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GEOTECHNICAL INVESTIGATION REPORT

REPORT NO: -GEO-69/2022-23 **DATE: -12.01.2023**

PROJECT NAME: -

GEOTECHNICAL INVESTIGATION FOR THE PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENT BUILDING (STILT+5 FLOORS) IN SITE NO: -185,186,187 &188, S.F.NO: -378/1A IN SESI AVENUE, KALAPATTY VILLAGE, COIMBATORE DISTRICT.

CLIENT: -

Mr.BALASAMY ROHIT,

SESI AVENUE, KALAPATTY VILLAGE, COIMBATORE DISTRICT.

SUNSTAR FOUNDATION CONSULTANT

(Civil Engineering Consultant & Contractor)

Soil Investigation, Pile Load Test, Plate Load Test, Rock Core Drilling, Earth Resistivity, Topographical Surveying & Laboratory Material Testing

(An ISO 9001: 2015 Certified laboratory)

No: -15/16, First Floor, Kasthuribai 4th Street, Ganapathy, Coimbatore-641006.

Mob: -+91-8300404680,8300404681, E-mail: -sunstarfoundationconsultant@gmail.com

www.sunstarfoundationconsultant.com

SUNSTAR FOUNDATION CONSULTANT CBE 641006

Er. RAVI, ME., PhD (Soil Mechanics)., F.I.E.F.I.G.S.
Chief Geotechnical Consultant,
Sunstar Foundation Consultant.

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1.INTRODUCTION: -

Mr.Balasamy Rohit, Sesi Avenue, Kalapatty Village,Coimbatore District has entrusted the work of conducting geotechnical investigations at structures locations pertaining to The Proposed Construction of Residential Apartment Building (Stilt+5 Floors) in Site No: -185,186,187 &188, S.F.No: -378/1A in Sesi Avenue, Kalapatty Village,Coimbatore District was awarded to M/S.Sunstar Foundation Consultant. This report presents results of the geotechnical investigation along with foundation recommendations for the proposed structure.

1.1. SCOPE OF WORK: -

- Conducting Geotechnical Investigation along with suitable foundation recommendation for the proposed structure.
- Determine the soil profile at the site with ground water level observation.
- Conducting SPT's at regular depth intervals.
- Collecting Disturbed Soil (DS)/ Undisturbed Soil (UDS) Samples.
- Conducting relevant laboratory tests on Soil samples.
- Submitting report with field observations, laboratory test results along with recommendations and safe bearing pressure at foundations level.

In this report, preliminary details of sub-soil layers encountered, brief account of the strength of the strata based on the field Tests, discussion on foundation options and recommendations for foundations are presented.

2. SITE DISCRIPTION: -

Based on boreholes information, sub surface profile for The Proposed Construction of Residential Apartment Building (Stilt+5 Floors) in Site No: -185,186,187 &188, S.F.No: -378/1A in Sesi Avenue, Kalapatty Village,Coimbatore District. The area where the tests were conducted for the borehole location-1&2 undulating level area with respect to surroundings.

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3. FIELD INVESTIGATION: -

3.1. Drilling in Overburden: -

Rotary drilling machines were mobilized and drilling operation was taken up. 100mm to 150mm diameter boreholes were made at pre-determined locations and the boreholes were advanced up-to varying depths. The drilling was commenced by driving a casing of suitable diameter in the upper layers of the bore holes. Drilling was carried out as per IS 1892 - 1979.In this method, boring is effected by the cutting action of a rotating bit that is kept in firm contact with the bottom of the hole. The bit is attached to the lower end of a hollow drill rod that is rotated by a suitable chuck. Drilling mud (usually Bentonite) is continuously forced down the hollow drill rods. The mud returning upwards though the annular space between the drill rods and the side of the hold brings the cutting to the surface. As stated earlier, the boreholes are terminated after sufficiently advancing in hard strata.

