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SOIL EXPLORATION AND FOUNDATION REPORT

FOR THE PROPOSED STILT FLOOR + FIVE FLOORS RESIDENTIAL BUILDING WITH 45 DWELLING UNITS AT MOOKAMBIGAI NAGAR 1ST MAIN ROAD, POONAMALLEE, CHENNAI COMPRISED IN OLD S.NOS 106/13 & 110/6, NEW S.NO. 25/3A1A1A & AS PER PATTA S. NO. 25/80 & 25/81 OF NOOMBAL VILLAGE WITHIN THE LIMITS OF THIRUVERKADU MUNICIPALITY

PROJECT NO. TJE/002403/2022



T.J.ENGINEERS

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EXPLORATION REPORT

PROJECT NO. TJE/002403/2022

1.INTRODUCTION:

M/S NATWEST ESTATES PRIVATE LIMITED has proposed to construct a Stilt Floor + Five Floors Building at Survey Nos. 25/80, 25/81, Mookambikai Nagar, Noombal Village, Poonammalle Taluk, Tiruvallur District, Chennai. The work was entrusted to T.J. Engineers by the promoter. The location is shown in Fig No.1. The object of the soil exploration work done is to determine the soil properties of the proposed area and to arrive at a most suitable type of foundation system for the proposed structure.

2.0 SCOPE OF WORK

The Scope of Work includes the following

Boreholes

- Drilling of borehole at the proposed borehole locations as per the direction of the client.
- Collection of disturbed and undisturbed soil samples and carrying out the relevant laboratory test on soils.
- Conducting of standard penetration test (SPT) within the borehole at every 1.50m depth interval or as required.
- Collection of soil sample from bore hole.
- Preparation of geotechnical investigation Report.

3.0. LOCATION & NUMBER OF BORE HOLES

For this purpose two bore holes were bored up to maximum depth of 30.00m below existing ground level. The bore hole location is shown in Fig No.1 and bore log details

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are given in Fig No. 2&3. The bore holes were terminated in the Very Dense Clayey Silty Sand (Cemented Sand) layer.

4.0. GEOTECHNICAL INVESTIGATIONS

4.1. Filed Investigations of Boreholes

Rotary drilling boring technique was adopted for boring in this area. The Standard penetration tests were conducted at change of layers and at specified intervals, commencing the test at 1.50m below the existing ground level. The relative density of different soil layers obtained from the N-Values are given in the Fig No. 2&3. Disturbed and un disturbed samples were collected from all the layers for visual observation, classification tests and strength characteristics of the soil layers.

Standard Penetration Test (SPT)

Standard Penetration Tests were conducted using a split spoon sampler, as per relevant codal provision (IS: 2131 – 1981). The SPT is generally conducted in sandy and completely weathered rock strata till 45 cm penetration with number of blows recorded for every 15 cm penetration. The number of blows required for first 15 cm in neglected. The blows for last 30 cm penetration or refusal 'N' value of 100 whichever is earlier is taken as the 'N' value. In the clayey strata, when driving of sampling tube is not possible, SPT was conducted to know the consistency of the soil strata and to retrieve the disturbed soil sample. The soil samples obtained in the split spoon sampler are carefully packed in polythene bags for testing in the laboratory. The SPT values obtained at every 1.50m interval are given in the bore logs.

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4.2. Grain Size Analysis

Combined sieve and hydrometer analyses were conducted on the various samples collected. The results are given in Table-1. Based on the proportion of the different soil constituents, the soil is classified.

4.3. Laboratory Tests for Borehole samples

The following laboratory tests were conducted on collected Soil Samples.

I. Moisture Content as per IS: 2720 (Part 2)

II. Specific Gravity as per IS: 2720 (Part 2)

III. Sieve Analysis as per IS: 2720 (Part 4)

IV. Hydrometer Analysis as per IS: 2720 (Part 4)

V. Liquid Limit as per IS: 2720 (Part 5)

VI. Plastic Limit as per IS: 2720 (Part 5)

VII. Swelling Index as per IS: 2720 (Part 40)

5.0. BORE LOG DATA

The Summary of bore hole log data for BH-01 & BH-02 is given in Fig-2&3. Bore hole location is given in Fig.1.

6.0. LABORATORY TEST RESULTS

The laboratory test results of boreholes samples are presented in Table-1 and Table-2.

7.0. GROUND WATER TABLE

The ground water table was encountered at the **depth of 4.00m**, below the existing ground level at the time of soil exploration.

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8.0. SUB SOIL PROFILE

From the visual identification from field test results, it is concluding that soil strata is not similar in BH-01 & BH-02 location.

