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DIGITALLY VERIFIED RECORD

Dr.E.SENTHIL KUMAR, M.E.(Structures), M.I.E., Ph.D.,
Structural Engineer Grade -I: Reg No. DTCP/SALEM/SE G-I/001/2026
& Chartered Engineer Reg No. (M-163735-5)

STABILITY CERTIFICATE *13.08.2026*

1. *The detailed engineering and structure design of the proposed building Comprising of Stilt +5 floors at (Old S.No:- 96/1B, 96/2A2, 113/2A pt, 113/2B pt) Plot Nos- 12, 13, 14,15,16,17, Layout Approved by SLPA, Approval no: 61/2021) NEW WARD-'W', BLOCK-14, T.S.NO-1/5,2/4 at Annathanapatty village, Salem City Municipal Corporation Limit, Salem District has been done by me based on the Report of geotechnical investigation (soil test) done by Surya Builders/2023-2024/06/012 and considering the functional requirement of the building.*

2. *It is certified that among other factors, the proposed building has been designed to resist earthquake. I have checked various parameters and found that the proposed building would be safe.*

3. *I further certify that:*

   (i) The minimum Grade of concrete is M25 & Steel Grade is Fe550.

   (ii) The design and analysis has been done by using code of practice for plain and reinforced concrete as per IS 456, design load as per IS875 Part1 & 2 and design of Design of earthquake resistant structure as per IS Code 1893 part 1-2016.

4. *The building will be safe and sound when used for the purpose for which it is designed.*

Stamp and Signature



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GOVERNMENT OF TAMILNADU

TOWN AND COUNTRY PLANNING DEPARTMENT

SALEM DTCP

THE SALEM DISTRICT TOWN AND COUNTRY PLANNING

Phone: 0427-2448625

Email: ddslmregion@gmail.com

Website: www.tn.gov.in/tcp

PROFESSIONAL REGISTRATION CERTIFICATE

This is to certify that **THIRU E. SENTHIL KUMAR**has enrolled as Structural Engineer Grade - I **(SEG-I)** with Salem District Town and Country Planning who has the qualification of **Master of Engineering in Structural Engineering and PH.D**experience in the relevant field for 15 years.

He has been given the **Registration No:DTCP/SALEM/SE G-I /001/2026 on 18.03.2026.** He shall be required to comply with the duties and responsibilities prescribed by the Government of Tamil Nadu in Part V, "Registration of Professionals", Tamil Nadu Combined Development and Building Rules, 2019 approved vide G.O.(MS) No. 18, Municipal Administration and Water Supply (MA1) Dept., Dated 04.02.2019.

This Registration is valid upto**17.03.2031.**



18 MAR 2026

சேலம்-636 302

சேலம் மாவட்டம்

18/3/26

ASSISTANT DIRECTOR,
DISTRICT TOWN AND COUNTRY PLANNING,
SALEM DISTRICT.

18/3/26

DESIGN BASIS REPORT
FOR "SHREE KRISHNAA'S ELITE APARTMENT "BUILDING AT ANNATHANAPATTY, SALEM

CLIENT : Mr.K.PRABHAKARAN
SALEM SHRI KRISHNAA
CONSTRUCTION COMPANY
PRIVATE LIMITED

LOCATION :

Plot Nos- 12, 13, 14,15,16,17

NEW WARD-'W',

BLOCK-14,

T.S.NO-1/5,2/4 at Annathanapatty village,

Salem City Municipal Corporation Limit,

Salem District.

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1.2 INTRODUCTION:-

The objective of this document is to present a complete Design Basis Report on design intent covering design criteria including assumptions, references, load cases, load combinations, etc. for the proposed construction of 'SHREE KRISHNAA'S ELITE APARTMENT " BUILDING OF (STILT + 5 FLOORS)

The structural design of the project is governed by the architectural concept and the requirements for services. The design of the structure shall be comply with relevant Indian Standards & codes and best prevailing practices taking the following into consideration.

SECTION - II

STRUCTURE

2.1 LOCATION:-

NEW WARD-'W',

BLOCK-14,

T.S.NO-1/5,2/4 at Annathanapatty village.

Salem City Municipal Corporation Limit,

Salem District.

2.2 STRUCTURAL SYSTEM:-

2.2.1. FOUNDATION

Based on the recommendations of Geo technical report submitted by SHRI BALAJI GEOTECH Salem, The Safe Bearing capacity considered for design of Footing is 350 KN/m2 at 3.0m from Existing Ground level.

The Levelling concrete provided Footings with M15 grade Concrete.

Grade beams Provision given to safe guard the structure against differential settlements.

2.2.2. SUPER STRUCTURE:-

The building is analyzed and designed for the appropriate loads as per IS code.

The super structure is of regular R.C.C. frame with Stilt+ 5 Floors.

The details of load calculations, the various standards followed and typical design calculations of main structural elements are given in the following sections.

2.3. STRUCTURAL STANDARDS AND CODES:-

The building structure for SHREE KRISHNAA'S ELITE APARTMENT (Stilt+ 5 Floors) designed as reinforced concrete framed structure subjected to vertical loads, self-weight, dead load and live load. All structural elements are designed in accordance with the applicable requirements and recommendations of the reference codes and standards.

LIST OF RELAVENT IS CODE		
Sl.No	Code	Description
1	IS:875(Part1)–1987	Code of Practice for Design Loads (Other Than Earthquake) for Building and Structures–Unit Weigh to Building Materials and Stored Materials
2	IS:875(Part2)–1987	Code of Practice for Design Loads(Other than Earthquake) for Building and Structures–Imposed Loads
3	IS:875(Part5)–1987	Code Of Practice for Design Loads (Other than Earthquake) for Building and Structures–Special Load sand Load Combinations.
4	IS:456–2000	Code of Practice for Plain and Reinforced Concrete.
5	IS:1904–1986	Code of practice for Design and Construction of foundation in Soils General Requirements.
6	IS:1786–1985	Specification for High Strength Deformed Steel bars and Wires for concreter reinforcement
7	IS: 1893(PART1) 2016	Criteria for Earthquake Resistant Design of Structures
8	IS:2502	Code of practice for the bending and fixing of bars for concrete reinforcement
9	SP 34	Handbook on concrete reinforcement and detailing guidelines

2.4 CONSTRUCTION MATERIAL:-

Concrete shall comply with IS 456-2000. Unless noted otherwise concrete is to be normal-weight, with a typical dry density of 25kN/m^3.

The Grade of Concrete is M25 adopted for all structural Members.

The Grade of steel is Fe550 for design.

SECTION - III

LOADINGS

3.0 LOADING:-

The structure is designed for the all floors as indicated in the approved architectural drawing. The building is analyzed for the following basic load cases:-

1. Dead Load
2. Live Load
3. Seismic load

The brief description of each basic load is as follows.

3.1 DEAD LOAD (DL):-

The dead load comprises of Self Weight of the structure and loading due to finishes and floorings etc., which are permanent in nature. The dead load of Self Weight, partitions, ceiling, flooring and façade etc., are applied as either area loads to slabs or line loads to beams.

The following Unit Weight of material Considered for Dead load Calculation asper IS 875 (Part):1987

Finishes including services load = 1.0kN/m2

Density of concrete = 25kN/m3

Density of brick = 20kN/m3

Density of water = 10 KN /m3

DEAD LOAD CALCULATIONS:-

1. Self-weight of All Structural Member Calculated by Analysis software
2. Grade beam wall load (UDL) = 3KN/m
3. Main wall load (9") =12KN/m
4. Partition wall load =6 KN/m
5. Parapet wall = 5.75 KN/m
6. Lift Duct Side wall = 11.75 KN/m
7. UDL reaction for Mid Landing Beam = 22KN/m
8. For slab (including floor finish) = 6 KN/m2
9. Dead load water tank = 15.25 KN/m2

3.2 LIVE (IMPOSED) LOAD (LL) :-

Live Loads considered on floor slabs are as per IS 875 (Part 2) – 1987.

In general, the following live loads will be considered.

Staircase = 3.0 kN/m2

Balconies &Passage =3.0 KN /m2

Room and Kitchen = 2.0 kN/m2

Utility = 2.0kN/m2

Toilets and bathrooms = 2.0 kN/m2

3.3 SEISMIC LOAD:-

Seismic loads considered on the structure are as per IS 1893 (Part1)-2016.

As per IS 1893 (Part-1) Annex –E Salem lies in seismic zone III of India. The following parameters are considered for Seismic Weight Calculation:-

Zone factor (Z) for zone III = 0.16

Importance factor (I) = 1.0

Response reduction factor (R)=3.00 [For OMRF]

Percentage of imposed load considered in Calculation of Seismic Weight Calculation

as Per IS 1893- 2016 , Table 10

Up to 3 KN /m2 =25%

Above 3.0Kn/m2= 50%

No imposed Load considered on Terrace roof as per clause 7.3.2 of IS 1893-2016

SECTION - IV

DESIGN

4.0 DESIGN OF STRUCTURAL ELEMENTS:-

4.1 DESIGN CONCEPTS:-

The Reinforced cement concrete (RCC) is adopted for the all Structural members. All RCC members are designed by LIMITE STATE METHOD as Mentioned in SECTION -5 of I.S 456:2000

4.2 ANALYSIS:-

The Structural analysis carried out by using STAAD PRO 2024 software and Element design carried out by Bentley RCDC Software. The analysis results are used for the design of columns, footings and beams.

4.3 LOAD COMBINATIONS:-

The load combinations for LIMIT STATE OF COLLAPSE

1. 1.5 (DL+LL)
2. 1.2(DL +LL+/-EL)
3. 1.5(DL+/-EL)
4. 0.9 DL+/-1.5 EL

Individual members of the frame shall be designed for the worst combination of forces such as bending moment, axial force, shear force, torsion, etc.

4.4 FOUNDATION DESIGN

The load combinations for LIMIT STATE OF SRVICEABLITY

1. 1.0 (DL+LL)
2. 1.0DL + 0.8 LL+/- 0.8 EL)

Design procedure of Footing carried out as per Clause 34.1 .I.S -456:2000.

Footing axial load adopted from analysis for Limit States of Serviceability load combination.

