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mahimtura consultants pvt. ltd.

consulting engineers

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Dir.: H. R. MAHIMTURA B.E., M.S. (U.S.A.), F.I.E.
Tel. : 91-22-4368 5000 2266 1212
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C/9990/2025

STABILITY CERTIFICATE

(To be furnished while filling application for registration of building)

The detailed engineering and structure design of the Proposed construction of High rise Group Development Building for Residential cum Commercial use consisting of 4 Blocks with combined & extended double basement floor (parking use) for Block A, B, & D (Club House) Block A : stilt floor (Parking use) + 1st floor to 17th floors with 269 dwelling units: Block B : Stilt floor (Parking use) + 1st floor to 17th floors with 269 dwellings units; Block C (LIG Block) stilt floor (Parking use) +1st floor to 14th floors with 122 dwelling units, Block D(clubhouse)- ground floor part(shop & atm) cum stilt floor part(parking use) + 1st floor (Multipurpose Hall Creche , AV room, Association room, Co-working space, lego room) + 2nd floor (gym, indoor games, tables, tennis room, VR/ video games room, squash court and aerobics) + 3rd floor part (Open Swimming Pool with Changing room), Totally 660 dwelling units, availing premium FSI with TOD benefit abutting madhavaram milk colony main road & thanthai periyar street , milk colony, madhavaram, Chennai -600 051, comprised in old S.No.988 pt, 989 pt, 991/1A1 pt,1007/1 pt, 1008pt,1009pt, T.S.no:2/4, Block no:24,Ward -B, Zone-03, old division no.30,new division No.25, situated in Madhavaram Village, Madhavaram Taluk, Chennai District, within the limits of Greater Chennai Corporation has been done by me/us based on the report of geotechnical investigation (soil test) done by Geo Marine consultants (P) LTD and considering the functional requirement of the building.

1. It is certified that among other factors, the proposed building has been designed to resist earthquake. I/we have checked various parameters and found that the proposed building would be safe.

2. I/We further certify that;
   i) The minimum grade of concrete is M:30/ M:35 / M:40
   ii) The design and analysis have been done using the code of practice for plain and reinforced concrete as per IS 456, design load as per IS Codes No. 875-2015 and design of earthquake resistant structure as per IS Code No. 1893-2016.

The building will be safe and sound when used for the purpose for which it is designed, to the best of my knowledge and belief today.

Thanking You
Yours faithfully,
For **Mahimtura Consultants Pvt. Ltd.**

HITEN RAMANLAL MAHIMTURA

Digitally signed by HITEN RAMANLAL MAHIMTURA
DN: C=IN, PostalCode=400026, S=MAHARASHTRA, STREET="5TH FLOOR SARASWATI SADAN ,47 BHULABHAI DESAI ROAD ,MUMBAI, BREACH CANDY ,400026", L=MUMBAI, O=Personal, SERIALNUMBER=140ef5e96d45454a9ec79e47df9b1f7a9600845903b3a2a96ad88ce4bb55ce, OID.2.5.4.65=99b5e526b7904bbebea51c4d2ff39265, Phone=9707192188144ea9e03eca3120e6218da0733778e14e0e14606d91bfbca7b5, E=HITEN@MAHIMTURA.NET, CN=HITEN RAMANLAL MAHIMTURA
Reason: I am the author of this document
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MAHIMTURA CONSULTANTS (P) LTD. MUMBAI

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Branch Office : Plot No. 18, S.Y.No. 48/1, PDA Colony, Near River View Colony, Rels Magos Village, Alto, Porvorim, Goa – 403 521. Ph: (0832) 6511436. E-mail: mcplgoa@yahoo.co.in.