In BH-01 Location the soil strata is made-up of nine distinctive layers

i. Silty Clayey Sand, 0.00m to 0.80m

ii. Medium Dense Silty Sand, 0.80m to 4.00m SPT's N= 12-20

iii. Medium Stiff Silty Clay, 4.00m to 5.00m SPT's N= 6

iv. Medium Dense to Dense Silty Sand, 5.00m to 17.00m SPT's N= 13-47

v. Dense Clayey Silty Sand, 17.00m to 19.50m SPT's N= 37

vi. Hard Sandy Silty Clay, 19.50m to 21.00m SPT's N= 35

vii. Very Hard Sandy Silty Clay (Cemented Clay), 21.00m to 26.00m SPT's N= 51->100

viii. Very Dense Silty Clayey Gravelly Sand, 26.00m to 27.20m SPT's N= >100

ix. Very Dense Clayey Silty Sand (Cemented Sand), 27.20m to 30.00m SPT's N= >100

In BH-02 Location the soil strata is made-up of four distinctive layers

i. Silty Clayey Sand, 0.00m to 0.80m

ii. Medium Dense Silty Sand, 0.80m to 4.00m SPT's N= 14-21

iii. Medium Stiff Silty Clay, 4.00m to 6.80m SPT's N= 5-7

iv. Medium Dense Silty Sand, 6.80m to 10.50m SPT's N= 19-27

The soil profile details are presented in the Bore logs, in Fig. 2&3.

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9.0 FOUNDATION ANALYSIS

From the bore log data from 0.00m to 0.80m Silty Clayey Sand layer is present. From 0.80m to 4.00m loose to medium dense Silty Sand layer layer is present. From 4.00m to 6.80m medium stiff Silty Clay layer is present, this layer has got swelling character. From 6.80m to 17.00m medium dense to dense Silty Sand layer is present. From 17.00m to 19.50m dense Clayey Silty Sand layer is present. From 19.50m to 21.00m hard Sandy Silty Clay layer is present, this layer also has got swelling character. From 21.00m to 26.00m very hard Sandy Silty Clay layer is present, this layer also has got swelling character. From 26.00m to 27.20m very dense Silty Clayey Gravelly Sand layer is present. Below 27.20m very dense Clayey Silty Sand (Cemented Sand) layer is seen, this layer is fair to rest pile foundation for heavy loaded structure.

The layer up to 6.80m is the critical zone for open foundation system for light loaded structure, within the critical zone medium stiff Clayey layer and medium dense Sandy layer is present, and the Clayey layer has got swelling character. For light loaded structure open foundation of **Isolated Footing** or **Continues Strip Raft Footing or Raft Foundation** can be adopted. The open foundation can be placed at the minimum **depth of 1.80m** below the existing ground level. The existing ground level was same as the road level at the time of soil investigation.

For heavy loaded structure, if the SBC is not sufficient for the expected loading intensity, it is advisable to provide deep foundation of pile foundations. The pile foundation can be Bored Cast in situ pile. The pile foundation should be terminated at the minimum **depth of 30.00m (Including Anchorage Depth)** below existing ground level.

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The pile should be anchored into the very dense Clayey Silty Sand (Cemented Sand) layer for minimum of 2.00m depth.

9.1. COMPUTATION OF THE SAFE BEARING CAPACITY AT 1.80m DEPTH

Based on the standard penetration test value the ultimate bearing capacity (qu) could be calculated from the following formula.

$qu = \gamma\ DfNq + 0.4\ \gamma'\ B\ N\gamma$

The observed N-Value is taken as 10 from Fig. 2&3.

The corresponding bearing capacity factors are,

$Nq = 10.96$

$N\gamma = 8.16$

Where,

γ = Bulk density = 1.50 t/m 3

γ' = Submerged density = 0.50 t/m 3

Df = Depth of footing = Considered minimum of 1.80m.

Dw = Depth of water table = 4.00m

B = Breadth of the footing = Consider minimum of 1.00m.

