLA -SITE NO: 01

DETAIL: PLINTH BEAM LAYOUT & REINFORCEMENT DETAILS

SCALE: 1"= 8'-0"	JOB NO: SREL-24-01	REV	DRN BY: UMA
DATE: 24.04.2024	DRG NO: ST- PL-001		CKD BY: GOWDAMAN
	SHEET NO:		APD BY: MURUGESH

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A3 - P

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Property Development & Approvals

Building plan approvals, construction, layout and sub-division approvals, zone conversion, and occupancy certification.

NRI & Investment Services

Escrow Services, Rentals, Litigation Solutions, Property Valuation and Property Management

Compliance & Special Services

Demolition, Vaasthu checks, regularization of unauthorized buildings, and office space solutions.

Smarter Tools, Stronger Decisions

Property Valuation & Cost Estimation

Guideline Value, Apartment Composite Value, Stamp Duty & Registration Fees Calculator, Building Value Calculator, Capital Gains Calculator, Area Unit Converter, Length Unit Converter

Ownership, Compliance & Legal Verification

Encumbrance Certificate (EC) Search, Find Your SRO, Temple Property Check, WAQF Property Check, View Patta/Chitta, View FMB, Patta Analyzer, EC Analyzer

Zoning & Land Use Analysis

Land Use Zone Finder, CRZ Zone Check, Aquifer Zone Check, CBA Check, EWS Check, Road Width Check, Proposed Road Widening Check, FSI Calculator

Property Documentation & Workflow Tools

Forms & Templates, Document Generator, Document Translation, Dictionary

Vastu & Property Management Tools

Home Vastu Shastra, Plot Vastu Shastra, Property Management

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