3.2. Standard Penetration Test: -

Standard Penetration Tests (SPT) were conducted as per IS 2131-1981. In a SPT test, a standard split spoon sampler is driven at the bottom of the borehole. The penetration resistance in terms of blows for 300mm penetration of the split spoon sampler is measured as 'N' Value. The blows are imparted by a standard weight of 63.5 kg falling through a height of 750 mm. The resistance is measured for 150 mm, 300 mm, and 450 mm penetrations. The resistance of first 150 mm is ignored since those blows are considered as seating blows due to he loosening of the strata. The resistance of next 300 mm is recorded as standard penetration value 'N`. If the sampler is driven less than 450 mm (total) then the penetration resistance is given for the last 300 mm of penetration. If the penetration depth is less than 300 mm and the blow count is more than 50 then the 'N' value is considered as 'Refusal' and actual penetration is recorded with the blows. These tests were conducted at close intervals of 1.50m

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Er. RAVI, ME., PhD (Soil Mechanics)., F.I.E.F.I.G.S.
Chief Geotechnical Consultant,
Sunstar Foundation Consultant.

depth intervals at shallow depths. Disturbed Soil (DS) collected in the SPT sampler was preserved in polythene covers and transported to the laboratory. One more polythene cover was provided to prevent the loss of moisture during the transit period.

3.3. Collection of Samples: -

Soil samples were collected through Split spoon was done up to bore hole termination level.

4. FIELD INVESTIGATION PHOTOGRAPHS: -

BOREHOLE LOCATION





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Chief Geotechnical Consultant,
Sunstar Foundation Consultant.

5.1. FIELD BORELOG DATA SHEET-1

NAME OF THE PROJECT: - "THE PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENT BUILDING (STILT+5 FLOORS) IN SITE NO: -185,186,187 &188, S.F.NO: -378/1A IN SESI AVENUE, KALAPATTY VILLAGE, COIMBATORE DISTRICT ".

TYPE OF BORING: -ROTARY	GPS COORDINATES	GROUND WATER TABLE: -Not Met
LOCATION: -BH-1	LATITUDE: -11.04725615	DATE OF STARTED: -12.01.2023
BED LEVEL: -	LONGITUTE: -77.02979978	DATE OF COMPLETED: -12.01.2023

Depth below 'NGL' (m)	Soil profile	Description of Strata	Thickness of stratum (m)	Depth of samples collected (m)	SPT- 'N' Values Details				Type of samples collected	Relative Density/ Consistency	Graphical representation of SPT- 'N' Values				
					N1=15cm	N2=30cm	N3=45cm	N_{VALUE}= N2+N3			20	40	60	80	100
0.00				0.00	-	-	-	-	-	-					
0.70		Low Plasticity Clay	0.70												
1.10		Low Plasticity Clay with kankar	0.40	1.00	8	12	18	30	V.Stiff	DS					
2.80		Gravelly Sand	1.70	2.00	13	20	24	44	Dense	DS					
				3.00	50/11	-	-	>50	V.Dense	DS					
				4.50	50/8	-	-	>50	V.Dense	DS					
				6.00	50/7	-	-	>50	V.Dense	DS					
		Soft Disintegrated Rock	7.20	7.50	50/5	-	-	>50	V.Dense	DS					
				9.00	50/4	-	-	>50	V.Dense	DS					
10.00		End of Borehole		10.00	50/3	-	-	>50	V.Dense	DS					

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5.2. FIELD BORELOG DATA SHEET-2

NAME OF THE PROJECT: - "THE PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENT BUILDING (STILT+5 FLOORS) IN SITE NO: -185,186,187 &188, S.F.NO: -378/1A IN SESI AVENUE, KALAPATTY VILLAGE, COIMBATORE DISTRICT ".