Substitute all the above values in the formula,

Qu = 1.50 x 1.80 x 10.96 + 0.4 x 0.50 x 1.00 x 8.16

= 29.592 +1.632

= 31.224 t/m ^{2}

Safe bearing capacity could be considered with a factor of safety of 2.5

Hence, safe bearing capacity q s = 31.224 / 2.50 = 12.489 t/m ^{2}. Say 12.50 t/m ^{2}

Therefore, Safe Bearing Capacity of the soil at the depth of 1.80m is 12.50 t/m ^{2}

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9.2. SETTLEMENT ANALYSIS FOR THE LOADING INTENSITY OF 12.50 t/m^2

LAYER -1, SILTY CLAY

The Consolidation Settlement of the Clay layer can be calculated using the following formula.

$$S_1 = (C_c / 1 + e_0) H \quad Log_{10}(\sigma'_0 + \Delta\sigma / \sigma'_0)$$

Where

C_c = Coefficient of compression index

= 0.35

e_0 = The initial void ratio

= 1.80

H =The thickness of the compressible clay layer

= 5.00m

σ'_0 = The original effective over burden pressure at the centre of the clay layer

= 6.15 t/m^2

$\Delta\sigma$ = The stress increases at the centre of the clay layer due to foundation loads (or) pressure

= 0.444 t/m^2

S_1 = (0.35 /1+1.80) 5.00 Log_{10} (6.15 + 0.444 /6.15)

= 0.125 x 5.00 x 0.0305

= 0.019062m x 10^{-3}

= 19.062mm

TOTAL SETTLEMENT

= 19.062mm < 50mm

The maximum allowable settlement as per IS: 1904-1986 for R.C.C structure with Isolated footing foundation is 50mm- 75mm.**The settlement is within the permissible limit. Hence it is Safe**

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9.3. LOAD CARRYING CAPACITY OF THE CONVENTIONAL PILE

Based on the standard penetration test value the ultimate load carrying capacity (q_u) could be calculated from the following formula.

$$\mathbf{Qu = Qb + Qs}$$

Qs = Ultimate unit skin friction on an individual layer

$$= Ks\ \sigma'_{vo} \tan\delta\ \pi\ D\ L$$

Where

Ks = Co-efficient of horizontal soil stress = 0.471

σ'_{vo} = average effective overburden pressure = 14.50 t/m^2

δ = angle of wall friction = 32

D = Dia meter of pile = 0.40m

L = Length of individual layer = 2.00m

= 0.471 x 14.50 x 0.624 x 3.14 x 0.40 x 2.00

= 10.705 t

$q_b = Nq\sigma'_{vo}Ab$

where

Nq = Bearing Capacity Factor = 85

σ'_{vo} = effective overburden pressure at the toe = 15.00 t/m^2

Ab = Cross sectional area at the base

D = Dia of Pile = 0.40m

= 85 x 15.00 x 3.14 x 0.40^2 /4

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= 160.14 t

Qu = Qb + Qs

= 160.14 +10.705 = 170.845 t

Q safe = Qu/F.S = 170.845/2.5 = 68.338 say 68.00 t

The safe load carrying capacity for five different diameter of piles for the **depth of 30.00m** are worked out and are given below.

Dia of Pile	Depth of Pile	Safe Load (Tones)
400mm	30.00m	68
450mm	30.00m	86
500mm	30.00m	105
600mm	30.00m	150
750mm	30.00m	233

The above safe loads are provisional and shall be confirmed by conducting pile load test as per the guide line laid down in IS: 2911 (Part: IV).

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10.0. CONCLUSIONS

1. Soil in the shallow depth is fair to adopt open foundation system for light loaded structure.
2. For light loaded structure, open foundation of **Isolated Footing** or **Continues Strip Raft Footing** or **Raft Foundation** can be adopted.
3. The open foundation can be placed at the minimum **depth of 1.80m** below the existing ground level. The existing ground level was same as the road level at the time of soil investigation.
4. The **Safe Bearing Capacity (SBC) at 1.80m depth is 12.50 t/m ^{2}.**
5. All the foundation should rest on 100 mm thick river sand or M Sand and 100mm thick 1:4:8 concrete mat.
6. The foundation bed should be well compacted by ramming before laying PCC.
7. Since the soil strata is sandy precaution should be taken in deep excavation for underground sump, Swimming pool and Septic tank etc. The soil under footing should not get disturbed, if get disturbed the foundation depth should be taken up to the deep excavation depth. The **foundation should rest on the natural undisturbed strata.**
8. **For heavy loaded structure**, if the SBC is not sufficient for the expected loading intensity, it is advisable to provide deep foundation of pile foundations. The pile foundation can be **Bored Cast in situ pile**. The pile foundation can be terminated at the minimum depth of **30.00m** below existing ground level, existing ground level at the time of soil investigation.
9. The safe load carrying capacity for five different diameter of piles for the **depth of 30.00m (Including Anchorage Depth)** are worked and are given below.