TYPICAL FOOTING DESIGN

DESIGN OF ISOLATED FLAT FOOTING

Project	KRISHNAAS ELITE APARTMENT	Date	
Detail		Designer	acs

Column Id		Footing ID		F4			
fck	25	N/mm2		fy	550	N/mm2	
Axial Load, P	1000	kN		Factored Load	1500	kN	
Footing Wt	100	kN					
SBC	350.00	KN/m2					
Req. Footing (Total Load/SBC)	3.14	m2	BM	Footing provided	3.35	m2	OK
Column Width-X-X	0.23	m	OFFSET	Column Depth-Y-Y	0.38	m	
Pedestal Width-X-X	0.380	m	0.725	Pedestal Depth-Y-Y	0.53	m	0.65
Applied Moment, Mx	0	kN-m		Applied Moment, My	0	kN-m	
Factored Mux	0	kN-m		Factored Muy	0	kN-m	
Zxx	1.02	m3		Zyy	1.02	m3	
Max. Pressure	298.61	kN/m2	Footing size OK				
Min. Pressure	298.61	KN/m2	Footing Size OK				
Design F.B. M. My	215.42	kN-m		Design F.B. M. Mx	173.16	kN-m	
Footing Details							
Width-X-X	1.83	m		Length-Y-Y	1.83	m	
1.37							
ALONG LENGTH-X-X DIRECTION				PARLLEL TO WIDTH-Y-Y DIRECTION			
EffectiveCover	55	mm		EffectiveCover	65	mm	
dreq	213.50	mm		dreq	191.41	mm	
Dprov	530	mm		Dprov	530	mm	
dprov	475	mm	OK	dprov	465	mm	OK
Steel Reinforcement							
Ast -Calculated	978.55	mm2		Ast-calculated	799.25	mm2	
Astmin	1043.1	mm2		Astmin	1021.14	mm2	
Astreq	1043.10	mm2		Astreq	1021.14	mm2	
Dia. Of Bar	10	mm		Dia. Of Bar	10	mm	
Aϕ	78.540	mm2		Aϕ	78.54	mm2	
No. of Bars	14			No. of Bars	14		
Astp	1099.6	mm2	OK	Astp	1099.6	mm2	OK
Spacing	133	mm		Spacing	133	mm	
Check for One-way Shear							
Along Length				Along Width			

τcmax-Table 20	2.8	N/mm2		τcmax-Table 20	2.8	N/mm2	
F.Shear Force Vu	204.918	kN		F.Shear Force Vu	151.64	kN	
pt	0.13	%		pt	0.13	%	
shear strength concrete - Tc	0.28	N/mm2		shear strength concrete - Tc	0.28	N/mm2	
Nominal shear stress Tv	0.236	N/mm2		Nominal shear stress Tv	0.18	N/mm2	
One Way shear check : Tv sholud less than Tc			OK	One Way shear check : Tv sholud less than Tc			OK
Check for Two-way Shear				Check for Bearing			
τ'uc	1.1180	N/mm2		Act. Bearing Stress	11.17	N/mm2	
ks	1.0000			A1	3348900	mm2	
τ2	1.1180	N/mm2		A2	0.2014	mm2	
Resisting Perimeter	3720	mm		SQRT(A1/A2)	2		
Resisting Area	1767000	mm2		Per. Bearing Stress	18	N/mm2	OK
Vuc2	1975.57	kN					
Two way shear, Vu2	1387.05	kN	OK				

DESIGN OF COMBINE FOOTING

REF.NO	DESIGN CALCULATION:							
CF1	LOAD ON COLUMN -A	C1	W1 =	1875	Kn	FCK	25	MPA
	LOAD ON COLUMN -B	C2	W2 =	1875	Kn			
	SELF WT. OF FOOTING			375	Kn			
	TOTAL DESIGN LOAD ON FOOTING(P)=			4125	Kn			
	S.B.C OF SOIL =	350	KN/M2					
	REQ.AREA OF FOOTING =		P/S.B.C	11.79	m2	AREA OK		
	FOOTING SIZE PROV.		L =	4.42	m	B =	2.75	M
	AREA OF FOOTING PROVIDED =			12.155	m2			
		AREA PROV >		AREA REQ.				
	CENTER TO CENTER DIST. BET.COLUMN(b) =				2.48	m		
	GEOMETRIC CENTER ,G.C =b/2				1.24	m		
	(X)DIST. OF C.G OF COLM.FROM CENTER OF FOOTING(X)=W2Xb/(W1+W2)=							
	C.G FROM 'A'		X	=	1.24	M		
	PROJ.a1 FROM CENTER OF COLUMN (C2)=							
	a1 =(L/2-X) =				0.97	M		
	PROJ.a2 FROM CENTER OF COLUMN (C1)=							
	a2=(L-a1-b) =				0.97	M		
	NET UPWARD PRESSURE(QNET)=(W1+W2)/(A PROV)=							
					308.52	KN/M2		
	MINI.ECCEN.Emin =.02M		0		M			
	MOMENT DUE TO Emini =		0		KN-M			
	AVAILABLE ECCEN, =(b/2-x)=		0		M			
	MOMENT DUE TO AVAILABLE ECCE.=			0	KN-M			
	CONSIDERING FULL WIDTH OF FOOTING							
	BM CALCULATION:							
	Q-Net Upward Pressure	308.515	KN/M2					
	MAX PRESSURE =QNET+6XM/BXLXL			308.52	KN/M2			
	MINI PRESSURE = QNET-6XM/BXLXL			308.52	KN/M2			
	UDL/ Meter Length			848.42	KN/M			
	SHEAR FORCE V1=qxa1=			822.96	KN			
	V2 =W1-V1			1052.04	KN			

	V3 =qxa2 =		822.96	KN	
	V4=W2- V2=		822.96	KN	

POSITION OF ZERO SHEAR (Y) FROM 'A' =(W1-q*a1)/q

Y = 1.24 M

BENDING MOMENT CALCULATION:

					F.BM	
LHS	M1 =	q*(a1^2)/2	KN-M	399.137	KN-M	598.706165 KN-M
RHS	M2 =	q*a2^2/2	KN-M	399.137	KN-M	598.706165 KN-M
AT CENTER	M3 =	q*(y+a1)2/2-w1*y		-253.125	-379.6875	379.69

MAX. SAGGING BEND.MOM 399.13744 KN-M

EFFECTIVE DEPTH FOR MAX BENDING MOMENT

fck 25 fy 550

d req #DIV/0! mm

D Provided 685 Cover 50 Dia 20

d Provided 625 mm

AREA OF REINFORCEMENT

		Ast Calculated	Ast Min	FINAL Ast	% Ast	LOCATION
LHS	Ast1	2057.110	2062.5	2062.500	0.120	BOTTOM
RHS	Ast2	2057.110	2062.5	2062.500	0.120	BOTTOM
AT MAX	Ast3	1291.580	2062.5	2062.500	0.120	TOP

PUNCHING SHEAR CALCULATIONS

P1	1875					
PDESTAL SIZE		C1	0.75	0.45	ENTER	
PUNCHING AREA		1.48	1.375	1.075	qo	308.52 K
PUNCHING SHEAR FORCE-V			1418.98			V= P- qo* Pun
		VU	2128.46			
PERIMETER		b'	4.9	m	b'	4900
tv= Vu/b,*d		tv	0.69501	N/mm2		

check tv should not exceed k'tc

	ks	0.5+Bc	1.10	ks 1
	Bc	0.60		
tc=0.25SRT(fck)			1.250	N/mm2

PRUNCHING SHEAR		RESULT		OK	
TRANSVERSE REINFORCEMENT DETAILS					
UNDER COLUMN -C1	P1		1875	KN	d' in m
Cantilever Projection of Slab			1.15	m	0.625 m
Band width= c.length +2d			2	m	
Net upward soil pressure for qo=P1/B*B.W		band width=	340.909	KN/M2	
Bending Moment to face of Column			225.426	KN-M	
		FBM	338.139	KN-M	
Band width		Ast	1051.1	mm2	
Mini Ast 0.12* band width			1500	mm2	
FINAL TRANSVER AST @C1			1500	mm2	
UNDER COLUMN -C2	P2		1875	KN	d' in m
C2		LENGTH	0.75	0.45	0.625
Cantilever Projection of Slab			1.15	m	
Band width= c.length +2d			2.00	m	
Net upward soil pressure for qo=P1/B*B.W		band width=	340.909	KN/M2	
Bending Moment to face of Column			225.426	KN-M	
		FBM	338.139	KN-M	
Band width		Ast	1587.71	mm2	
Mini Ast 0.12* band width			1500	mm2	
FINAL TRANSVER AST@C2			1587.71	mm2	

4.5 COLUMN DESIGN

The columns are designed directly STAAD PRO 2024- RCDC for the worst load combination and Reinforcement detailed as per SP: 34 & IS 13920-2016

TYPICAL COLUMN DESIGN CALCULATIONS

GENERAL DATA

Column No.	:	C22	
Level	:	1.5 m To 2.5 m	
Frame Type	=	Non-Ductile	
Response Reduction Factor	=	3	
Design Code	=	IS 456 : 2000 + IS 13920 : 2016	
Grade Of Concrete	=	M25	N/sqmm
Grade Of Steel (Main)	=	Fe550	N/sqmm
Grade Of Steel (Shear)	=	Fe550	N/sqmm
Grade Of Steel - Flexural Design	=	Fe550	N/sqmm
Grade Of Steel - Shear Design	=	Fe550	N/sqmm
Column B	=	300	mm
Column D	=	600	mm
Clear Cover	=	40	mm
Clear Floor Height @ B	=	2700	mm
Clear Floor Height @ D	=	2770	mm

No Of Floors	=	1
No Of Columns In Group	=	1
Column Type	:	Unbraced
Minimum eccentricity check	:	One Axis at a Time
Code defined D/B ratio	:	4
D/B Ratio	:	2 <= 4 Hence, Design as Column
No Of Floors	=	1
No Of Columns In Group	=	1
Column Type	:	Unbraced
Minimum eccentricity check	:	One Axis at a Time
Code defined D/B ratio	:	4
D/B Ratio	:	2 <= 4 Hence, Design as Column

CALCULATION DESIGN MOMENT

Direction	M analysis	Mmin (Abs)		Mdesign	MsIndx (Abs)	Mdesign-final
	A	B		C	E	F
Major Axis - Mux	0.26	-		0.26	0	0.26
Minor Axis - Muy	-4.67	50.77		-50.77	0	-50.77

Where

A = Moments directly from analysis

B = Moments due to minimum eccentricity

C = Maximum of analysis moment and min. eccentricity = Max (A,B)

E = Moment due to slenderness effect

F = Final design Moment = Max(C- Top Bottom , D- Top Bottom) + E

FINAL CRITICAL DESIGN FORCES

Pu	=	2538.66	kN
Mux	=	0.26	kNm
Muy	=	-50.77	kNm

RESULTANT MOMENT (Combined Action)

Moment Capacity Check

Pt Calculated	=	1.59	
Reinforcement Provided	=	4-T20 + 8-T16	
Load Angle	=	Tan^{-1}(Muy/Mux)	
	=	89.7	deg
MRes	=	50.77	kNm
MCap	=	70.86	kNm
Capacity Ratio	=	MRes/ MCap	