TYPE OF BORING: -ROTARY	GPS COORDINATES	GROUND WATER TABLE: -Not Met
LOCATION: -BH-2	LATITUDE: -11.04745852	DATE OF STARTED: -13.01.2023
BED LEVEL: -	LONGITUTE: -77.02999692	DATE OF COMPLETED: -13.01.2023

Depth below 'NGL' (m)	Soil profile	Description of Strata	Thickness of stratum (m)	Depth of samples collected (m)	SPT- 'N' Values Details				Type of samples collected	Relative Density/ Consistency	Graphical representation of SPT- 'N' Values				
					N1=15cm	N2=30cm	N3=45cm	N_{VALUE}= N2+N3			20	40	60	80	100
0.00		Low Plasticity Clay	0.60	0.00	-	-	-	-	-	-					
0.60				1.00	9	11	20	31	V.Stiff	DS					
1.20		Low Plasticity Clay with kankar	0.60												
		Gravelly Sand	1.90	2.00	13	18	25	43	Dense	DS					
3.10				3.00	50/10	-	-	>50	V.Dense	DS					
				4.50	50/9	-	-	>50	V.Dense	DS					
				6.00	50/6	-	-	>50	V.Dense	DS					
		Soft Disintegrated Rock	8.90	7.50	50/4	-	-	>50	V.Dense	DS					
				9.00	50/3	-	-	>50	V.Dense	DS					
				10.50	50/3	-	-	>50	V.Dense	DS					
12.0		End of Borehole													

ROCK CORE RECOVERY DETAILS FROM 10.50M TO 12.00M
DEPTH ENCLOSED THE PAGE NO: -10

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Er. RAVI, ME., PhD (Soil Mechanics)., F.I.E.F.I.G.S.
Chief Geotechnical Consultant,
Sunstar Foundation Consultant.

6. LABORATORY INVESTIGATION: -.

Following laboratory tests are conducted on the soil samples

- Natural Moisture Content
- Natural Wet Density
- Natural Dry Density
- Atterberg's Limit
- Sieve Analysis
- Specific Gravity
- Shear Test Parameters
- Consolidation Test

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Er. RAVI, ME., PhD (Soil Mechanics)., F.I.E.F.I.G.S.
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6.1. LABORATORY TEST RESULTS-1

NAME OF THE PROJECT: - "THE PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENT BUILDING (STILT+5 FLOORS) IN SITE NO: -185,186,187 &188, S.F.NO: -378/1A IN SESI AVENUE, KALAPATTY VILLAGE, COIMBATORE DISTRICT ".

TYPE OF BORING: -ROTARY
BED LEVEL: -

LOCATION: -BH -1
GROUND WATER TABLE: - NOT MET

DATE OF LAB TESTING STARTED: - 13.01.2023
DATE OF LAB TESTING COMPLETED: 19.01.2023

SOIL PROFILE AND DEPTH IN METRES	DEPTH OF SAMPLING	IS SOIL CLASSIFICATION	NATURAL WET DENSITY (kN/m³)	NATURAL MOISTURE CONTENT (%)	NATURAL DRY DENSITY (kN/m³)	INDEX TEST (Atterberg Limit)			SIEVE ANALYSIS % PASSING IS SIEVE ANALYSIS					SPECIFIC GRAVITY	SHEAR TEST PARAMETERS		CONSOLIDATION TEST			STANDARD PENETRATION TEST		
						LIQUID LIMIT (%)	PLASTIC LIMIT (%)	PLASTICITY INDEX	Gravel (>4.75MIC)	SAND			SILT&CLAY (<75MIC) HYDROMETER ANALYSIS		COHESION (UCC) (C - Ton / m²)	ANGLE OF INTERNAL FRICTION (φ)	COMPRESSION INDEX (Cc) (Kg/cm²)	INITIAL VOIDS RATIO (Eo)	PRE - CONSOLIDATION PRESSURE (Kg/cm²)	DEPTH OF TESTING (m)	OBSERVED N VALUE	SAFE BEARING CAPACITY (T/m²)
										COARSE	MEDIUM	FINE										
	0.00	CL	14.55	10.35	13.18	32	28	4	4	5	7	12	72	2.74	3.90	-	-	-	-	0.00	-	-
	0.70	CL/ KAN	14.70	-	14.70	36	NP	NP	9	11	13	15	52	2.71	3.60	26	-	-	-	1.00	30	20.42
	1.10	SW	16.75	-	16.75	NP	NP	NP	31	28	26	10	5	2.68	-	30	-	-	-	2.00	44	23.74
	2.80	SDR	21.80	-	21.80	NP	NP	NP	69	16	8	4	3	2.66	-	35	-	-	-	3.00	>50	36.70
																				4.50	>50	40.48
																				6.00	>50	43.63
																				7.50	>50	47.03
																				9.00	>50	50.37
	10.00																			10.00	>50	53.14

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CL-CLAY OF LOW PLASTICITY, CL/KAN-CLAY WITH KANKAR, SW-GRAVELLY SAND, SDR-SOFT DISINTEGRATED ROCK NP- NON-PLASTIC, DS- DISTURBED SAMPLE.