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Dia of Pile	Depth of Pile	Safe Load (Tones)
400mm	30.00m	68
450mm	30.00m	86
500mm	30.00m	105
600mm	30.00m	150
750mm	30.00m	233

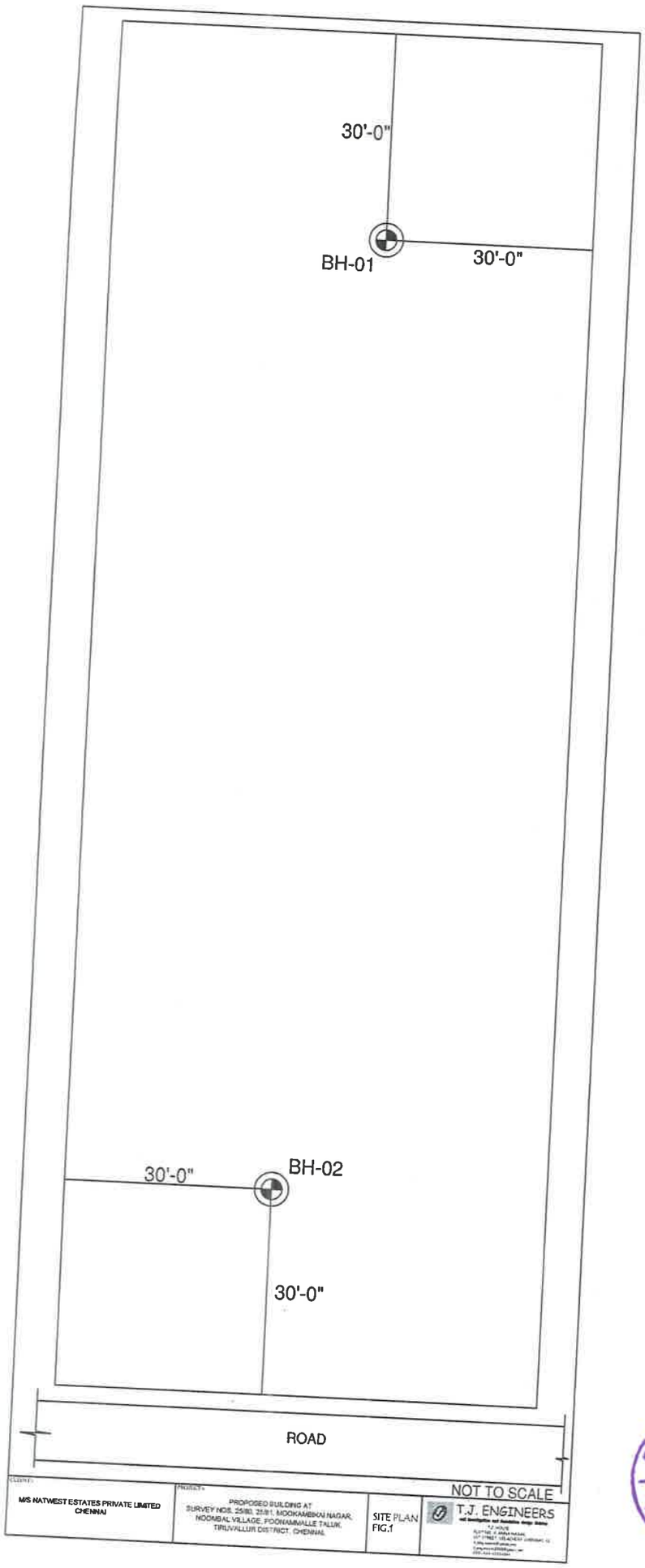
The above safe loads are provisional and shall be confirmed by conducting pile load test as per the guide line laid down in IS: 2911 (Part: IV)

10. The **pile should be anchored** in to the very dense Clayey Silty Sand (Cemented Sand) layer for minimum of **2.00m depth**.
11. The pile boring should be done with **Percussion** drilling or **Hydraulic** Rotary Drilling with DMC method.
12. Proper precaution should be taken while boring the pile against sliding of bore hole, by maintaining the proper dense bentonite slurry or providing temporary casing or permanent casing and pile concreting should be done as soon as boring is over.
13. The bore hole should be flushed properly before laying concrete. It has to be ensured by comparing the depth of pile bore hole and Tremie Pipe. The depth of Tremie Pipe lowered can be 300mm less than borehole depth. If the gap between bore hole bottom and Tremie Pipe bottom is more than 300mm, there is a possibility of slush deposit in the bore hole.
14. For foundation design stress due to any eccentricity and horizontal force should be considered and stability should be checked.
15. All suggested foundation recommendations and precautions should be adopted while undertaking the design aspects and construction activities.

For T.J. Engineers

04– 08 – 2022

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BORE LOG

PROJECT : Proposed Building at Survey Nos. 25/80, 25/81, Mookambikai Nagar, Noombal Village, Poonammalle Taluk, Tiruvallur District, Chennai.