C/9990/2024

Date: 26th December, 2024

DESIGN BASIS REPORT

PROJECT : **PROPOSED RESIDENTIAL PROJECT AT MADHAVARAM, CHENNAI.**

CLIENT : **RADIANCE REALTY DEVELOPERS INDIA Ltd.**

CHIEF ARCHITECT : **NATARAJ & VENKAT ARCHITECTS.**

STRUCTURAL CONSULTANT : **MAHIMTURA CONSULTANTS PVT. LTD.**

GEO-TECH CONSULTANT : **GEO MARINE CONSULTANTS (P) LTD.**

Revision No	Date
R0	31.05.2024
R1	06.06.2024
R2	**02.09.2024**

Prepared by	Reviewed by	Approved by
JAGANNATH	IMRAN KHAN	PROF. MANIYAR

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1. INTRODUCTION

The proposed construction of high rise residential cum commercial group development buildings consisting of 4 blocks with combined & extended double basement floor (Parking use) for Block A, B & D (Clubhouse), in Block : A Stilt floor (Parking use) + First floor to 17th floors with 270 dwelling units, Block : B Stilt floor (Parking use) + First floor to 17th floors with 270 dwelling units, Block : C (LIG Block) stilt floor (Parking use) + First floor to 14th floors with 122 dwelling units, Block : D (Club house) – Ground floor part cum Stilt floor part to 3rd floor part totally 662 dwelling units, abutting Madhavram Milk Colony main road & Thanthai Periyar Street, milk colony, madhavaram, chennai - 600051. Comprised in old S.No.988 Part, 989 Part, 991/1A1 Part, 1007/1 Part, 1008 Part, 1009 Part, T.S.No: 2/4, Block No: 24, Ward-B, Situated in Madhavaram Village, Madhavaram Taluk, Chennai Distrcit, Within the limits of Greater Chennai Corporation. Zone No: 03, Old Subdivision No: 030, New Subdivision No: 025.

LMR/OHT without any future provisional floor. Terrace is provided for normal utilities with no garden landscaping.

2. STRUCTURAL SYSTEM

The Structural system for the residential building consists of RCC Shear wall Well-Distributed. (Building with Ductile RC Structural walls)
Block-A & B
The total height of the building up to Terrace slab is 55.70m (from Stilt level).
Block-C
The total height of the building up to Terrace slab is 46.55m (from ground level).

The floor-to-floor heights of the floors are shown below in the following table:

BLOCK AREA FLOOR LEVELS	FLOOR TO FLOOR HEIGHT (M)
Basement 2	3.3
Basement 1	3.5
Stilt floor	3.85
1st Floor to Terrace	3.05

3. DESIGN LOADS

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3.1 GRAVITY LOADS

The super imposed dead load that are envisaged to act permanently (wherever applicable) & Live loads for each occupancy category as per IS:875(part-2)–1987 and Live load reduction for upper floors is considered as per IS:875(part-2)

BASEMENT

Description		kN/m^2
Dead load	Slab & Beam	Self-weight
Live load	**Parking**	2.5
	Pump Room	5
Floor finish	**Parking**	1.5

STILT FLOOR

Description		kN/m^2
Dead load	Slab & Beam	Self-weight
Live load	Parking	2.5
	Fire Tender	25
	DG Set	15
	Electric room	5
	Driveway	2.5
	Staircase and Lobby	3
Floor finish		1.5

TYPICAL PLAN

Description		kN/m^2
Dead load	Slab & Beam	Self-weight
Live load	**Living/Dining/Bedroom/Ser. room**	2
	Corridor/ lobby	3
	Staircase	3
	Kitchen	2
	Toilet	2
	Balcony	3
Water Proofing		3
Floor finish		1.25
Filling Load	**Toilet (100mm filling)**	2
	Balcony (150mm filling)	3
	Kitchen (150mm filling)	3

TERRACE FLOOR PLAN

Description		kN/m^2
Dead load	Slab & Beam	Self-weight
Live load	Open terrace	2
Water Proofing		3

LIFT LOAD

Description		KN
Total weight of structure	In the absence of Lift load data. Assumed load per lift	400

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OVER HEAD TANK

Description		kN/m^2
Dead load	Slab & Beam	Self-weight
Live load	(The location and capacity as per architectural drawing)	
Floor finish	Tank Waterproofing	3
	Tank Top	1.5

PARTITION WALL LOAD

External Wall (150/200mm thk lightweight Block wall) = 6.4 kN/m^3 (650kg/m3)

(As per Architectural Drawing)

Internal Wall (100mm thk lightweight Block wall) = 6.4 kN/m^3 (650kg/m3)

(As per Architectural Drawing)

3.2 WIND LOAD

Wind Load (875 – PART 3- 2015):-

a)	Basic Wind Speed, Vb	:	50 m/s
b)	Terrain category	:	3
c)	Risk Coefficient, K_1	:	1
d)	Topography Factor,K3	:	1
e)	Terrain Factor, K2	:	1.11
f)	Importance Factor, K4	:	1.0
g)	Design Wind Speed, Vz max	:	$VbK_1K_2K_3K_4$
h)	Wind Pressure (max),Pz max	:	0.6 Vz^2
i)	Dynamic Evaluation	:	YES
j)	Gust Factor (G)	:	YES (For 20 year return period for Service) YES (For 50 year return period for Design)

Lateral Acceleration (IS 16700 Cl.6.2.3):

From serviceability considerations, under standard wind loads with return period of 10years, the maximum structural peak combines lateral acceleration a_{max} in the building for along and across wind actions at any floor level shall not exceed values given in table below, without or with the use of wind dampers in the building.