"GEOTECHNICAL INVESTIGATION CONDUCTED BY M/S.SUNSTAR FOUNDATION CONSULTANT,COIMBATORE."

6.1. LABORATORY TEST RESULTS-2

NAME OF THE PROJECT: - "THE PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENT BUILDING (STILT+5 FLOORS) IN SITE NO: -185,186,187 &188, S.F.NO: -378/1A IN SESI AVENUE, KALAPATTY VILLAGE, COIMBATORE DISTRICT ".

TYPE OF BORING: -ROTARY
BED LEVEL: -

LOCATION: -BH -2
GROUND WATER TABLE: - NOT MET

DATE OF LAB TESTING STARTED: - 13.01.2023
DATE OF LAB TESTING COMPLETED: 19.01.2023

SOIL PROFILE AND DEPTH IN METRES	DEPTH OF SAMPLING	IS SOIL CLASSIFICATION	NATURAL WET DENSITY (kN/m³)	NATURAL MOISTURE CONTENT (%)	NATURAL DRY DENSITY (kN/m³)	INDEX TEST (Atterberg Limit)			SIEVE ANALYSIS % PASSING IS SIEVE ANALYSIS					SPECIFIC GRAVITY	SHEAR TEST PARAMETERS		CONSOLIDATION TEST			STANDARD PENETRATION TEST		
						LIQUID LIMIT (%)	PLASTIC LIMIT (%)	PLASTICITY INDEX	Gravel (>4.75MIC)	SAND			SILT&CLAY (<75MIC) HYDROMETER ANALYSIS		COHESION (UCC) (C - Ton / m²)	ANGLE OF INTERNAL FRICTION (φ)	COMPRESSION INDEX (Cc) (Kg/cm²)	INITIAL VOIDS RATIO (Eo)	PRE - CONSOLIDATION PRESSURE (Kg/cm²)	DEPTH OF TESTING (m)	OBSERVED N VALUE	SAFE BEARING CAPACITY (T/m²)
										COARSE	MEDIUM	FINE										
	0.00	CL	14.40	10.40	13.00	31	26	5	3	4	9	14	70	2.72	3.80	-	-	-	-	0.00	-	-
	0.60	CL/ KAN	14.60	-	14.60	36	NP	NP	8	10	12	16	54	2.70	3.65	27	-	-	-	1.00	31	21.16
	1.20																			2.00	43	23.18
		SW	16.85	-	16.85	NP	NP	NP	30	26	29	11	4	2.69	-	32	-	-	-	3.00	>50	36.73
	3.10																			4.50	>50	40.57
																				6.00	>50	43.75
		SDR	22.05	-	22.05	NP	NP	NP	72	14	9	3	2	2.67	-	38	-	-	-	7.50	>50	47.26
																				9.00	>50	50.55
																				10.50	>50	53.51
	12.00																					