BORE HOLE NO : B.H:01

TYPE OF BORING : Rotary Drilling

DATE OF COMMENCEMENT : 30 - 07 - 2022

DATE OF COMPLETION : 31 - 07 - 2022

G.W.L : 4.00m

Depth below G.L	Soil profile	Description of soil	Thickness of layer	Depth at witch samples are collected D.S	Depth at witch samples are collected U.D.S	Standard penetration testdata: Depth at witch test is conducted	N-Value	Relative density/ Consistency
0.00		Silty Clayey SLCS1 (Grayish Brown)	0.80	0.80				
1.00						1.50	10	Loose
2.00		Silty Sand-SLS1 (Whitish Yellowish Brown)	3.20					
3.00						3.00	16	Medium Dense
4.00				4.00				
		Silty Clay-SLC1 (Dark Gray)	1.00			4.50	6	Medium Stiff
5.00				5.00				
6.00						6.00	13	Medium Dense
7.00						7.50	16	Medium Dense
8.00								
9.00						9.00	25	Medium Dense
10.00						10.50	29	Medium Dense
11.00		Silty Sand-SLS2 (Whitish Gray)	12.00					
12.00						12.00	31	Dense
13.00						13.50	34	Dense
14.00								
15.00						15.00	44	Dense
16.00						16.50	47	Dense

Graphical representation of penetration resistance: 10 20 30 40 50

Remark: All Dimensions in meters

End of Boring

FIG: 2

cont....

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BORE LOG

PROJECT : Proposed Building at Survey Nos. 25/80, 25/81, Mookambikai Nagar, Noombal Village, Poonamallee Taluk, Tiruvallur District, Chennai.