Shear Calculation	Along D	Along B
Critical Analysis Load Combination	4	4
Critical Load Combination	[2] : 1.5 (LOAD 1: DEAD LOAD) +1.5 (LOAD 2: LIVE LOAD)	[2] : 1.5 (LOAD 1: DEAD LOAD) +1.5 (LOAD 2: LIVE LOAD)
Design shear force, Vu (kN)	-0.8907	1.3158
Pu (kN)	2518.4	2518.4
Deff , Beff (mm)	550	250
Aeff (sqmm)	165000	150000
Design shear stress, Tv	Vu / (B x Deff)	Vu / (D x Beff)
Tv (N/sqmm)	0.0054	0.0088
Pt (%)	0.796	0.796

Design shear strength, Tc (N/sqmm)	0.5882	0.5882
Shear Strength Enhancement Factor (Compressive Force)	1 + 3 x Pu / (B x D x Fck)	
	2.8316	3.0147
Shear Strength Enhancement Factor (Tensile Force)	-	-
Shear Strength Enhancement Factor (max)	1.5	1.5
Shear Strength Enhancement Factor	1.5	1.5
Enhanced shear strength, Tc-e (N/sqmm)	0.8822	0.8822
Design shear check	Tv < Tc x Enhancement factor	Tv < Tc x Enhancement factor
Shear Links Design	**Shear Links Not Required**	**Shear Links Not Required**

DESIGN OF LINKS

Links in the zone where special confining links are not required

Normal Links

Diameter of link	= 8	mm
	> Max. longitudinal bar dia / 4	
	= 5	mm

Criterion for spacing of normal links

Min. Longitudinal Bar dia X 16	= 256	mm
Min. dimension of column	= 300	mm
Max. 300 mm	= 300	mm
Provided spacing	= 250	Mm

4.6. BEAM DESIGN

Beam at various location viz Grade beam, mid landing beam for staircase, Roof level beam are designed as Continuous doubly reinforced section using **STAAD PRO 2024- RCDC** Software for the load combination and Reinforcement detailed as per **SP: 34.**

DESIGN OF TYPICAL ROOF BEAM DESIGN CALCULATIONS

Group	:	**G5**	
Beam No	:	**B10**	
Analysis Reference (Member)	:	681 (6.15 m)	
Beam Length	:	1750	mm
Breadth (B)	:	**230**	mm
Depth (D)	:	**600**	mm
Effective Depth (d)	:	545	mm
Design Code	:	IS 456	
Beam Type	:	Regular Beam	
Grade Of Concrete (Fck)	:	**M25**	N/sqmm
Grade Of Steel (Main)	:	**Fe550**	N/sqmm
Grade Of Steel (Shear)	:	**Fe550**	N/sqmm
Top/Bottom Clear Cover (Cmin)	:	25	mm
Side Clear Cover	:	25	mm
Es	:	2x10^5	N/sqmm
Ptnominal (%) (Bn)			

Flexure Design

	Beam Bottom			Beam Top		
	Left	**Mid**	**Right**	**Left**	**Mid**	**Right**
Critical L/C - Analysis	4	-	-	4	4	4
Critical L/C - RCDC	2	-	-	2	2	2
Mu (kNm)	10.95	-	-	51.41	182.8	251.82
Tu (kNm)	0	0	0	0	0	0
M_{Tu} (kNm)	0	0	0	0	0	0
Mud (kNm)	10.95	0	0	51.41	182.8	251.82
MuLim (kNm)	222	222	222	222	222	222
R	0.16	0	0	0.753	2.676	3.686
Ptmin (%) (A)	0.2	0.2	0.2	0.2	0.2	0.2
Ptclc (%) (B')	0.2	0.2	0.2	0.2	0.654	0.934
Pcclc (%) (C)	0	0	0	0	0	0.111
PtPrv (%)	0.271	0.271	0.271	0.271	0.752	0.962
AstCalc (sqmm)	250.7	250.7	250.7	250.7	819.38	1171.08

AstPrv (sqmm)	339.3	339.3	339.3	339.3	942.48	1206.36
Reinforcement Provided	3-T12	3-T12	3-T12	3-T12		

Note: Calculation of Ast

AstCalc = Max {B',C} x B X d (for Mu > 0)

AstCalc = Bn x B X d (for Mu = 0)

Where,

A	= Ptmin	=	Min % area of flexural reinforcement
B'	= Ptclc	=	Maximum of % reinforcement required for Bending Moment and Min % area of flexural reinforcement (A)
Bn	= Ptnominal	=	Nominal % area of reinforcement
C	= Pcclc	=	% area of compression reinforcement required
D	= Ast Crack	=	Area of reinforcement required for Crack width

Shear Design			
	Left	Mid	Right
Critical L/C - Analysis	4	4	4
Critical L/C - RCDC	2	2	2
PtPrv (%)	0.271	0.752	0.752
Vu (kN)	144.48	154.5	154.5
Tu (kNm)	0	0	0
V_{Tu} (kN)	0	0	0
Vut (kN)	144.48	154.5	154.5
Tv (N/sqmm)	1.15	1.23	1.23
Tc (N/sqmm)	0.38	0.58	0.58
Vc (kN)	47.32	72.12	72.12
Vus=Vut-Vc (kN)	97.16	82.38	82.38
Legs	2	2	2
Stirrup Rebar	8	8	8
Asv Torsion (sqmm)	-	-	-
Asv Torsion Prv (sqmm)	-	-	-
Asv Reqd (sqmm/m)	493.74	418.67	418.67
SvCalc (mm)	200	240	240
SvPrv (mm)	200	240	240
Asv Total Prv (sqmm)	502.7	418.92	418.92

SFR Design

Beam Width	= 230	mm
Beam Depth	= 600	mm
Web Depth	= 600 <= 750	mm

Side Face Reinforcement not required.

4.7. TYPICAL SLAB DESIGN

Slabs are designed as one way & two way based on Length /breadth ratio Criteria

L/B > 2 = ONEWAY SLAB

L/B< 2 = TWO WAY SLAB

Detailing of Reinforcement carried out as IS: SP34

DESIGN OF TWO WAY SLAB

Here the aspect ratio Ly/Lx is less than 2 hence the slab is designed as a two way slab.

Two Adjacent Edges Discontinuous:

Basic dimensions of slab = Lx Ly

Lx=3.58 Ly= 5.56

Basic Ly/Lx ratio = 1.553 <2

Hence designed as two way slab

Clear cover to reinforcement d' = 20 mm

Provided overall depth D = 125.00 mm

Effective depth d = 100.00 mm

Diameter of bar = 10 mm

Select Grade of Concrete fck = 25 N/mm²

Select Grade of Steel fy = 550 N/mm²

LOAD CALCULATION:-

Dead load of the slab DL = 3.750 kN/m²

Floor finish (Roof finish) FF = 1.5 kN/m²

Live load LL = 2 kN/m²

Total load TL = 7.250 kN/m²

MOMENT & AREA OF STEEL CALCULATIONS

Span	Moment Coefficient		Mu kN.m	Mu/bd² N/mm²	Pt %	Ast reqd mm²	Min Ast mm²	Dia of bar mm	Spacing mm	Ast pro mm²	
shorter	$\alpha_{x\ neg}$	0.080	11.16	1.12	0.27%	271.35	120	10	150	523.60	SAFE
	$\alpha_{x\ pos}$	0.060	8.37	0.84	0.20%	200.51	120	10	150	523.60	SAFE
longer	$\alpha_{y\ neg}$	0.045	0.04	0.00	0.00%	0.85	120	10	150	523.60	SAFE
	$\alpha_{y\ pos}$	0.034	4.74	0.47	0.11%	111.42	120	10	150	523.60	SAFE

CHECK FOR DEFLECTION

The effective depth provided	?	100.000	mm
From figure 3 of I.S 456:1978 modification factor is			
Modification factor	?	1.26	
Required depth under deflection consideration	?	92.41	mm
			HENCE SAFE

4.8 DESIGN OF STAIRCASE

Staircase Designed as One way Slab and Spanning to One Direction

	LOAD CALCULATION					
1	**STAIR PORTION**				KN/M^3	
a	self weight of slab		0.177	25	4.425	
b	Floor Finish				1	
c	hand rail				0.25	
	Total load				**5.675**	
	Riser	6	inch	0.15	m	
	Tread	12	inch	0.30	m	
(i)	Equivalent Horizontal Slab Dead load				6.34	
(ii)	Weight of steps		(1/2*T*R*25)/T		1.9050	
(iii)	Live Load				3	
		Total load on Stair Portion			11.25	Kn/m2
2	**LOAD ON LANDING SLAB**					
(i)	self weight				4.425	
(ii)	Floor Finish				1.5	
(iii)	live load				3	
		Total load on Landing slab			8.925	Kn/m2

DRAWING TITLE :			STAIRCASE DESIGN
FLIGHT			GROUND TO MID LANDING
			STAIR
LIVE LOAD	3	KN/M2	11.25 KN/M
Concrete- M	25	N/MM2	A 2.93 B
Steel Fe	550	N/MM2	4.45

SPANS (M)			DEPTHS (MM)		LOADING (KN/M2)	
TOTAL SPAN LENGTH-A-C	SPAN A-B 'm'	SPAN B-C 'm'	D'	Effective -d	AB-STAIR PORTION	BC-LANDING
4.45	2.93	1.52	177	152	11.25	9.00

DESIGN						SPACING
Point of Zero S. F. from A(m)	Max. F.BM kN-m	Mu/bd2	%Pt req.	Req. Ast (mm2)	Dia mm	Prov. mm
2.173	39.84	1.725	0.395	600	12	100

Deflection Check						
f_y	f_s	$p_{t\ req.}$	$p_{t\ prov.}$	MF_t	Constant	Span Ratio
550	153.82	0.395	0.744	1.56	20	29.28	31.20	Safe

4.90 CONCLUSION

From the Analysis & design results satisfies the Codal Requirements for the SHREE KRISHNAA'S