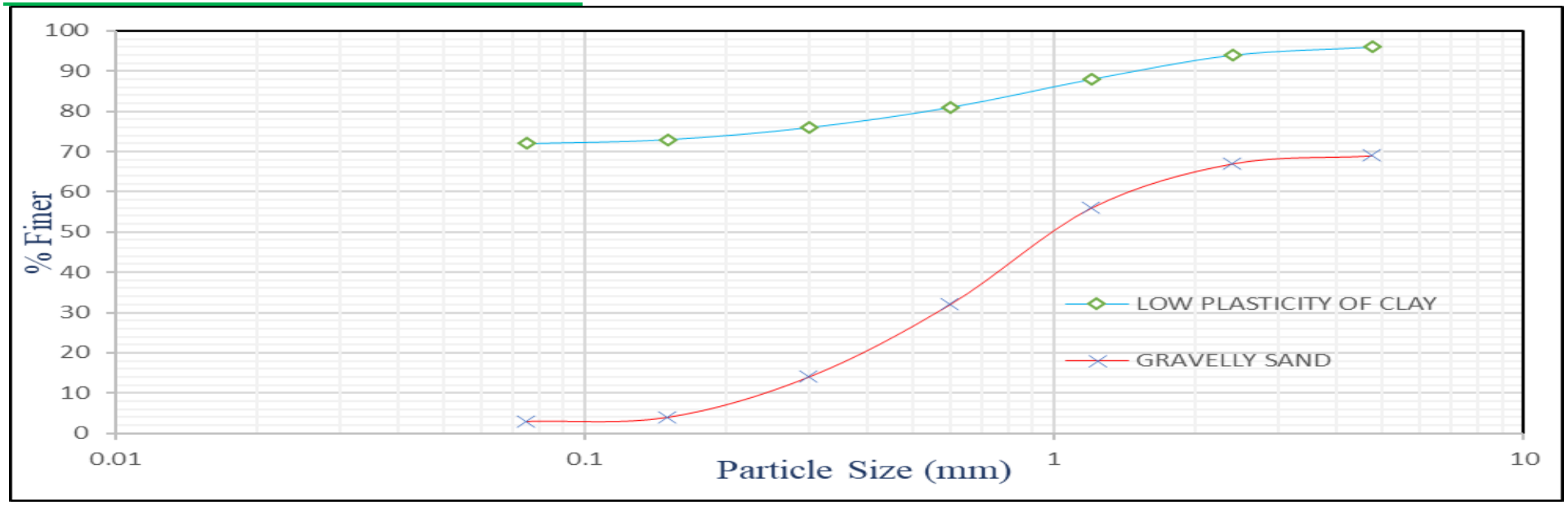
Depth at which collected (m)	Length of Individual Pieces (cm)	Total length (cm)	Core Recovery (%)	Rock Quality Designation (%)	Ultimate Unconfined Compressive Strength (MPa)	Remarks
10.50 – 12.00	5.50+6.00 2 Small Pieces	11.50	7.66	-	9.585	SDR – Soft Disintegrated Rock

CL-CLAY OF LOW PLASTICITY, CL/KAN-CLAY WITH KANKAR, SW-GRAVELLY SAND, SDR-SOFT DISINTEGRATED ROCK NP - NON-PLASTIC, DS- DISTURBED SAMPLE.

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Chief Geotechnical Consultant
Sunstar Foundation Consultant.

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7.GRAIN SIZE DISTRIBUTION CURVE: -



8. DISCUSSION ON FOUNDATION: -

8.1. Sub Surface Conditions: -

Subsurface profile at this site generally consists of top low plasticity of clay with kankar mixtures then gravelly sand overlying underlain by soft disintegrated rock. Groundwater accumulation in boreholes was monitored during and after completion of drilling activities. Groundwater was observed in boreholes at depths between not met below ground surface. Seasonal and annual fluctuations in ground water levels can be expected.

8.2. Engineering Appraisal: -

Based on field and laboratory results on the samples collected from investigation locations, the shear strength parameters such as c (cohesive strength) and φ (angle of internal friction) can be correlated with SPT 'N' values.

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Chief Geotechnical Consultant.
Sunstar Foundation Consultant.

8.3. Design Methodology: -

The safe bearing capacity of the soil can be calculated on shear failure criteria and settlement criteria and any foundation system has to satisfy the above said criteria. The former one depends on shear strength and the bearing capacity equation suggested by IS 6403 as follows. Net ultimate bearing capacity is given by C Nc Sc dc ic + γ D (Nq – 1) Sq dq iq + 0.5 γ B Nγ Sγ dγ iγ W'

The safe bearing capacity is the maximum gross intensity of loading that the soil can carry safely without failing in shear which is obtained by the net ultimate bearing capacity divided by factor of safety (2.5 to 3.0)

The later one follows settlement criteria, that the intensity of the loading will cause within the permissible settlement is termed as allowable bearing pressure given by correcting the field SPT 'N' values.