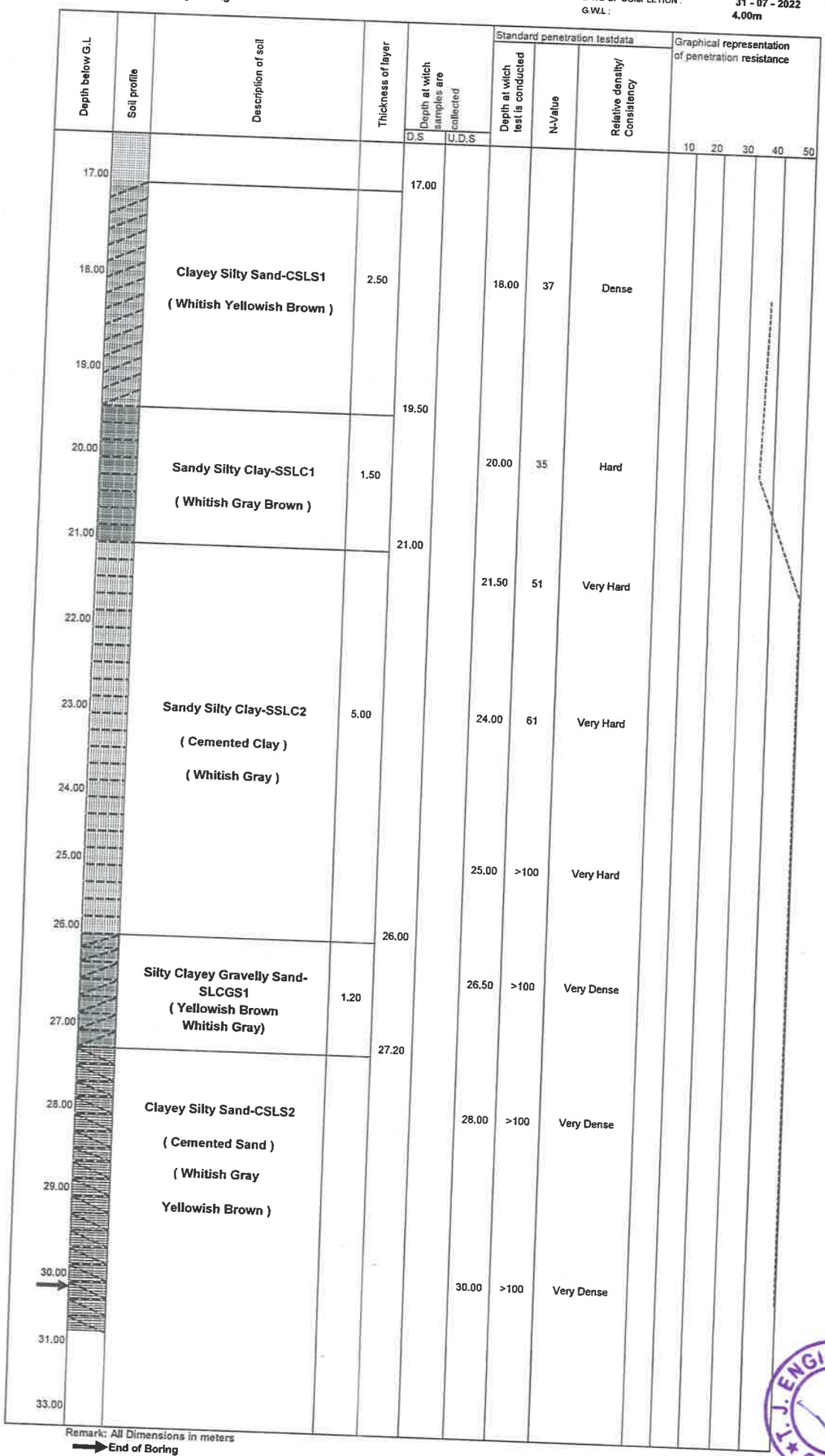
BORE HOLE NO : B.H:01

TYPE OF BORING : Rotary Drilling

DATE OF COMMENCEMENT : 30 - 07 - 2022

DATE OF COMPLETION : 31 - 07 - 2022

G.W.L : 4.00m



Remark: All Dimensions in meters

End of Boring

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FIG: 2

BORE LOG

PROJECT : Proposed Building at Survey Nos. 25/80, 25/81, Mookambikai Nagar, Noombal Village, Poonammalle Taluk, Tiruvallur District, Chennai.