ELITE APARTMENT BUILDING

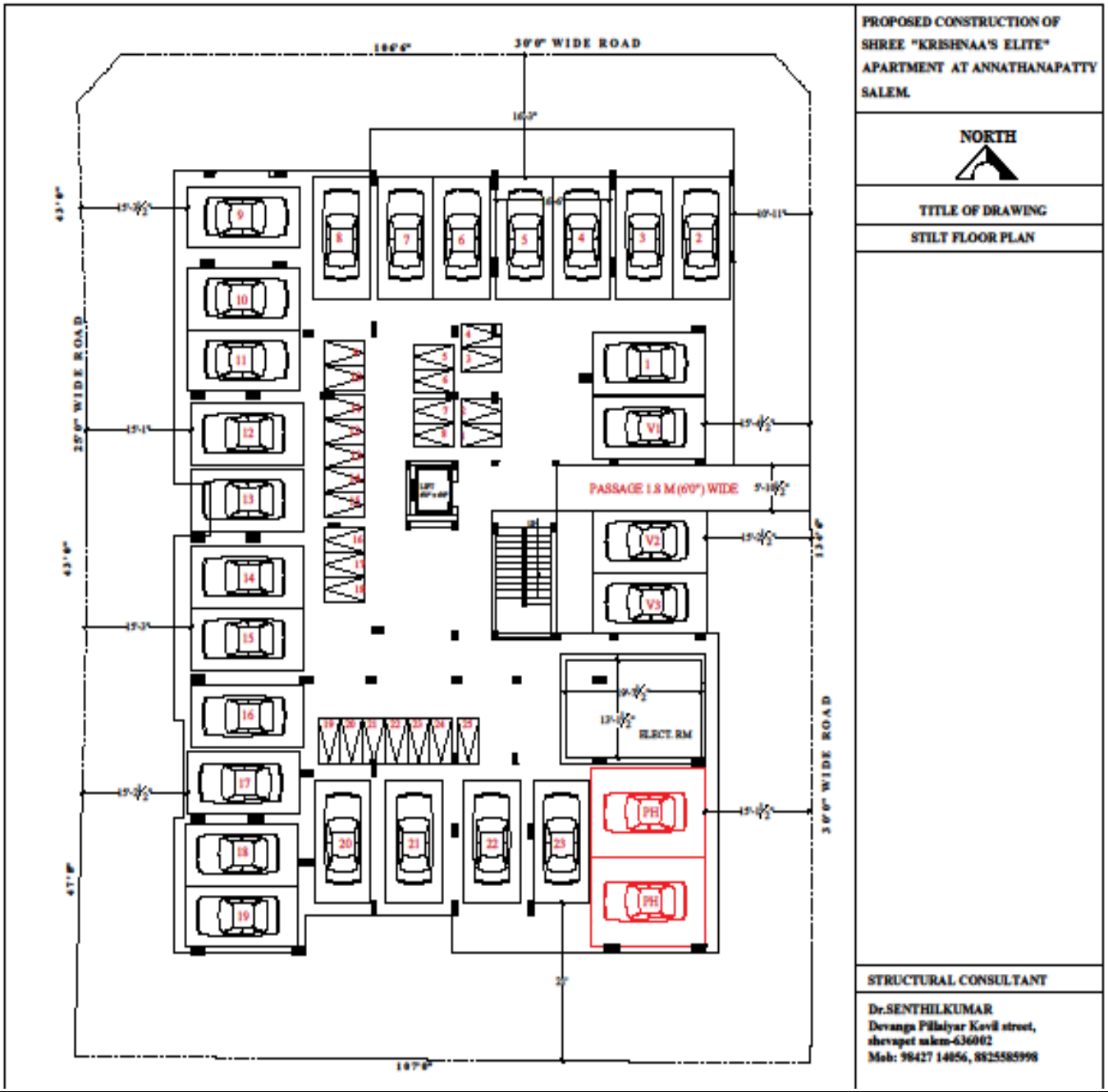
********END OF THE DESIGN REPORT*********

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SECTION - V

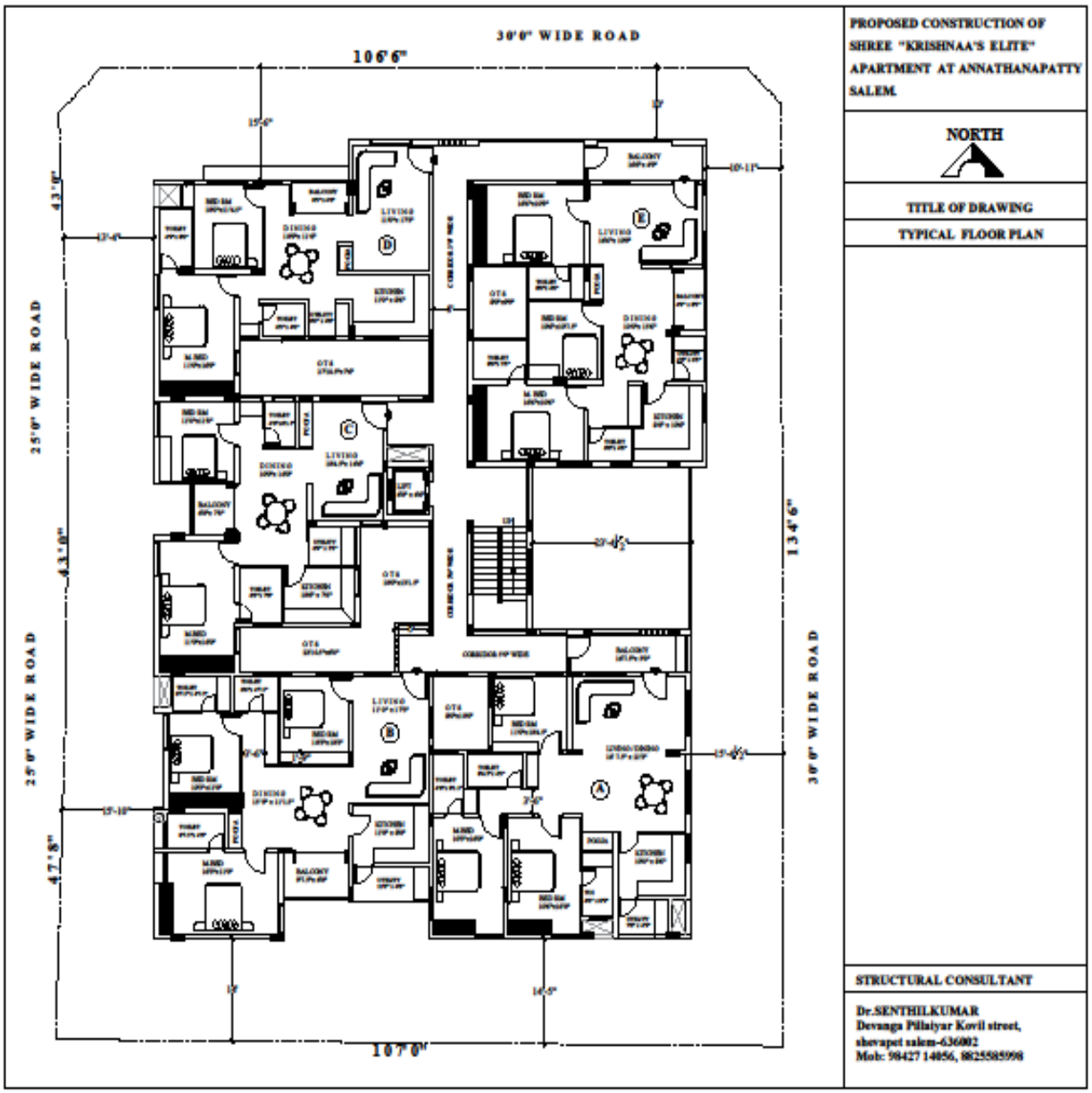
DRAWINGS

STILT FLOOR PLAN



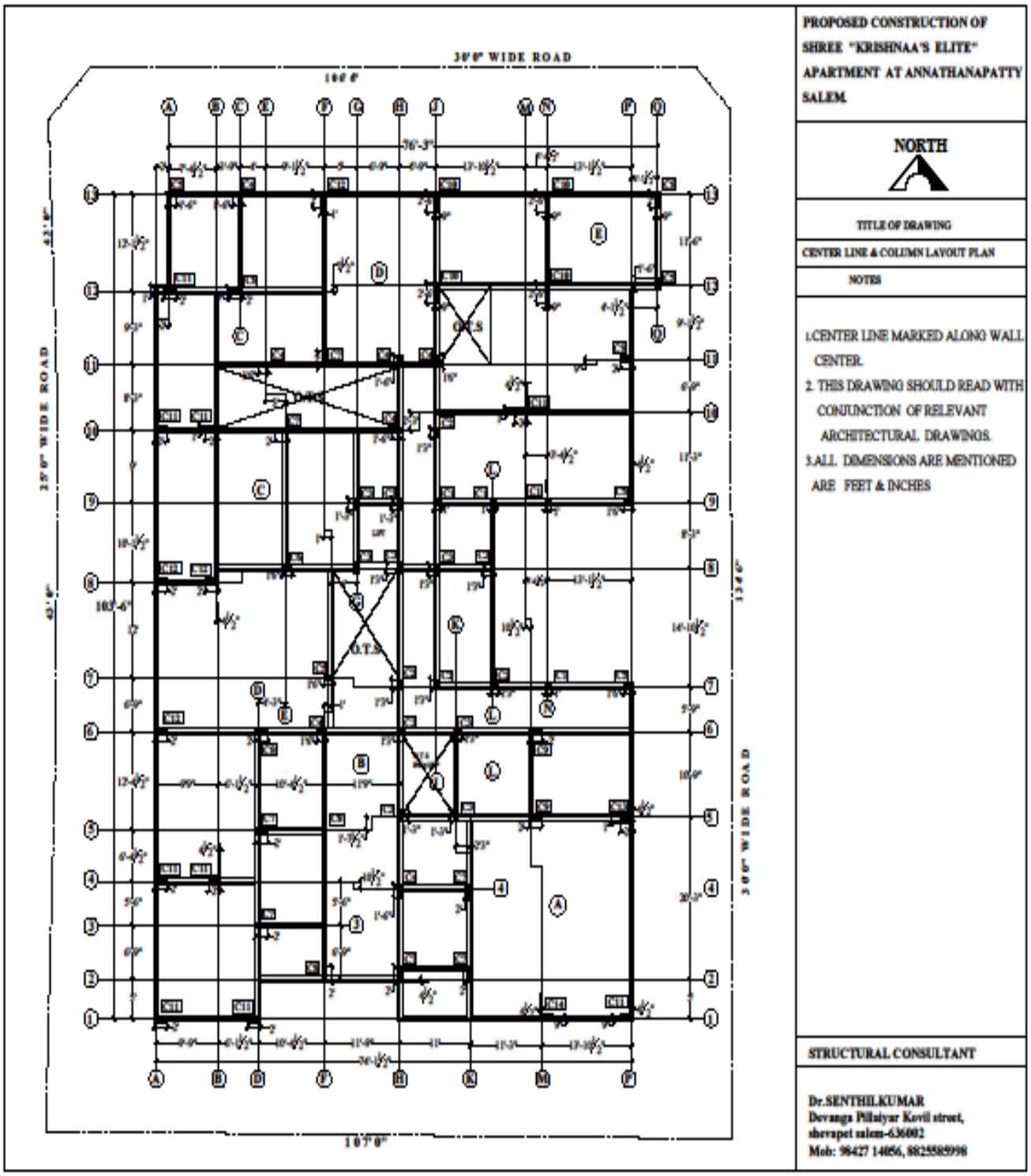
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TYPICAL FLOOR PLAN