Building Use	Maximum Peak Combined Acceleration a_{max} m/s^2
Residential	0.18
Mercantile	0.25

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3.3 Earthquake Load (1893 – 2016):-

a)	Seismic Zone No.	:	II (For Chennai region)
b)	Zone Factor (Z)	:	0.16
c)	Response Reduction Factor (R)	:	4 (Building with Ductile RC Structural walls)
d)	Importance Factor (I)	:	1.2
e)	Type of Soil	:	Soil type II
f)	Building Dimension	:	Block-A 35.94m in 'x' direction & 86.80m in 'y' direction Block-B 37.16m in 'x' direction & 86.85m in 'y' direction Block-C 14.28m in 'x' direction & 47.52m in 'y' direction
g)	Height of Building(H)	:	Block-A & Block-B 62.50m from basement- to Terrace 55.85m from Ground up to Terrace Block-C 46.70m from Ground up to Terrace.
h)	Dynamic analysis	:	Response spectrum method of analysis as per Clause 6.4.3 is used in evaluating design earthquake force.

i) Fundamental natural time period (Ta) for plus shape building:

$$T_a = \frac{0.075h^{0.75}}{\sqrt{A_w}} \geq \frac{0.09h}{\sqrt{d}}$$

From Cl. 7.6.2.(b) – IS1893 (Part-I):2016

Horizontal seismic co-efficient

$$Ah = \frac{Z \times I \; Sa}{2 \times R \; g}$$

Design Base Shear

V_B = Ah x W

Where, W = Seismic weight of bldg. as per CL.7.4 - IS1893 (Part-I):2016
25 % upto& including live load 3.0kN/sq.m
50 % above 3.0kN/sq.m

Note: Vertical Earthquakes Effects shall be considered in accordance with Clause 6.3.3 Of IS: 1893 (Part 1): 2016.

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4. SLENDERNESS RATIO (Aspect Ratio)

The maximum values of the ratio of height to minimum base width shall not exceed the values given in below table. (IS16700 Clause 5.1.2)

Table 2 Maximum Slenderness Ratio (H_t/ B_t)
(*Clause* 5.1.2)

Sl No.	Seismic Zones	Structural System				
		Moment Frame	Structural Wall	Structural Wall + Moment Frame	Structural Wall + Perimeter Frame	Structural Wall + Framed Tube
(1)	(2)	(3)	(4)	(5)	(6)	(7)
i)	V	NA	8	8	9	9
ii)	IV	NA	8	8	9	9
Iii)	III	4	8	8	9	10
iv)	II	5	9	9	10	10

Building Aspect Ratio –

Block-1
X = 55.85/35.94 = 1.554 < 9
Y = 55.85/86.80 = 0.644 < 9

Block-2
X = 55.85/37.16 = 1.5 < 9
Y = 55.85/86.85 = 0.643 < 9

Block-3
X = 46.70/14.28 = 3.27 < 9
Y = 46.70/47.52 = 1.048 < 9

5. PLAN ASPECT RATIO (IS 16700 Clause 5.2.2)

The maximum plan aspect ratio (L/B) of the overall building shall not be exceed 5.
Plan Aspect Ratio
Block-1
L= 86.80 m
B=35.94 m
L/B= 86.80/35.94= 2.415 <5

Block-2
L= 86.85m
B=37.16m
L/B= 86.85/37.16= 2.337 <5

Block-3
L= 47.52m
B=14.28m
L/B= 44/14.2= 3.3278 <5

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6. HEIGHT LIMIT FOR STRUCTURAL SYSTEMS

The maximum building height in meter for buildings with different structural system shall not exceed values given in table.

Table 1 Maximum Value of Height, *H* Above Top of Base Level of Buildings with Different Structural Systems for Lateral Loads, in metre

(*Clause* 5.1.1)

Sl No.	Seismic Zones	Structural System				
		Moment Frame	Structural Wall Well-Distributed[1)]	Structural Wall + Moment Frame	Structural Wall + Perimeter Frame	Structural Wall + Framed Tube
(1)	(2)	(3)	(4)	(5)	(6)	(7)
i)	V	NA	120	150	150	180
ii)	IV	NA	150	200	200	225
iii)	III	60	200	225	225	250
iv)	II	80	250	250	250	250

[1)] Well-distributed shear walls are those walls outside of the core that are capable of carrying at least 25 percent of the lateral loads.