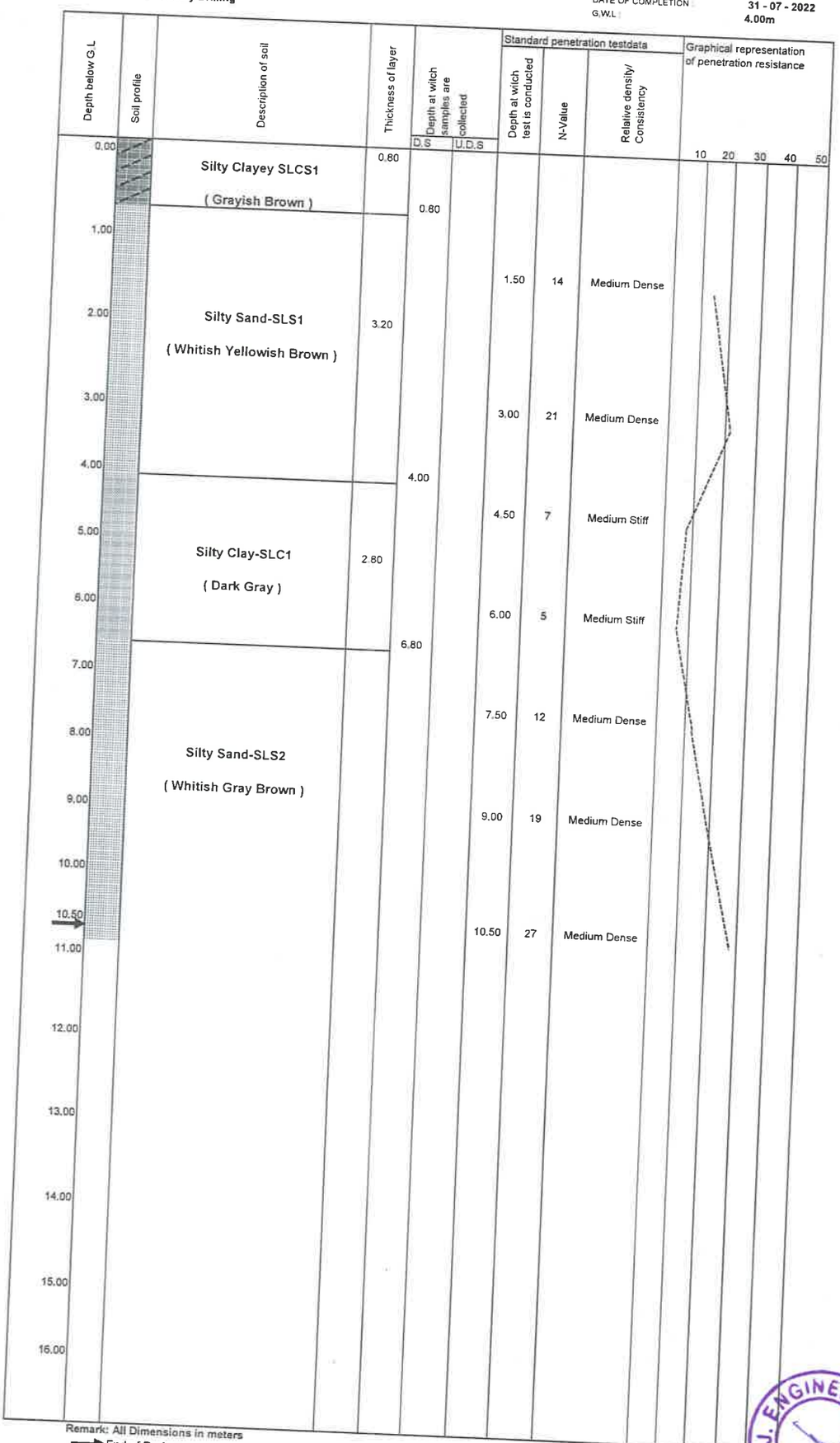
BORE HOLE NO : B.H:02

TYPE OF BORING : Rotary Drilling

DATE OF COMMENCEMENT : 31 - 07 - 2022

DATE OF COMPLETION : 31 - 07 - 2022

G.W.L : 4.00m



Remark: All Dimensions in meters

End of Boring

FIG: 3

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Project: Proposed Building at Survey Nos. 25/80, 25/81, Mookambikai Nagar, Noombal Village, Poonammalle Taluk, Tiruvallur District, Chennai.