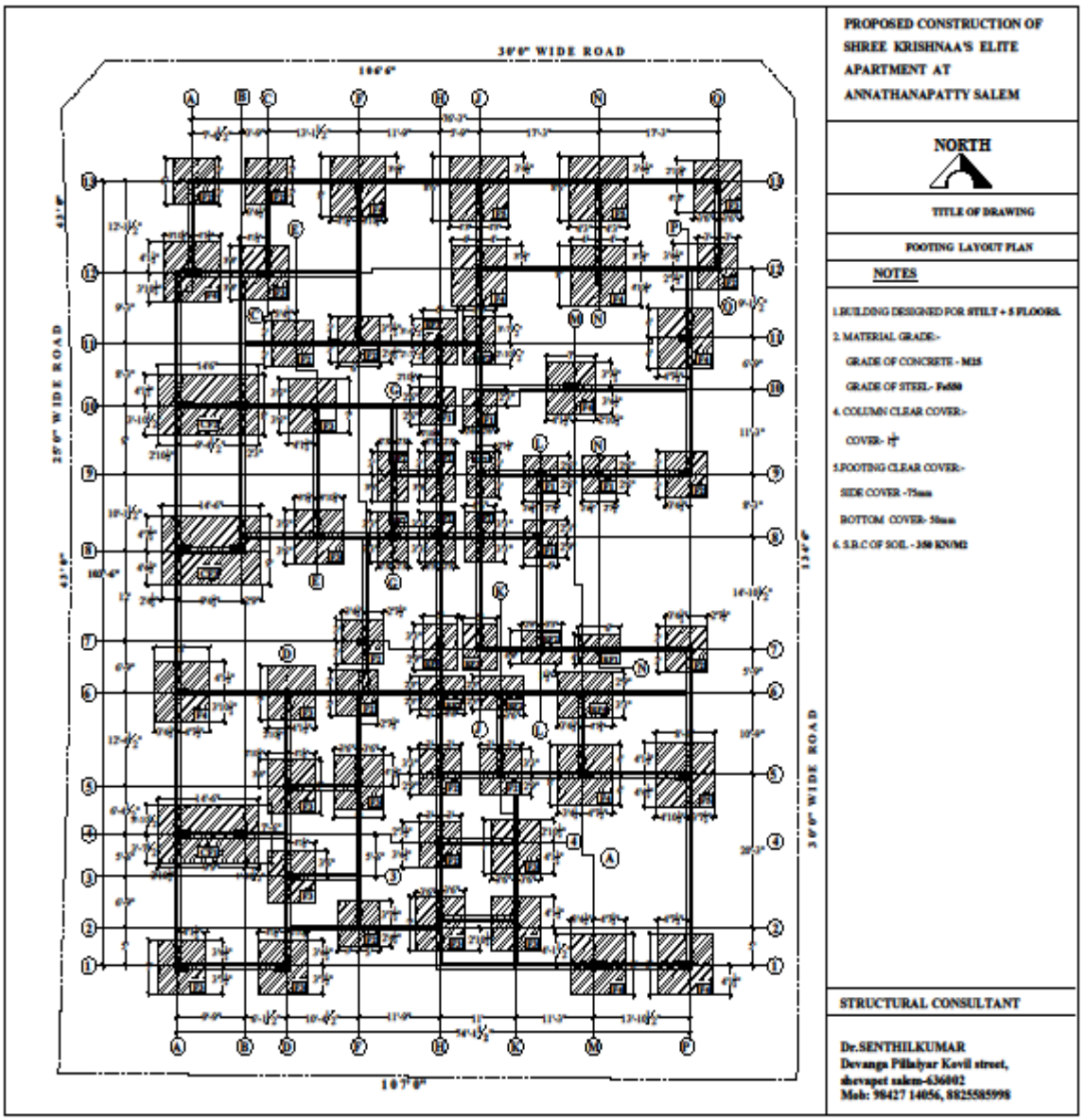
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CENTER LINE PLAN



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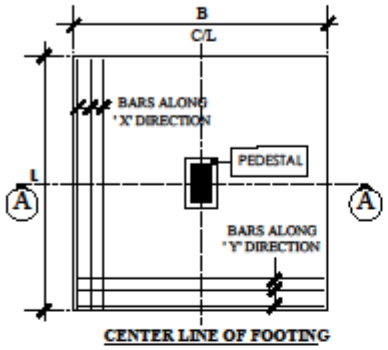
FOOTING LAYOUT PLAN



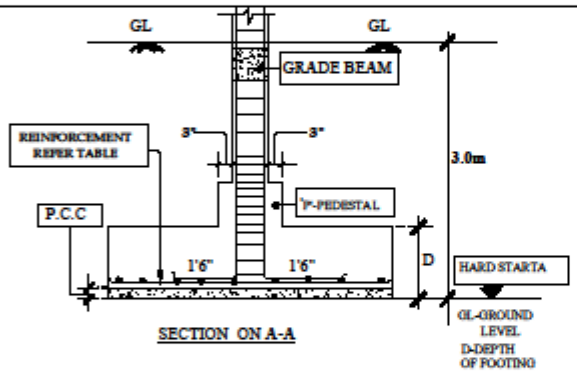
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FOOTING REINFORCEMENT PLAN

TYPICAL FOOTING DETAILS



CENTER LINE OF FOOTING



SECTION ON A-A

ISOLATED FOOTING REINFORCEMENT DETAILS

TYPE	NOS	FOOTING SIZE		DEPTH	PEDESTAL HEIGHT	REINFORCEMENT	
		L	B	D	'P'	BARS ALONG 'X' DIRECTION	BARS ALONG 'Y' DIRECTION
F1	5	5'0"	5'0"	1'3"	1'6"	10 NOS 10#	10 NOS 10#
F2	13	6'0"	6'0"	1'9"	1'6"	14 NOS 10#	14 NOS 10#
F3	13	7'0"	7'0"	2'0"	1'6"	14 NOS 12#	14 NOS 12#
F4	10	8'0"	8'0"	2'3"	2'0"	12 NOS 16#	12 NOS 16#
F5	3	8'6"	8'6"	2'6"	2'0"	12 NOS 16#	12 NOS 16#

PROPOSED CONSTRUCTION OF SHREE KRISHNAA'S ELITE APARTMENT AT ANNATHANAPATTY SALEM

TITLE OF DRAWING

FOOTING REINFORCEMENT DETAILS

NOTES

1.BUILDING DESIGNED FOR **STILT + 5 FLOORS.**

2. MATERIAL GRADE:-

GRADE OF CONCRETE - **M25**

GRADE OF STEEL- **Fe550**

4. COLUMN CLEAR COVER:-

COVER- 1½"