1. **Correction due to dilatancy (N').** This correction is based on the assumption that critical void ratio occurs at approximately N = 15, and in fine grained cohesionless soils, the coefficient of permeability is low that the excess pore water pressure developed by the driving impedes the penetration of split spoon sampler, thus increasing the N value.

$$N' = 15 + 0.5 (N_O - 15)$$

2. **Correction due to overburden pressure**

$$N = 0.77\ N'\ Log_{10} (2000/ P'_O)$$

P'ₒ effective over burden pressure should be greater than 25 kpa

Net allowable bearing pressure developed by Terzaghi and peck (1948)

$$Q_S = 3.5 (N_{cor} - 3) ((B + 0.3)/ 2B)^2 \times R_{W2} \times F_D + P$$

The Allowable Bearing Pressure of Soil = Net Allowable Bearing Pressure of Soil + P'_o

Where

N' = Corrected N value

N'' = Corrected N value due to dilatancy

B = Size of the Footing = constant 1.00x1.00m

RW = Water Correction = constant 0.30m to0.50m

FD = Depth of Factor

P = Over Burden Pressure

SUNSTAR FOUNDATION CONSULTANT CBE 641006

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Chief Geotechnical Consultant.
Sunstar Foundation Consultant.

7. FOUNDATION RECOMMENDATION: -

Based on the sub-soil conditions and possible loads on foundations, following recommendations for Geotechnical Investigation for The Proposed Construction of Residential Apartment Building (Stilt+5 Floors) in Site No: -185,186,187 &188, S.F.No: -378/1A in Sesi Avenue, Kalapatty Village,Coimbatore District.

- The **Isolated/Combined Footing** will be recommended for the proposed structure.
- Rest the Foundation depth at **3.00m to 3.50m** from the Existing Ground Level.
- The recommended SBC of **36.70 Ton/m^2 at 3.00m depth**. Its calculated based on SPT– 'N' values conducted inside the borehole.

Er. M.ANANDHAKUMAR, BE, M.I.G.S
Project Engineer,
Sunstar Foundation Consultant.

Er.M.ANANDHAKUMAR,B.E,M.I.G.S
Project Engineer,
Sunstar Foundation Consultant.

Er. RAVI, ME., PhD (Soil Mechanics)., F.I.E.F.I.G.S.
Chief Geotechnical Consultant,
Sunstar Foundation Consultant.

Dr.E.RAVI,M.E.,Ph.D(Soil Mechanics).,F.I.E,F.I.G.S.,
Chief Geotechnical Consultant ,
Sunstar Foundation Consultant.

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9.BIS CODE & REFERENCES: -

S. NO	CODE NUMBERS	DESCRIPTION
1	IS 1888: 1982	Method of load tests on soils
2	IS 6403: 1981	Code of practice for Determination of Bearing capacity of foundation.
3	IS 1892: 1979	Code of practice for subsurface investigations for foundations.
4	IS 8009(Part-1&2) 1976	Code of practice for calculation of settlement of foundations
5	IS 2131: 1981	Method of standard penetration test for soils
6	IS 2132: 1986	Code of practice for thin-walled tube sampling of soils
7	IS 2720 Part 1 to Part 31 & IS 2720 Part 33 to Part 41	Methods of Test of Soils
8	IS 3043 :1987	Code of practice of Earthling
9	IS 4464: 1985	Code of practice for presentation of drilling information and core description in foundation investigation
10	IS: 2911 (PART 1/ SEC 1- 4) - 1979	Code of practice for pile design
11	IS 6935: 1973	Method of determination of water level in a borehole

SUNSTAR FOUNDATION CONSULTANT CBE 641006

Er. RAVI, ME., PhD (Soil Mechanics)., F.I.E.F.I.G.S.
Chief Geotechnical Consultant.
Sunstar Foundation Consultant.



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Forms & Templates, Document Generator, Document Translation, Dictionary

Vastu & Property Management Tools

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