7. DEFLECTION

Beam Slab deflection: -Span /Depth ratio as per IS: 456-2000.

8. BUILDING SWAY AND INTER-STOREY DRIFT (IS 16700 Cl.5.4.1)

When design lateral forces are applied on the building, the maximum inter-storey elastic lateral drift ratio under serviceability loads (wind load with return period of 20 years), which is estimated based on the sectional properties for serviceability loads shall be limited to H/500. For a single story the drift limit may be relaxed to h/400. For the design earthquake, the drift shall be limited to h/250.

9. BASIC LOADS AND LOAD COMBINATIONS

The various loads are combined in accordance with the stipulations in IS: 875 (Part 5):1987. Wherever imposed load is combined with earthquake load the appropriate part of the imposed load as specified in IS: 1893-2016 is adopted both for evaluating earthquake effect and for combined load effects, used in such combination.

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- **BASIC LOAD CASES**

Load Case Type	Mark	Description
Primary	DL	SELF WEIGHT OF STRUCTURAL MEMBER
Primary	SDL	SUPERIMPOSED DEAD LOAD
Primary	LL1	LIVE LOAD =<3 kN/m2 (Live 1)
Primary	LL2	LIVE LOAD > 3 kN/m2 (Live 2)
Primary	LL3	NON REDUCIBLE LIVE LOAD
Primary	EQX	SEISMIC LOAD IN X DIRECTION
Primary	EQY	SEISMIC LOAD IN Y DIRECTION
Primary	WLX	WIND LOAD IN X DIRECTION
Primary	WLY	WIND LOAD IN Y DIRECTION

ULTIMATE LOAD COMBINATION:-

Description of Load Combination	Load Factor for Primary Load Cases			
	Dead Load	Live Load	Wind Load	Seismic Load
Dead load + Live load	1.5	1.5	0.0	0.0
Dead load ± Wind load	1.5	0.0	±1.5	0.0
Dead load ± Seismic load	1.5	0.0	0.0	±1.5
Dead load + Live load ± Wind load	1.2	1.2	±1.2	0.0
Dead load + Live load ± Seismic	1.2	1.2	0	±1.2
Dead Load ± Wind Load	0.9	0.0	±1.5	0.0
Dead Load ± Seismic Load	0.9	0.0	0.0	±1.5

For Vertical Earthquake as per 6.3.3 of IS:1893:2016

Description of Load Combination	Load Factor for Primary Load Cases			
	Dead Load	Live Load	Wind Load	Seismic Load/Spec
DL + LL ± (A ± 0.3B ±0.3C)	1.2	1.2	0.0	1.2
DL + LL ± (B ±0.3A± 0.3C)	1.2	1.2	0.0	1.2
DL + LL ± (C ±0.3A± 0.3B)	1.2	1.2	0.0	1.2
DL ± (A ±0.3B± 0.3C)	1.5 or 0.9	0.0	0.0	1.5
DL ± (B ±0.3A± 0.3C)	1.5 or 0.9	0.0	0.0	1.5
DL ± (C ±0.3A± 0.3B)	1.5 or 0.9	0.0	0.0	1.5

Note :- (A = Spec X , B= Spec Y, C= Spec Z)

- **Serviceability Load Combination: -**

Description of Load Combination	Load Factor for Primary Load Cases			
	Dead Load	Live load	Wind Load	Seismic Load
Dead load + Live load	1.0	1.0	0.0	0.0
Dead load ± Wind load	1.0	0.0	±1.0	0.0
Dead load ± Seismic load	1.0	0.0	0.0	±1.0
Dead load + Live load ± Wind load	1.0	0.8	±0.8	0.0
Dead load + Live load ± Seismic	1.0	0.8	0.0	±0.8

10. ANALYSIS METHODOLOGY & MODELLING

The structure is analyzed for Static loads (both dead and live loads) and Lateral loads due to Earthquake/Wind loads and its combinations. Software used is Etabs version 18 & for foundation safe version 16. The structure will be modeled as 3D space frame. Floor beams shall be modeled as single line elements. Vertical elements shall be modeled as shell elements accounting for in plane and out of plane behavior. Floor slabs shall be modeled as shell/membrane elements for diaphragm effect only.