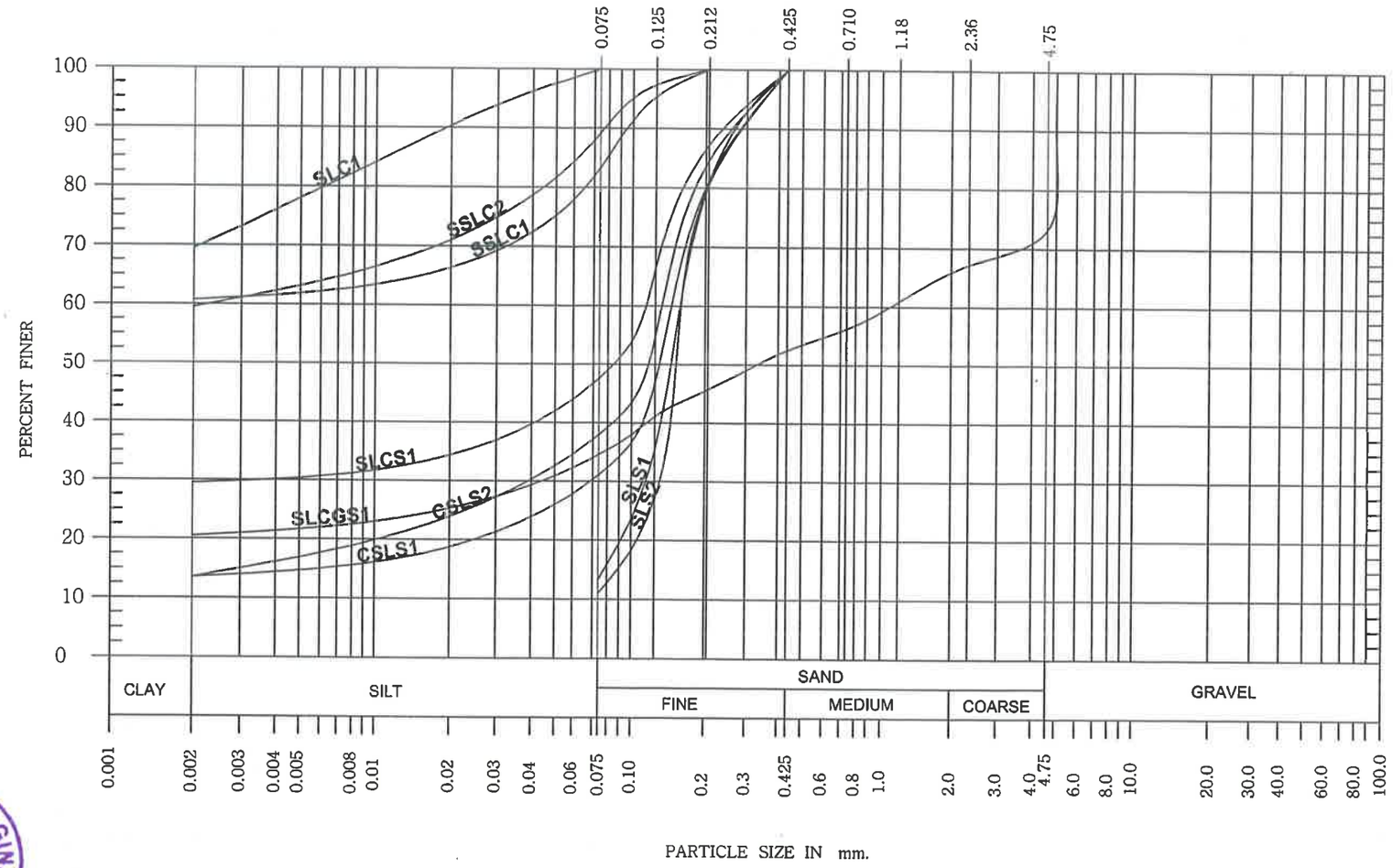


FIG. 4 GRAIN SIZE DISTRIBUTION CURVE-BH-01

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LABORATARY TEST RESULTS

TABLE-1

B.H No	Layer thickness	Soil classification	Depth of sample collected		SP. gravity	%of free swell	Grain size distribution				Remarks
			DS / SPT	UDS			Clay	Silt	Sand	Gravel	
	m		m	m		%	%	%	%	%	
1	0.00 - 0.80	Silty Clayey Sand-SLCS1	0.50	–	2.64	–	29	18	53	0	–
	0.80 - 4.00	Silty Sand-SLS1	1.50	–	2.63	–	0	14	86	0	–
	4.00 - 5.00	Silty Clay-SLC1	4.50	–	2.70	39	69	31	0	0	–
	5.00 - 17.00	Silty Sand-SLS2	6.00	–	2.63	–	0	12	88	0	–
	17.00 - 19.50	Clayey Silty Sand-CSLS1	18.00	–	2.64	–	14	18	68	0	–
	19.50 - 21.00	Sandy Silty Clay-SSLC1	20.00	–	2.66	35	61	23	16	0	–
	21.00 - 26.00	Sandy Silty Clay-SSLC2 (Cemented Clay)	21.50	–	2.65	29	59	30	11	0	–
	26.00 - 27.20	Silty Clayey Gravelly Sand-SLCGS1	26.50	–	2.63	–	21	14	39	26	–
	27.20 - 30.00	Clayey Silty Sand-CSLS2 (Cemented Sand)	30.00	–	2.65	–	14	24	62	0	–
2	0.00 - 0.80	Silty Clayey Sand-SLCS1	0.50	–	2.64	–	28	17	55	0	–
	0.80 - 4.00	Silty Sand-SLS1	1.50	–	2.63	–	0	13	87	0	–
	4.00 - 6.80	Silty Clay-SLC1	4.50	–	2.70	40	71	29	0	0	–
	6.80 - 10.50	Silty Sand-SLS2	10.50	–	2.63	–	0	11	89	0	–

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LABORATARY TEST RESULTS

TABLE -2

B.H No	Layer thickness	Soil classification	Depth of sample collected		LL	PL	Bulk density	Consolidation Parameters		Shear Strength Parmeters		Remarks
	m		DS / SPT m	UDS m	%	%	t/m³	e_o	Cc	Cu t/m²	φ degree	
1	0.00 - 0.80	Silty Clayey Sand-SLCS1	0.50	–	–	–	–	–	–	–	–	–
	0.80 - 4.00	Silty Sand-SLS1	1.50	–	–	–	1.54	–	–	–	31	–
	4.00 - 5.00	Silty Clay-SLC1	4.50	–	68	24	1.50	1.75	0.34	4.10	–	–
	5.00 - 17.00	Silty Sand-SLS2	6.00	–	–	–	1.56	–	–	–	35	–
	17.00 - 19.50	Clayey Silty Sand-CSLS1	18.00	–	–	–	1.57	–	–	–	36	–
	19.50 - 21.00	Sandy Silty Clay-SSLC1	20.00	–	59	21	1.50	–	–	23.45	–	–
	21.00 - 26.00	Sandy Silty Clay-SSLC2 (Cemented Clay)	21.50	–	58	19	1.53	–	–	34.17	–	–
	26.00 - 27.20	Silty Clayey Gravelly Sand-SLCGS1	26.50	–	–	–	1.58	–	–	–	38	–
	27.20 - 30.00	Clayey Silty Sand-CSLS2 (Cemented Sand)	30.00	–	–	–	1.60	–	–	–	41	–
2	0.00 - 0.80	Silty Clayey Sand-SLCS1	0.50	–	–	–	–	–	–	–	–	–
	0.80 - 4.00	Silty Sand-SLS1	1.50	–	–	–	1.54	–	–	–	32	–
	4.00 - 6.80	Silty Clay-SLC1	4.50	–	70	26	1.50	1.8	0.35	3.35	–	–
	6.80 - 10.50	Silty Sand-SLS2	10.50	–	–	–	1.54	–	–	–	34	–

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