5.FOOTING CLEAR COVER:-

SIDE COVER -75mm

BOTTOM COVER- 50mm

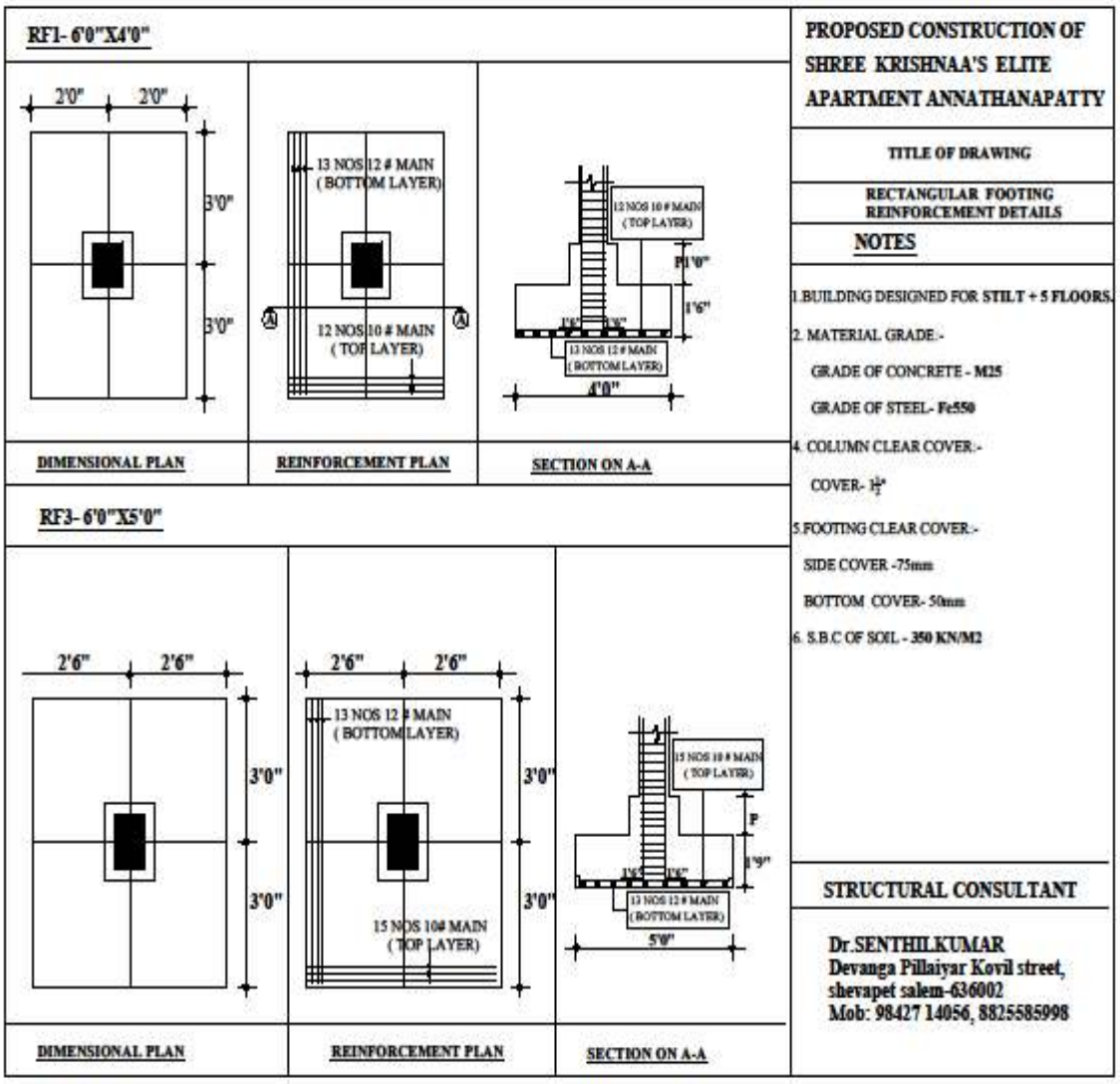
6. S.B.C OF SOIL - **350 KN/M2**

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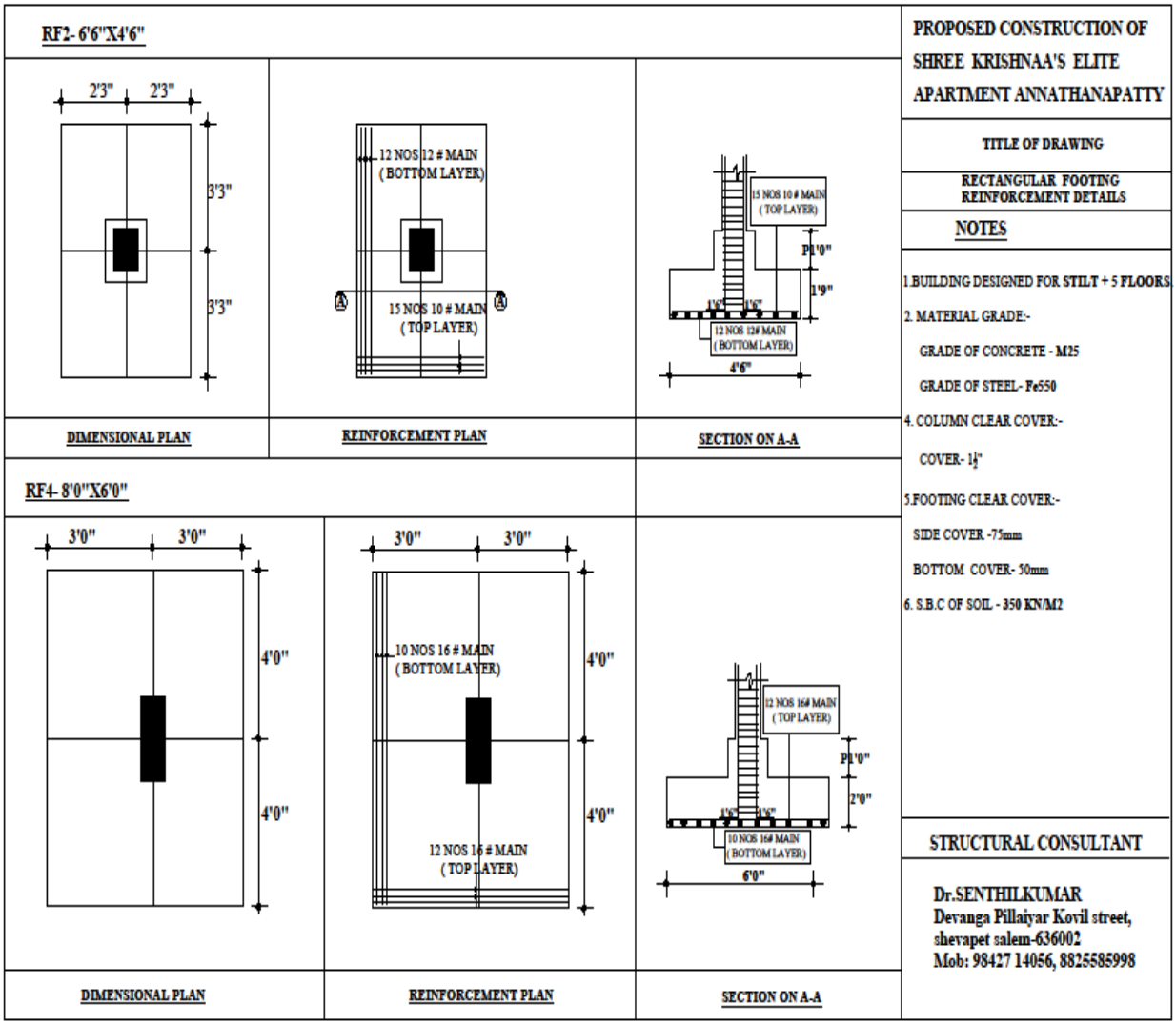
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RECTANGULAR FOOTING



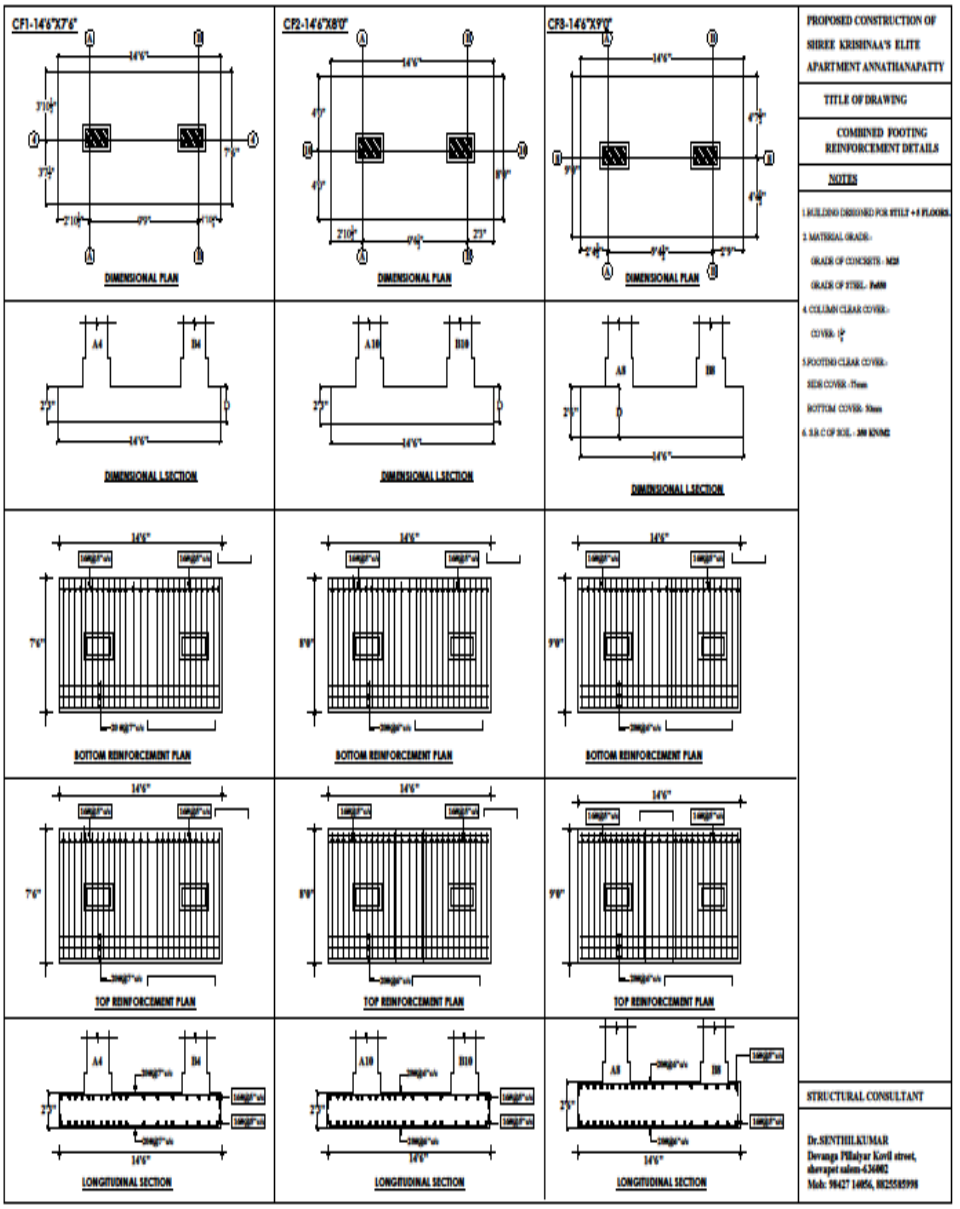
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RECTANGULAR FOOTING



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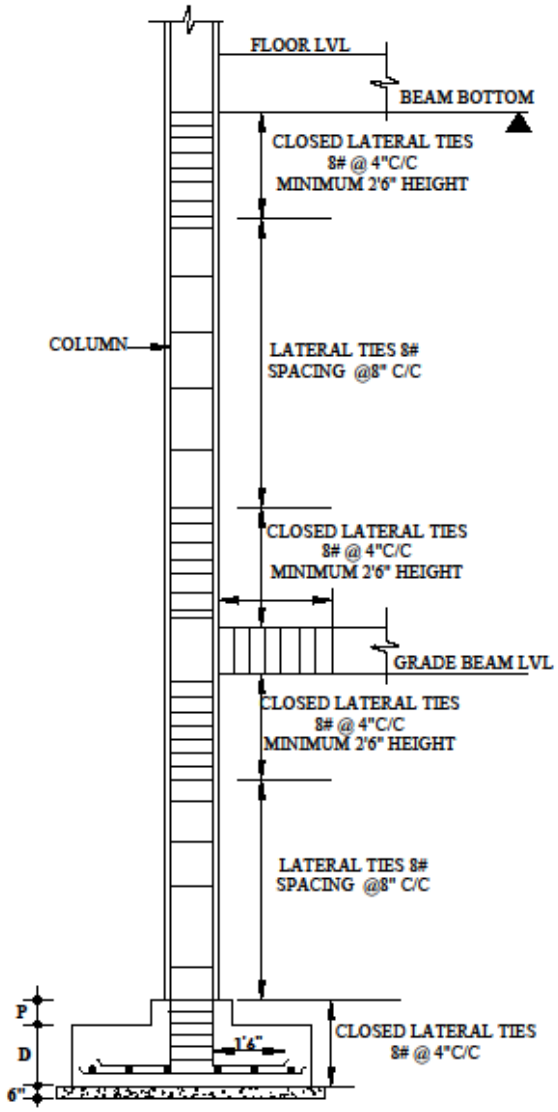
COMBINED FOOTING DETAILS



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LATERAL TIES ARRANGEMENTS

LATERAL TIES ARRANGEMENT DETAILS



SECTION ON COLUMN & FOOTING

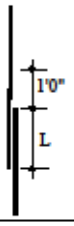
PROPOSED CONSTRUCTION OF SHREE KRISHNAA'S ELITE APARTMENT AT ANNATHANAPATTY SALEM

TITLE OF DRAWING

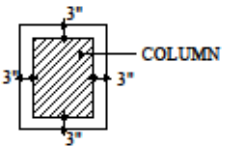
LATERAL TIES ARRANGEMENT DETAILS

NOTES

1.BUILDING DESIGNED FOR **STILT + 5 FLOORS.**
2. MATERIAL GRADE:-
GRADE OF CONCRETE - **M25**
GRADE OF STEEL- **Fe550**
4. COLUMN CLEAR COVER:-
COVER- 1½"
5.COLUMN LAP LENGTH DETAILS
12 mm : **2'0"**
16mm : **2'6"**
20mm : **3'0"**
25mm : **3'9"**
6.CONCRETE MIX DETAILS:-
M25 DESIGN MIX SHOULD BE CARRIED OUT AS PER I.S -10262