Gravity load resisting system

The gravity load resisting system consists of conventional beams and slabs at typical floor. Gravity loads from floors system is transferred to column to wall and raft foundation.

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Lateral load resisting system

The lateral load resisting system consists of Building with Well distributed Ductile RC Structural Walls + perimeter frame. The floor slab acting as a diaphragm (collector) shall transfer lateral load to R.C walls. Modifiers used in the design are as follows (IS16700:2023, Clause 7.2):

Modifier: (IS 16700 Cl.7.2)

Structural Elements	Service Condition	Strength Condition
Slab	0.35	0.25
Beam	0.7	0.35
Columns	0.9	0.7
Walls	0.9	0.7

11. DESIGN LIFE

The design life of the structure is assumed as 50 years.

12. DESIGN METHODOLOGY

All structures shall be designed according to the Limit State Method as specified in IS: 456-2000. Appropriate loads and its combinations, as per relevant clauses in IS 456:2000 code, for the most unfavorable effects are chosen for design.

13. CONSTRUCTION DETAILS

1. Foundation - Raft foundation / Open footing. Mix M: 30.
2. Walls /columns Conc. Mix - M: 30 / M: 40
3. Beams & Slab Conc. Mix - M: 30/M25.
4. Retaining Wall Conc. Mix - M: 30.

14. REINFORCEMENT

Fe550 Steel reinforcement Grade used for project will be tested for its physical and chemical property's along with the prescribe elongation required for conforming to IS: 1786-2008.

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15. EXPOSURE CONDITION

Structural elements below Plinth level and External column and beam are designed for moderate exposure condition and internal column, floor beams and slabs are designed for mild exposure condition as per Table 16 of IS 456-2000.

16. MATERIALS

The self-weight of the various elements are computed based on the built weight of materials as given below:-

Materials	Unit Weight in kN/m^3
Steel	78.50
Plain Concrete	24.00
Reinforced Concrete	25.00
Soil	18.00
Saturated Soil	22.00 to 25.00
Water	10.00
Concrete Block	24.00
Fly ash Brick	18.00
Toilet Sunk Filling Load	20.0
Plaster	20.4
Plum concrete	24.0
M.G.O Wall/Light weight masonry	10
Hollow Block work	14.5

17. COVER TO REINFORCEMENT

The clear cover to main reinforcement is provided for 2 hours fire rating for primary load resisting system (Gravity/Lateral).

	Cover	Fire Resistance(Hours)
Staircase waist slab	25mm	2
Terrace and Floor Slabs	25mm	2
Shear Walls & Columns	40mm	2
Floor Beams	40mm	2
Footing	75mm	-
Raft slab	75mm	
Retaining Walls (Backfilling side)	50mm	-
Retaining Walls (Internal side)	40mm	-

18. FOUNDATION SYSTEM

Raft foundation for an allowable net safe bearing pressure of 600 kPa is recommended with modules of subgrade 25,000 kN/m3 at 6m below existing ground level.

As Per Geotechnical report.

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19. CODES & STANDARDS

All the designs are conforming to the relevant Indian Standards with latest amendments. Some of the relevant Indian Standard Codes, which have been followed for the structural design, are given below:

CODE	DESCRIPTION
IS:875(Part-1)-2018	Code of Practice for Design Loads (Other than Earthquake) for Building and Structures-Unit Weights of Buildings Materials and Stored Material.
IS:875(Part-2)-2018	Code of Practice for Design Loads (Other than Earthquake) for Building and Structures-Imposed loads.
IS:875(Part-3)-2020	Code of Practice for Design Loads (Other than Earthquake) for Buildings and Structures –Wind Load.
IS:875(Part-5)-2018	Code of Practice for Design Loads (Other than Earthquake) for Buildings and Structures - Special Loads and Load Combinations.
IS:456 -2000	Code of Practice for Plain and Reinforced Concrete.
IS:1893(Part-1) 2016	Indian Standard Criteria for Earthquake Resistant Design of Structures
IS:432(Part-1)-1982	Specification of Mild Steel and Medium Tensile Steel bars & Hard drawn Steel Wire for concrete reinforcement–Mild Steel & Medium Tensile Steel Bars
IS:432(Part-2)-1995	Specification of Mild Steel and Medium Tensile Steel bars and Hard drawn Steel Wire for concrete reinforcement –Hard drawn Steel Wire.
IS:1786-2018	Specification for High Strength Deformed Steel Bars and Wires for Concrete Reinforcement
IS:13920-2016	Ductile Detailing of reinforced concrete structures subjected to seismic forces – code of practice.
IS:1904-2021	Codes of practice for design & construction of foundation in soils: General requirements (third revision)
IS:2911(Part 1/Sec 1) : 2010	Code of practice for design & construction of pile foundations: Part 1 concrete piles, Section 1 Driven cast in – situ concrete piles (second revision)
IS:2911(Part 1/Sec 2) : 2010	Code of practice for design & construction of pile foundations: Part 1 concrete piles, Section 2 Bored cast in – situ piles (second revision)
IS:2911(Part 1/Sec 3) : 2010	Code of practice for design & construction of pile foundations: Part 1 concrete piles, Section 3 Driven precast concrete piles (second revision)
IS:2911(Part 1/Sec 4) : 2010	Code of practice for design & construction of pile foundations: Part 1 concrete piles, Section 4 Bored precast concrete piles
IS:2950(Part1): 2013	Code of practice for design & construction of raft foundations: Part 1 Design (second revision)
IS:4326: 2013	Earthquake Resistant Design and construction of building code of practice (second revision)
IS:3370(Part1):2021 (Part 2): 2021	Code of Practice concrete structures for the storage of liquid General requirements Reinforced concrete structures
IS: 800:2017	Code of practice for general construction in steel
IS:16700:2023	Criteria for Structural Safety of Tall Concrete Buildings

HITEN R. MAHIMTURA
Consulting Structural Engineer
Registration No. **SE/GR-I/2019/03/021**
Chennai Metropolitan Development Authority,
Government of Tamil Nadu

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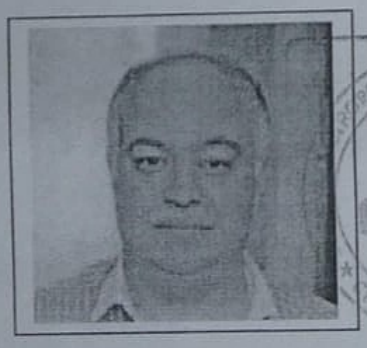
CHENNAI METROPOLITAN DEVELOPMENT AUTHORITY
'ThalamuthuNatarajan Building', No.1, Gandhi Irwin Road,
Egmore, Chennai-600 008.

PROFESSIONAL REGISTRATION CERTIFICATE

This is to certify that the **Thiru. HITEN RAMANLAL MAHIMTURA** has enrolled as **Registered Structural Engineer Grade-I (SE)** with Chennai Metropolitan Development Authority (CMDA) who has the qualification of **M.S.,(Civil Engg.)** and experience in the relevant field for **33years**.

He has been given the Registration No.**SE/GR-I/2019/03/021** Renewed on **22.03.2024.** He shall be required to comply with the duties and responsibilities prescribed by the Government of Tamil Nadu in Part-V, "Registration of Professionals", Tamil Nadu Combined Development and Building Rules, 2019 approved vide G.O.(Ms) No.18 MAWS (MA1) Dept. dated 04.02.2019.

This Registration is valid upto **21.03.2029**.



METROPOLITAN DEVELOPMENT AUTHORITY
CHENNAI-600 008.

22/3/24

(A. MEGANATHAN)
Superintendent



our services doesn't just end there

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Compliance & Special Services

Demolition, Vaasthu checks, regularization of unauthorized buildings, and office space solutions.



empowering your real estate decisions with smart tools

Property Valuation & Cost Estimation

Guideline Value, Apartment Composite Value, Stamp Duty & Registration Fees Calculator, Building Value Calculator, Capital Gains Calculator, Area Unit Converter, Length Unit Converter

Ownership, Compliance & Legal Verification

Encumbrance Certificate (EC) Search, Find Your SRO, Temple Property Check, WAQF Property Check, View Patta/Chitta, View FMB, Patta Analyzer, EC Analyzer

Zoning & Land Use Analysis

Land Use Zone Finder, CRZ Zone Check, Aquifer Zone Check, CBA Check, EWS Check, Road Width Check, Proposed Road Widening Check, FSI Calculator

Property Documentation & Workflow Tools

Forms & Templates, Document Generator, Document Translation, Dictionary

Vastu & Property Management Tools

Home Vastu Shastra, Plot Vastu Shastra, Property Management

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