COLUMN LAP DETAIL



FOOTING PEDESTAL

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COLUMN REINFORCEMENT PLAN

COLUMN REINFORCEMENT DETAILS

COLUMN TYPE	NO	COLUMN SIZE	COLUMN SECTION	LATERAL TIES
C1	4	9"X1'0"	9" × 1'0", 4-16# + 2-12#	6" × 9", 1, 8# @ 4"/8"C/C
C2	4	9"X1'3"	9" × 1'3", 4-16# + 2-12#	6" × 12", 1, 8# @ 4"/8"C/C
C3	10	9"X1'3"	9" × 1'3", 4-16# + 4-12#	6" × 12", 1, 2, 8# @ 4"/8"C/C
C4	7	9"X1'6"	9" × 1'6", 4-16# + 4-12#	6" × 15", 1, 2, 8# @ 4"/8"C/C
C5	5	9"X1'6"	9" × 1'6", 4-20# + 4-16#	6" × 15", 1, 2, 8# @ 4"/8"C/C
C6	1	9"X1'6"	9" × 1'6", 8-20#	6" × 15", 1, 2, 8# @ 4"/8"C/C
C7	7	9"X2'0"	9" × 2'0", 4-16# +6-12#	6" × 21", 1, 2, 8# @ 4"/8"C/C
C8	5	9"X2'0"	9" × 2'0", 8-20#	6" × 21", 1, 2, 8# @ 4"/8"C/C

PROPOSED CONSTRUCTION OF SHREE KRISHNAA'S ELITE APARTMENT AT ANNATHANAPATTY SALEM

TITLE OF DRAWING

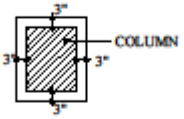
COLUMN REINFORCEMENT DETAILS

NOTES

1.BUILDING DESIGNED FOR STILT + 5 FLOORS.
2. MATERIAL GRADE:-
GRADE OF CONCRETE - M25
GRADE OF STEEL- Fe550
4. COLUMN CLEAR COVER:-
COVER- 1½"
5.COLUMN LAP LENGTH DETAILS
12 mm : 2'0"
16mm : 2'6"
20mm : 3'0"
25mm : 3'9"
6.CONCRETE MIX DETAILS:-
M25 DESIGN MIX SHOULD BE CARRIED OUT AS PER I.S -10262



COLUMN LAP DETAIL



FOOTING PEDESTAL

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COLUMN REINFORCEMENT PLAN

COLUMN REINFORCEMENT DETAILS

COLUMN TYPE	NO	COLUMN SIZE	COLUMN SECTION	LATERAL TIES
C9	3	9"X2'0"	9", 2'0", 10-20#	6", 21", 1, 2, 8# @ 4"/8"C/C
C10	4	9"X2'6"	9", 2'6", 8-25#+4-20#	6", 27", 1, 2, 3, 8# @ 4"/8"C/C
C11	9	1'0"X2'0"	1'0", 2'0", 6-20#+4-16#	9", 21", 1, 2, 8# @ 4"/8"C/C
C12	4	1'0"X2'0"	1'0", 2'0", 10-20#	9", 21", 1, 2, 8# @ 4"/8"C/C
C13	1	1'0"X2'0"	1'0", 2'0", 6-25#+ 4-20#	9", 21", 1, 2, 8# @ 4"/8"C/C
C14	1	1'0"X2'0"	1'0", 2'0", 10-25#+ 2-20#	21", 1, 3, 2, 8# @ 4"/8"C/C

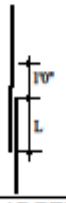
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TITLE OF DRAWING

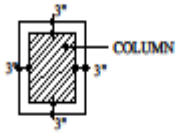
COLUMN REINFORCEMENT DETAILS

NOTES

1.BUILDING DESIGNED FOR STILT + 5 FLOORS.
2. MATERIAL GRADE:-
GRADE OF CONCRETE - M25
GRADE OF STEEL- Fe550
4. COLUMN CLEAR COVER:-
COVER- ½"
5.COLUMN LAP LENGTH DETAILS
12 mm : 2'0"
16mm : 2'6"
20mm : 3'0"
25mm : 3'9"
6.CONCRETE MIX DETAILS:-
M25 DESIGN MIX SHOULD BE CARRIED OUT AS PER I.S -10262



COLUMN LAP DETAIL



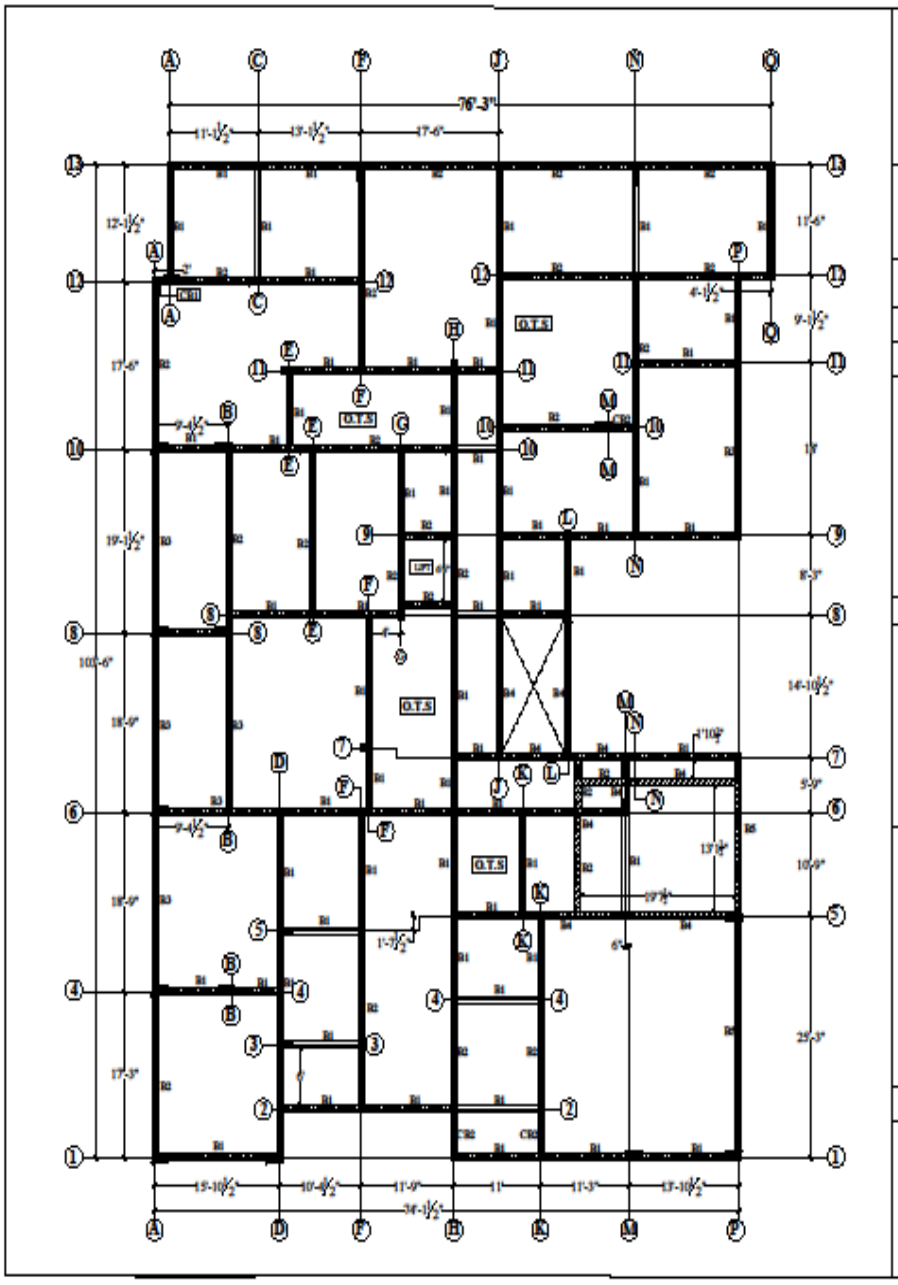
FOOTING PEDESTAL

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GRADE BEAM LAYOUT PLAN



PROPOSED CONSTRUCTION OF SHREE KRISHNAA'S ELITE APARTMENT AT ANNATHANAPATTY,SALEM

NORTH

TITLE OF DRAWING

GRADE BEAM LAYOUT PLAN

NOTES

1.GRADE BEAM DESIGNED FOR 3'0" WALL LOAD
2. MATERIAL GRADE:-
GRADE OF CONCRETE - M25
GRADE OF STEEL- Fe550
4. BEAM CLEAR COVER:-
COVER- 25mm

SCHEDULE OF GRADE BEAM

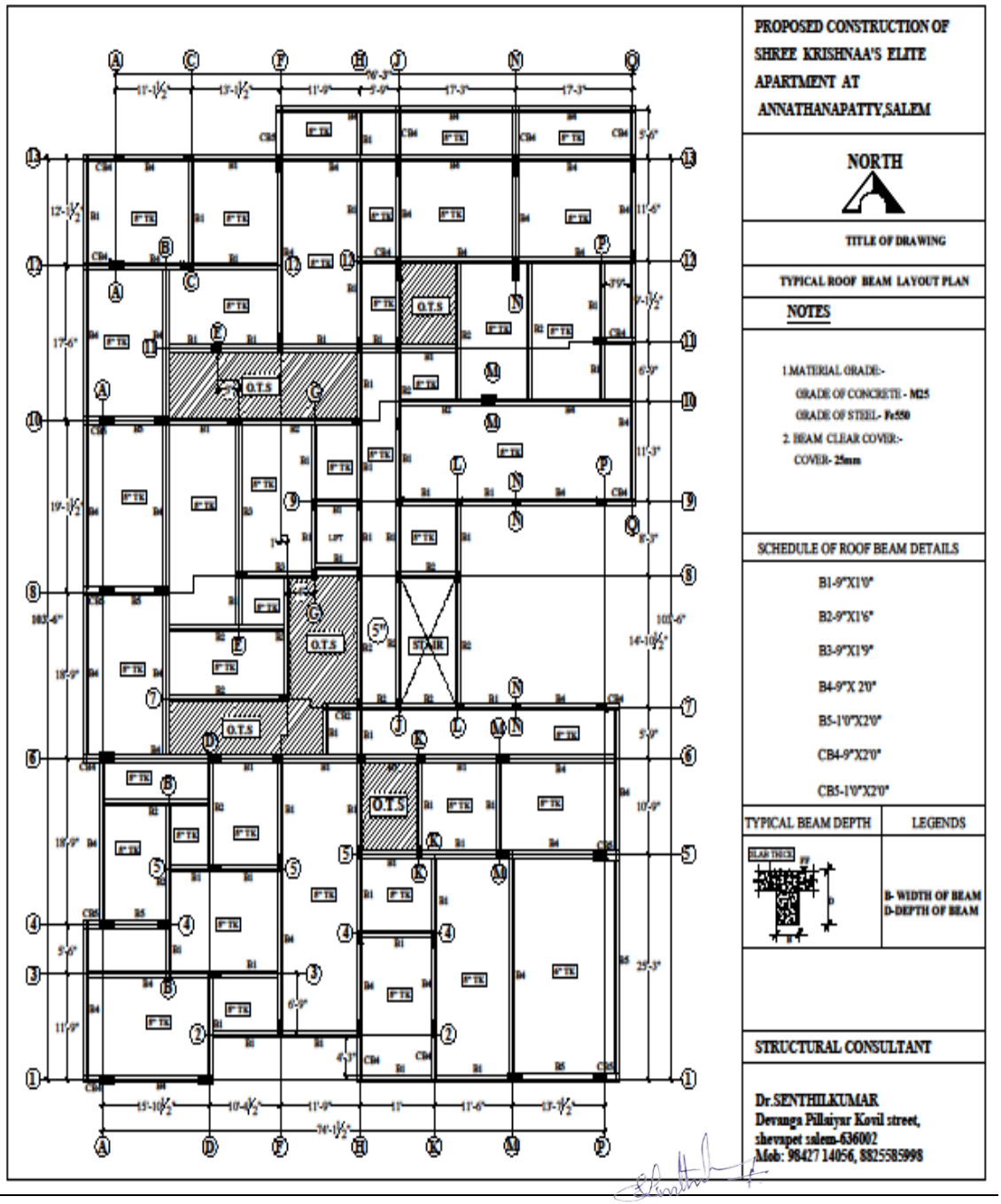
B1-9"X9"
B2-9"X1'0"
B3-9"X1'3"
B4-9"X1'3"
B5-9"X1'6"

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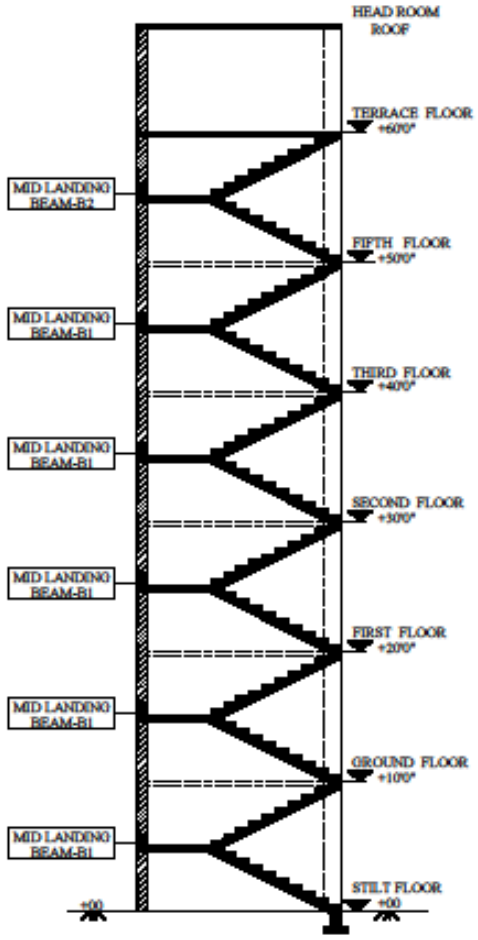
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TYPICAL ROOF BEAM LAYOUT PLAN

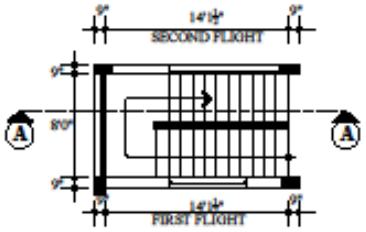


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STAIR CASE GENERAL ARRANGMENT



SECTION ON A-A



STAIR CASE PLAN

PROPOSED CONSTRUCTION OF SHREE KRISHNAA'S ELITE APARTMENT AT ANNATHANAPATTY

TITLE OF DRAWING

GENERAL ARRANGEMENT OF STAIRCASE

NOTES

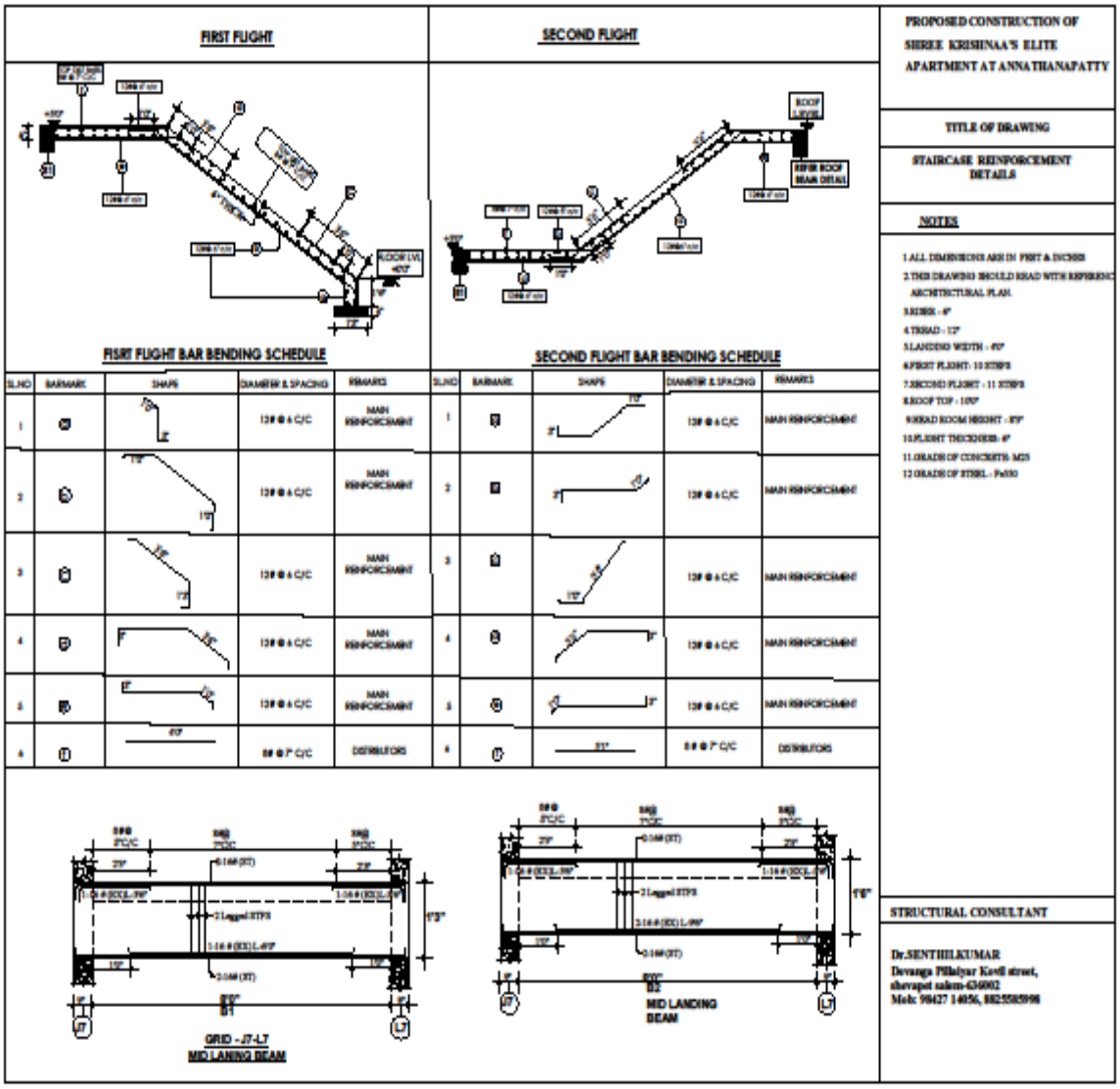
1. ALL DIMENSIONS ARE IN FEET & INCHES
2. THIS DRAWING SHOULD READ WITH REFERENCE ARCHITECTURAL PLAN.
3. RISER - 6"
4. TREAD - 12"
5. LANDING WIDTH - 4'0"
6. FIRST FLIGHT- 10 STEPS
7. SECOND FLIGHT - 11 STEPS
8. ROOF TOP - 10'0"
9. HEAD ROOM HEIGHT - 8'3"
10. FLIGHT THICKNESS- 6"
11. GRADE OF CONCRETE- M25
12. GRADE OF STEEL - Fe550

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STAIRCASE REINFORCEMENT PLAN



THE LANDLENS SUITE

Eight ways to know a piece of land before you pay for it.

This report covers one part of the picture. These cover the rest, drawn from the same government records, for any property in Tamil Nadu.

LandLens One
Ask anything about a property and get an answer grounded in live government records, with the sources shown.
/landlens/one

Verify My Land
Every record check run on one property and returned as a single report.
/landlens/verify-my-land

Legal Opinion
An advocate's written opinion on the title, built on the records we pull for it.
/landlens

One Map
Every layer on one map: the parcel, its boundary, zoning, water bodies and what sits around it.
/landlens/one/map

PriceMap
What land actually changed hands for around this plot, taken from the registered deeds.
/landlens/pricemap

Build Scope
What you are allowed to build here: floor space, height, coverage and setbacks.
/landlens/build-scope

Property Tracker
Watch a property and hear about it when something new is registered against it.
/landlens/property-tracker

Deal Room
Run the transaction in one place: documents, stamping, signatures and the parties.
/landlens/deal-room

FREE TOOLS · NO ACCOUNT NEEDED

Twenty-four checks you can run yourself, at no cost.

Records and verification

The registers themselves, read for you.

- Encumbrance Certificate
- Patta and Chitta
- Field Measurement Sketch
- EC Analyzer
- Bank charge check
- Court case search
- Seller background check
- Approval check by survey

Value and duty

What it is worth and what it costs to register.

- Guideline value by street
- Guideline value by survey
- Stamp duty and registration fees
- Capital gains
- Floor space index
- Area unit converter

Zoning and land use

What the plan says can happen on this land.

- Land use zone finder
- CMDA land use check
- Coastal regulation zone
- Master plan check
- Water bodies finder
- Airport height clearance

Registers and directories

Who holds it, and which office keeps the record.

- Find your registering office
- Temple property check
- Waqf property check
- RERA register

Every free tool
/tools

Services we run for you
/services

The LandLens suite
/landlens/one

Verified RealEstate™

Every claim in this document traces back to a government record.

- Legal opinions, due diligence and title verification
- Building plan, layout and sub-division approvals
- Completion and occupancy certificates, regularisation
- Escrow, negotiation and NRI property management
- Land survey, soil testing, architecture and construction

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Prepared from the registers named on the page each finding sits on. Where a register could not be read, this document says so in its place rather than leaving the gap to be read as a clean result.